BHAVESH SHRIMALI

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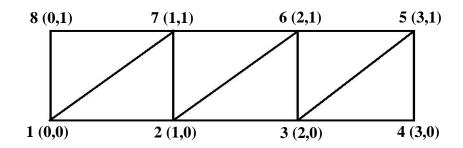
CEE 570: Finite Element Method

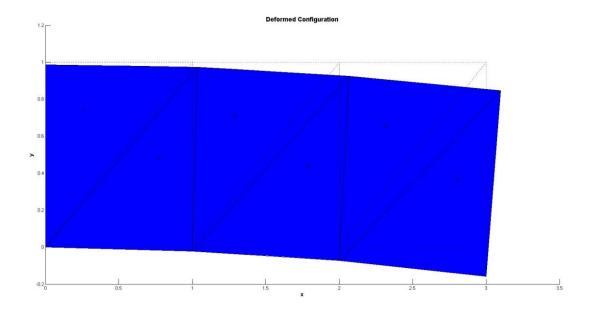
HW #4 Date: 03/06/2016

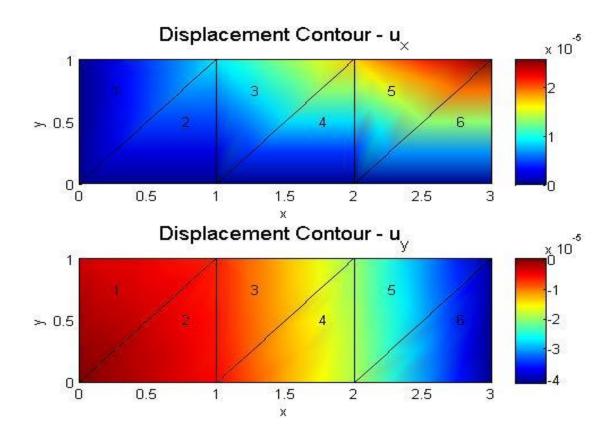
Problem 2:

(a)

Results:





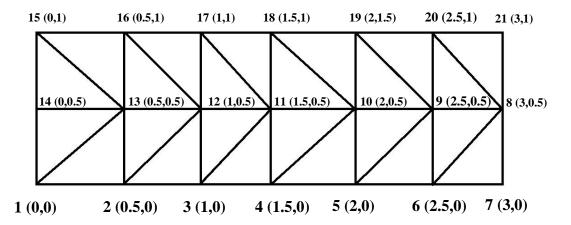


Displacement Values:

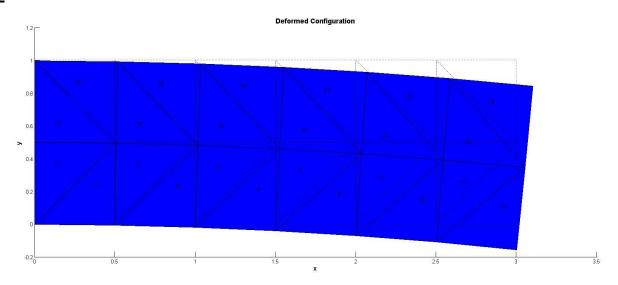
Node #	Displacement in x (* 10 ⁻⁴)	Displacement in y (* 10 ⁻⁴)
1	0	0
2	0	-0.0568
3	0	-0.1889
4	0	-0.4132
5	0.2559	-0.4013
6	0.1718	-0.1979
7	0.09	-0.0682
8	0	-0.035

The displacement values of the top right node, as obtained from the FEM code, is highlighted.

