

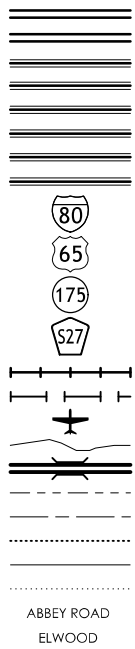
BRIDGE NEW - PPCB
IM-035-3(203)87--13-77
07-19-2022

POLK COUNTY
POLK COUNTY - DESIGN NO. 2118

LEGEND

INTERSTATE HIGHWAY
PRIMARY HIGHWAY-DIVIDED
PRIMARY HIGHWAY
PORTLAND CEMENT CONCRETE ROAD
ASPHALT ROAD
BITUMINOUS ROAD
GRAVEL ROAD
EARTHEN ROAD

INTERSTATE HIGHWAY
UNITED STATES HIGHWAY
STATE HIGHWAY
COUNTY HIGHWAY
RAILROAD
PIPELINE
AIRPORT
HYDROLOGY
BRIDGE
STATE BOUNDARY
COUNTY BOUNDARY
CORPORATE BOUNDARY
TOWNSHIP LINE
SECTION LINE
ROAD NAMES
UNINCORPORATED PLACE



PLANS OF PROPOSED IMPROVEMENTS ON THE
INTERSTATE ROAD SYSTEM
POLK COUNTY
BRIDGE NEW - PPCB
RAMP B OVER UPRR
FRA NO. X

THE IOWA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY AND BRIDGE CONSTRUCTION, SERIES 2015, PLUS APPLICABLE GENERAL SUPPLEMENTAL SPECIFICATIONS, DEVELOPMENTAL SPECIFICATIONS, SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS SHALL APPLY TO CONSTRUCTION WORK ON THIS PROJECT.

ENGLISH STANDARD
BRIDGE PLANS

STANDARD ISSUED REVISED

PRELIMINARY
NOT FOR CONSTRUCTION

REVISIONS

TOTAL SHEETS
53

PROJECT NUMBER

IM-035-3(203)87--13-77

R.O.W. PROJECT NUMBER

PROJECT IDENTIFICATION NUMBER

10-77-035-010-03

INDEX OF SHEETS

NO.	DESCRIPTION
1	TITLE SHEET
2	ESTIMATE SHEET - DESIGN NO. 2118
2-35	DESIGN NO. 2118
SPS.1-SPS.2	SOIL PROFILE SHEET
C.1	ESTIMATE SHEET FOR ROADWAY
A.1-U.1	ROADWAY SHEETS



1-800-292-8989

www.iowaonecall.com



STANDARD ROAD
PLANS

STANDARD ROAD PLANS ARE LISTED
ON SHEET NUMBER C.1

DESIGN DATA URBAN

2022	AADT	5,300	V.P.D.
2050	AADT	6,400	V.P.D.
2050	DHV	1,030	V.P.H.
TRUCKS		8	%
Total			
Design	ESALs	-	

INDEX OF SEALS

SHEET NO.	NAME	TYPE
1	RYAN R. PARADIS	STRUCTURAL DESIGN
SPS.1	BRIAN T. HAVENS	SOILS DESIGN
A.1	CHRISTOPHER M. MALMBERG	ROADWAY DESIGN

STRUCTURAL DESIGN



I hereby certify that this engineering document was prepared by me or under my direct personal supervision and that I am a duly licensed Professional Engineer under the laws of the State of Iowa.

Signature RYAN R. PARADIS Date ??-??-20??
Printed or Typed Name

My license renewal date is December 31, 2022

Pages or sheets covered by this seal: SHEETS 1 THRU 35

PROJECT WEBSITE:
<http://www.e-Builder.net>
ACCESS TO THE PROJECT WEBSITE FOR SUBCONTRACTORS, FABRICATORS, AND SUPPLIERS SHALL BE GRANTED BY THE PRIME CONTRACTOR.

ALL WORKING DRAWINGS INCLUDING SHOP DRAWINGS AND FALSEWORK DRAWINGS SHALL BE SUBMITTED THROUGH THE PROJECT WEBSITE AND WILL BE REVIEWED BY:
HDR ENGINEERING
BRIDGE SECTION
1917 S. 67TH STREET
OMAHA, NE 68106

LOCATION MAP
PART OF CITY OF DES MOINES

PROJECT DIRECTORY NAME: 7703501010

ESTIMATED BRIDGE QUANTITIES					
ITEM NO.	ITEM CODE	ITEM	UNIT	TOTAL	AS BUILT QUANTITY
1	2402-2720000	EXCAVATION, CLASS 20	CY	430	
2	2403-0100010	STRUCTURAL CONCRETE (BRIDGE)	CY	88.0	
3	2403-7000210	HIGH PERFORMANCE STRUCTURAL CONCRETE	CY	322.0	
4	2404-7775000	REINFORCING STEEL	LB	22,849	
5	2404-7775005	REINFORCING STEEL, EPOXY COATED	LB	7,969	
6	2404-7775009	REINFORCING STEEL, STAINLESS STEEL	LB	66,724	
7	2407-0562865	BEAM, PPC, BTB65	EACH	4	
8	2407-0562870	BEAM, PPC, BTB70	EACH	4	
9	2407-0562875	BEAM, PPC, BTB75	EACH	4	
10	2408-7800000	STRUCTURAL STEEL	LB	3,780	
11	2414-6424038	CONCRETE BARRIER RAIL, 3'-8"	LF	462.0	
12	2501-0201057	PILES, STEEL, HP 10 X 57	LF	4,070	
13	2501-6335010	PREBORED HOLES	LF	160	
14	2507-2638620	MACADAM STONE SLOPE PROTECTION	SY	390	
15	2507-2638660	BRIDGE WING ARMORING - MACADAM STONE	SY	20	
16	2526-8285000	CONSTRUCTION SURVEY	LS	1.0	
17	2533-4980005	MOBILIZATION	LS	1.0	
18	2595-0005150	RR PROT LIAB INSUR FOR UPRR	LS	1.0	