<u>2 (b):</u>



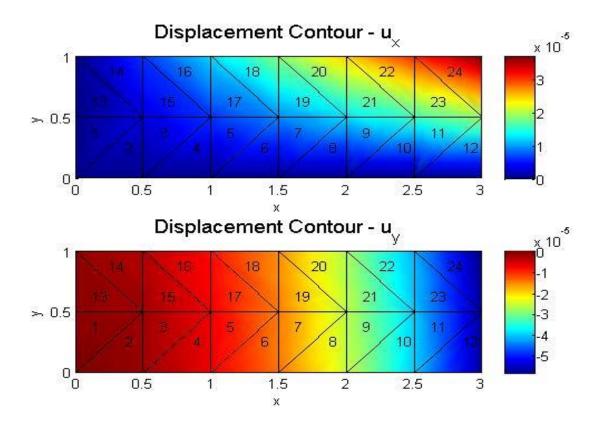
Results:



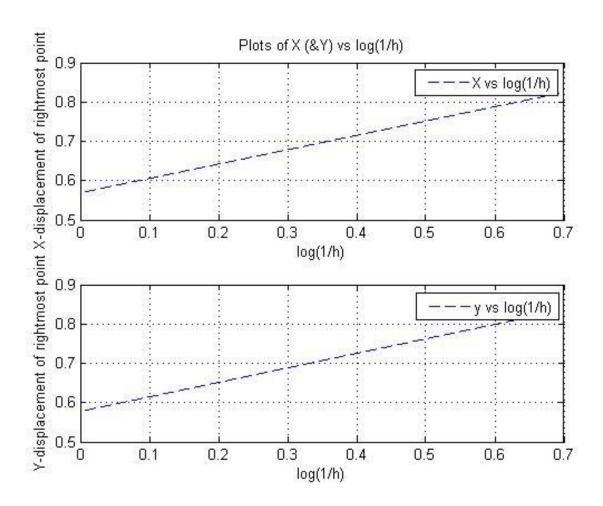
Node #	Displacement in x (* 10 ⁻⁴)	Displacement in y (* 10 ⁻⁴)
1	0	0
2	0	-0.0195
3	0	-0.0666
4	0	-0.1442
5	0	-0.2525
6	0	-0.3916
7	0	-0.5678
8	0.193	-0.5615

9	0.1577	-0.3957
10	0.1251	-0.2569
11	0.0935	-0.1483
12	0.0624	-0.0706
13	0.0312	-0.024
14	0	-0.0122
15	0	-0.0155
16	0.0628	-0.0352
17	0.1246	-0.0823
18	0.1863	-0.1602
19	0.2478	-0.2691
20	0.3087	-0.4087
21	0.3694	-0.579

The displacement of the top right node, as obtained from the FEM code, is highlighted in the table.



Plot of u vs log(1/h) for x and y displacement values {normalized with exact values}:



Matlab Code for Plotting:

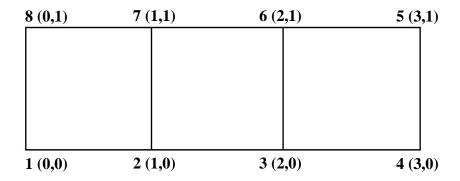
```
%Plot of u (&v) vs log(1/h) for different values of h:
x_plot=[log(1) log(2)];
u_plot=[0.2559 0.3694]/0.45;
v_plot=[-0.4013 -0.579]/(-0.69375);

subplot(2,1,1)
plot(x_plot,u_plot,'--');
grid on
xlabel('log(1/h)');
ylabel('X-displacement of rightmost point');
legend('X vs log(1/h)');
```

```
title('Plots of X (&Y) vs log(1/h)');
subplot(2,1,2)
plot(x_plot,v_plot,'--');
grid on
xlabel('log(1/h)');
ylabel('Y-displacement of rightmost point');
legend('y vs log(1/h)');
```

Problem 3:

Solution to 2 (a) using quads:

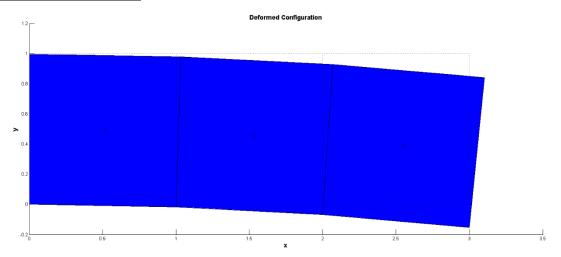


Nodal Displacements:

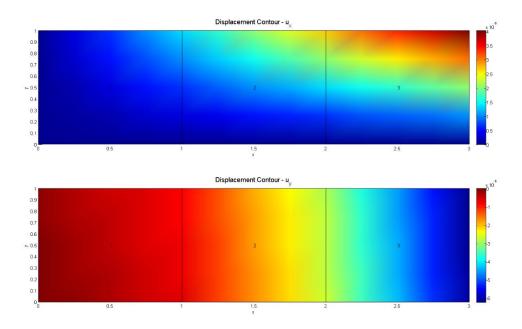
Node #	Displacement in x (* 10 ⁻⁴)	Displacement in y (* 10 ⁻⁴)
1	0	0
2	0	-0.0672
3	0	-0.2687
4	0	-0.6045
5	0.403	-0.6213
6	0.2687	-0.2854
7	0.1343	-0.084
8	0	-0.0168

The displacement of top right node is highlighted.

Deformed Configuration



Contour Plots:



Solution to 2(b) using quads:

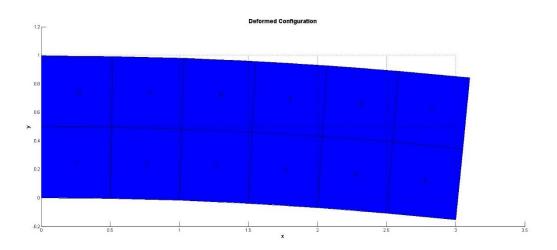
5 (0,1)	16 (0.5,1) 1	7 (1,1) 18	3 (1.5,1) 19	(2,1.5) 20	(2.5,1) 21	(3,1)
14 (0,0.5)	13 (0.5,0.5)	12 (1,0.5)	11 (1.5,0.5)	10 (2,0.5)	9 (2.5,0.5)	8 (3,0
(0,0)	2 (0.5,0) 3	(1,0) 4	(1.5,0)	5 (2,0) 6	(2.5,0)	7 (3,0)

Results:

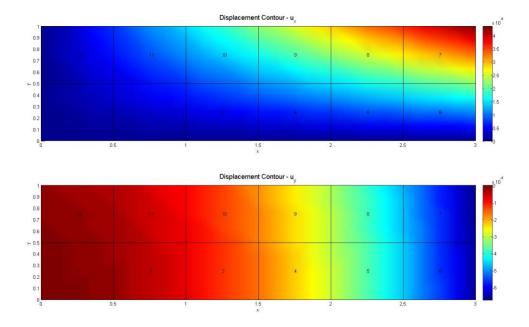
Node #	Displacement in x (* 10 ⁻⁴)	Displacement in y (* 10 ⁻⁴)
1	0	0
2	0	-0.0182
3	0	-0.0729
4	0	-0.164
5	0	-0.2915
6	0	-0.4551
7	0	-0.6558
8	0.22	-0.6599
9	0.1825	-0.46
10	0.1458	-0.2961
11	0.1093	-0.1685
12	0.0729	-0.0774
13	0.0364	-0.0228
14	0	-0.0046
15	0	-0.0182
16	0.0729	-0.0364
17	0.1457	-0.0911
18	0.2186	-0.1822
19	0.2915	-0.3098
20	0.3641	-0.4739
21	0.4356	-0.673

The nodal displacement is highlighted.

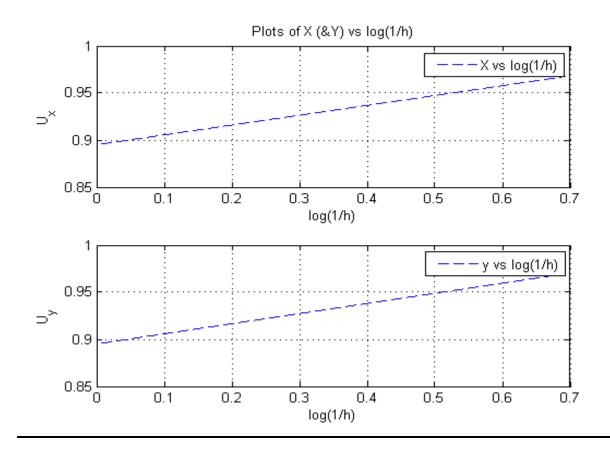
Deformed Configuration:



Contour Plots:



Plot of u vs log (1/h) for x and y displacement values {normalized with exact values}:



Problem 3(d): Plane Strain

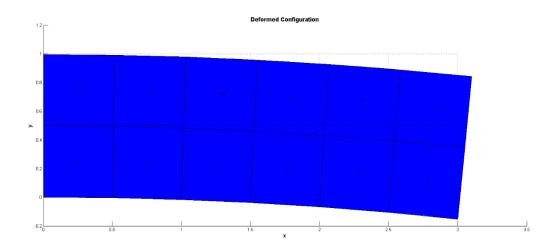
For the plane strain condition, the displacement values obtained for the case of 12 elements is as follows:

Node #	Displacement in x (* 10 ⁻⁴)	Displacement in y (* 10 ⁻⁴)
1	0	0
2	0	-0.017
3	0	-0.0682
4	0	-0.1534
5	0	-0.2727
6	0	-0.4257
7	0	-0.6135
8	0.206	-0.6188
9	0.1708	-0.4318
10	0.1364	-0.2784
11	0.1023	-0.1591
12	0.0682	-0.0739

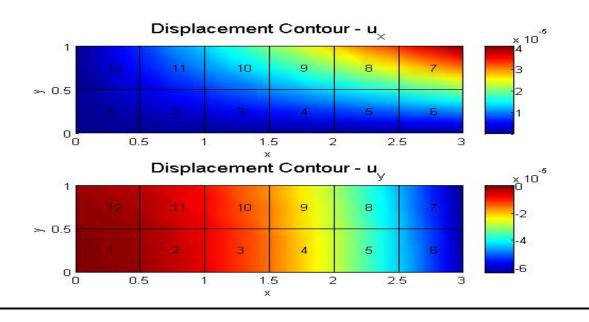
13	0.0341	-0.0227
14	0	-0.0057
15	0	-0.0227
16	0.0682	-0.0398
17	0.1364	-0.0909
18	0.2045	-0.1761
19	0.2727	-0.2955
20	0.3406	-0.449
21	0.4075	-0.6353

The displacement of the top right node is highlighted:

Deformed Configuration:



Contour Plots:

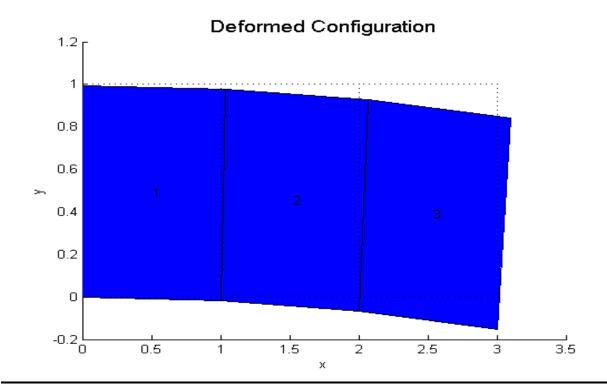


Part (ii)

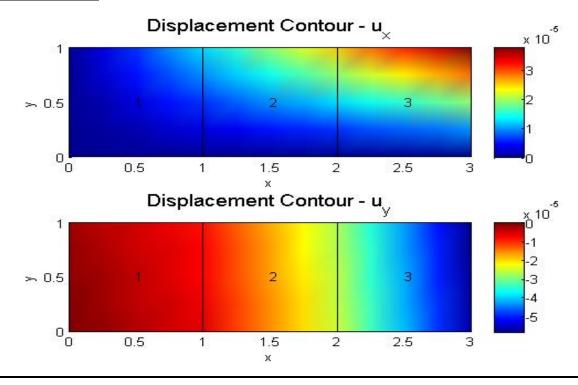
For the plane strain condition, the displacement values obtained for the case of 3 elements is as follows:

Node #	Displacement in x (* 10 ⁻⁴)	Displacement in y (* 10 ⁻⁴)
1	0	0
2	0	-0.0625
3	0	-0.25
4	0	-0.5625
5	0.375	-0.5833
6	0.25	-0.2708
7	0.125	-0.0833
8	0	-0.0208

The displacement of the top right node is highlighted.



Contour Plot:

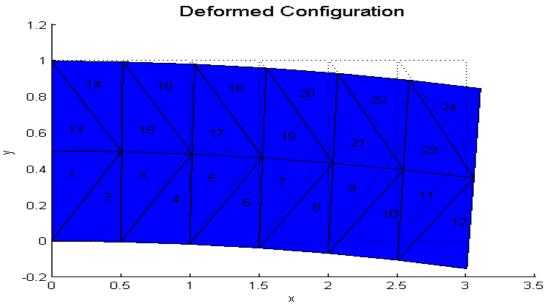


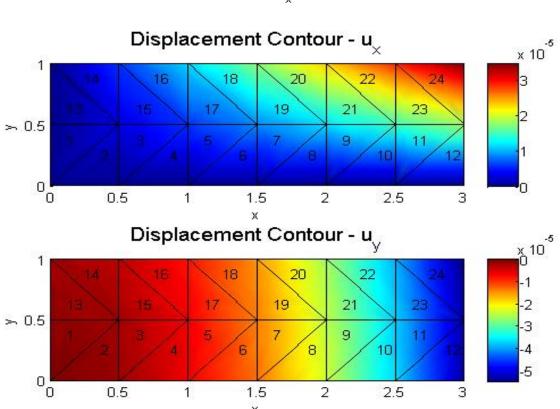
Using Triangular Elements:

Using triangular elements, and keeping number of elements equal to 24, the following results were obtained for the plane strain condition:

Node #	Displacement in x (* 10 ⁻⁴)	Displacement in y (* 10 ⁻⁴)
1	0	0
2	0	-0.0185
3	0	-0.0624
4	0	-0.1348
5	0	-0.2357
6	0	-0.3654
7	0	-0.5302
8	0.1804	-0.525
9	0.1474	-0.3706
10	0.1168	-0.2411
11	0.0873	-0.1399
12	0.0582	-0.0674
13	0.0291	-0.024
14	0	-0.0132
15	0	-0.0194
16	0.0586	-0.038

17	0.1162	-0.0819
18	0.1737	-0.1546
19	0.2311	-0.2561
20	0.2879	-0.3864
21	0.3452	-0.5459



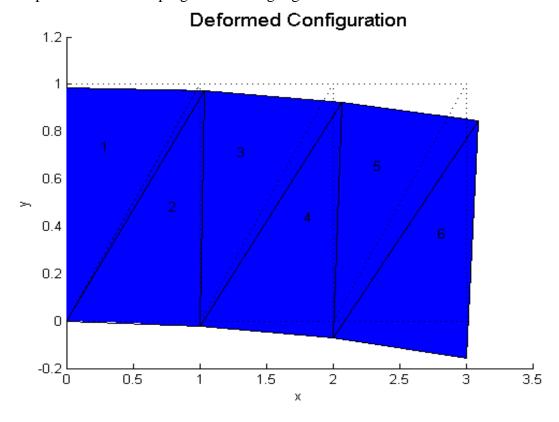


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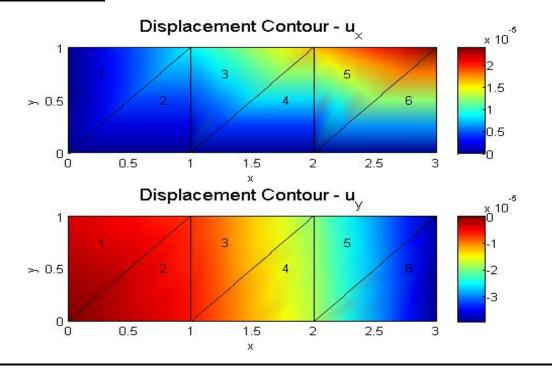
For the case of triangular elements, and when the number of elements is equal to 3, the following results were obtained.

Node #	Displacement in x (* 10 ⁻⁴)	Displacement in y (* 10 ⁻⁴)
1	0	0
2	0	-0.0557
3	0	-0.1807
4	0	-0.3918
5	0.2395	-0.3824
6	0.1618	-0.1922
7	0.0861	-0.0693
8	0	-0.0389

The displacement of the top right node is highlighted



Contour Plot:



Summary of the tabulated values of displacement of the top right node for Plane Stress and Plane Strain cases:

Case	Element Type	# of Elements	Ux (*10-4)	U _y (10 ⁻⁴)
		6	0.2559	-0.4013
	Triangular	24	0.3694	-0.579
		3	0.403	-0.6213
Plane Stress	Quadrilateral	12	0.4356	-0.673
		6	0.2395	-0.3824
	Triangular	24	0.3452	-0.5459
		3	0.375	-0.5833
Plane Strain	Quadrilateral	12	0.4075	-0.6353

The input files used for the assignment are appended below:

For Plane Strain:

(a) Triangular Elements (# of Elements = 6)

```
% Mesh Nodal Coordinates
NodeTable = [0 0]
             1 0
             2 0
             3 0
             3 1
             2 1
             1 1
             0 1];
numnp = length(NodeTable);
% Mesh Element Connectivities
ix = [1 7 8 0 1]
     1 2 7 0 1
      2 6 7 0 1
      2 3 6 0 1
      3 5 6 0 1
      3 4 5 0 1
      ];
nen = 4;
numel = 6;
% Mesh Boundary Conditions and Loads
BCLIndex = [6 1]';
NodeBC = [1 1 0]
          1 2 0
          2 1 0
          3 1 0
          4 1 0
          8 1 0
          ];
NodeLoad = [5 1 5];
% Mesh Material Properties
young = 10e5;
pois = .25;
```

```
thick = 1;
PSPS = 'n';
MateT = [young pois thick];
FEA Program
(b) Triangular Elements (# of Elements = 24)
% Mesh Nodal Coordinates
NodeTable = [0 0]
              0.5 0
              1 0
              1.5 0
              2 0
              2.5 0
              3 0
              3 0.5
              2.5 0.5
              2 0.5
              1.5 0.5
              1 0.5
              0.5 0.5
              0 0.5
              0 1
              0.5 1
              1 1
              1.5 1
              2 1
              2.5 1
              3 11;
```

numnp = length(NodeTable);

12 16 13 0 1

```
12 17 16 0 1
      11 17 12 0 1
      11 18 17 0 1
      10 18 11 0 1
      10 19 18 0 1
      9 19 10 0 1
      9 20 19 0 1
      8 20 9 0 1
      8 21 20 0 1
      ];
nen = 4;
numel = 24;
% Mesh Boundary Conditions and Loads
BCLIndex = [10 2]';
NodeBC = [1 1 0]
          1 2 0
          2 1 0
          3 1 0
          4 1 0
          5 1 0
          6 1 0
          7 1 0
          14 1 0
          15 1 0];
NodeLoad = [8 1 3.75]
            21 1 3.125];
% Mesh Material Properties
young = 10e5;
pois = .25;
thick = 1;
PSPS = 'n';
MateT = [young pois thick];
FEA Program
(c) Quadrilateral Elements (# of elements = 3)
% Mesh Nodal Coordinates
NodeTable = [0 0]
             1 0
             2 0
             3 0
             3 1
             2 1
             1 1
```

```
0 1];
numnp = length(NodeTable);
% Mesh Element Connectivities
ix = [1 2 7 8 1]
      2 3 6 7 1
      3 4 5 6 1];
nen = 4;
numel = 3;
% Mesh Boundary Conditions and Loads
BCLIndex = [6 1]';
NodeBC = [1 1 0]
          1 2 0
          2 1 0
          3 1 0
          4 1 0
          8 1 01;
NodeLoad = [5 1 5];
% Mesh Material Properties
young = 10e5;
pois = .25;
thick = 1;
PSPS = 'n';
MateT = [young pois thick];
FEA Program
       Modification in the Elas_2D file:
  (i)
if PSPS == 's' %Plane Stress
    Dmat = ElemE/(1-Elemv^2)*[1 Elemv 0; Elemv 1 0; 0 0 (1-
Elemv)/2];
else %Plane Strain
    Dmat = ElemE/((1+Elemv) * (1-2*Elemv)) * [1-Elemv Elemv 0; Elemv
1-Elemv 0;0 0 (1-2*Elemv)/2;
end
```

(ii) Modification in the shp_2l.m file:

```
elseif nel == 4
```

```
% shp(i,j). j = denotes the node number,
% i = 1, derivative w.r.t r
% i = 2, derivative w.r.t s
% i = 3, shape funtion
clear shp
syms xc eta real
shp(3,1)=0.25*(1-xc)*(1-eta);
shp(1,1) = diff(shp(3,1),xc);
shp(2,1) = diff(shp(3,1),eta);
shp(3,2) = 0.25*(1+xc)*(1-eta);
shp(1,2) = diff(shp(3,2),xc);
shp(2,2) = diff(shp(3,2),eta);
shp(3,3) = 0.25*(1+xc)*(1+eta);
shp(1,3) = diff(shp(3,3),xc);
shp(2,3) = diff(shp(3,3),eta);
shp(3,4) = 0.25*(1-xc)*(1+eta);
shp(1,4) = diff(shp(3,4),xc);
shp(2,4) = diff(shp(3,4),eta);
xc=r;
eta=s;
shp=double(subs(shp));
```

(d)Quadrilateral Elements (# of elements = 12):

```
2 0
             2.5 0
             3 0
             3 0.5
             2.5 0.5
             2 0.5
             1.5 0.5
             1 0.5
             0.5 0.5
             0 0.5
             0 1
             0.5 1
             1 1
             1.5 1
             2 1
             2.5 1
             3 1
             ];
numnp = length(NodeTable);
% Mesh Element Connectivities
ix = [1 \ 2 \ 13 \ 14 \ 1]
      2 3 12 13 1
      3 4 11 12 1
      4 5 10 11 1
      5 6 9 10 1
      6 7 8 9 1
      8 21 20 9 1
      9 20 19 10 1
      10 19 18 11 1
      11 18 17 12 1
      12 17 16 13 1
      13 16 15 14 1
      ];
nen = 4;
numel = 12;
% Mesh Boundary Conditions and Loads
BCLIndex = [10 2]';
NodeBC = [1 1 0]
          1 2 0
          2 1 0
          3 1 0
          4 1 0
```

1.5 0

```
5 1 0
6 1 0
7 1 0
14 1 0
15 1 0
];

NodeLoad = [8 1 3.75
21 1 3.125];

% Mesh Material Properties
young = 10e5;
pois = .25;
thick = 1;
PSPS = 'n';
MateT = [young pois thick];
```