ITEM NO.	ESTIMATE REFERENCE INFORMATION
1	EXCAVATION QUANTITIES FOR PIER NO.1 HAVE BEEN COMPUTED ASSUMING THE PROPOSED GRADING FOR PROJECT NUMBER IM-035-3(194)87--13-77 IS COMPLETED PRIOR TO BRIDGE CONSTRUCTION. IN ALL OTHER LOCATIONS PIER EXCAVATION QUANTITIES ARE BASED ON EXISTING GROUND CONDITIONS.
2	ALL ABUTMENT FOOTING AND PIER FOOTING CONCRETE IS TO BE CLASS "C".
3	THIS BID ITEM INCLUDES THE CONCRETE FOR THE DECK, ABUTMENT DIAPHRAGMS, PIER DIAPHRAGMS AND WINGWALLS. REFER TO THE "DEVELOPMENTAL SPECIFICATIONS FOR HIGH PERFORMANCE CONCRETE FOR STRUCTURES" FOR ADDITIONAL INFORMATION. INCLUDES ALL RESILIENT JOINT FILLER REQUIRED. INCLUDES FURNISHING AND PLACING SUBDRAIN (INCLUDING EXCAVATION), FLOODABLE BACKFILL, POROUS BACKFILL, GEOTEXTILE FABRIC, WATER FLOODING AND SUBDRAIN OUTLET AT ABUTMENTS AND TOE OF BERM. INCLUDES FURNISHING AND PLACING 3 INCH DIAMETER PVC PLASTIC PIPE AND EXPANDING FOAM IN THE ABUTMENT WINGS. INCLUDES COST FOR MATERIALS AND LABOR TO PROVIDE CONCRETE RUSTICATION LINES.
7-9	INCLUDES PIER AND ABUTMENT BEARING MATERIAL AND COIL TIES. INCLUDES CONTRACTOR FILLING OUT BEAM NUMBERS BY LOCATION AND BEAM SEAT ELEVATIONS IN "PPC BEAM DATA SPREADSHEET" AND FORWARDING ELECTRONIC SPREADSHEET TO THE ENGINEER.
11	INCLUDES MATERIAL AND LABOR ASSOCIATED WITH PROVIDING AND INSTALLING THE RIGID STEEL CONDUIT, JUNCTION BOXES, AND FITTINGS. INCLUDES 462 L.F. OF 2" DIAMETER RIGID STEEL CONDUIT. IF PLACEMENT OF CONCRETE IS DONE BY THE SLIPFORMING METHOD, CLASS BR CONCRETE IS REQUIRED. CAST-IN-PLACE BARRIER RAILS SHALL USE CLASS C MIX. PRICE BID FOR THIS ITEM SHALL INCLUDE THE COST OF CAST-IN-PLACE FORMS IF REQUIRED FOR PLACEMENT OF THE CONCRETE.
12	PILING SHALL BE GRADE 50. FOR ADDITIONAL NOTES, SEE DESIGN SHEETS 9 - 11.
14	INCLUDES FURNISHING AND PLACING ENGINEERING FABRIC, MACADAM STONE, 4" x 6" TREATED TIMBERS, ½" DIAMETER STEEL PINS (OR REBARS), POROUS BACKFILL OR GRANULAR SUBBASE BACKFILL AT FRONT FACE OF ABUTMENT FOOTING AND ALL REQUIRED EXCAVATING, SHAPING, AND COMPACTING.
15	INCLUDES FURNISHING AND PLACING ENGINEERING FABRIC, MACADAM STONE, 4" x 6" TREATED TIMBERS, ½" DIA. STEEL PINS (OR REBARS), AND ALL REQUIRED EXCAVATING, SHAPING AND COMPACTING FOR WING ARMORING.

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ROADWAY QUANTITIES ARE SHOWN ELSEWHERE IN THESE PLANS.

DESIGN FOR VARIABLE SKEW RADIUS = 1,820'

214'-0 X 28'-0 PRETENSIONED
PRESTRESSED CONCRETE BEAM BRIDGE

66'-0 & 71'-0 END SPANS 77'-0 INTERIOR SPAN

ESTIMATED QUANTITIES

STA. 32591+41.72 (RAMP B) APRIL, 2022

POLK COUNTY

IOWA DEPARTMENT OF TRANSPORTATION

DESIGN SHEET NO. 1 OF 34 FILE NO. 31046 DESIGN NO. 2118



SUMMARY OF CONCRETE QUANTITIES

LOCATION	STRUCTURAL CONCRETE	HPC STRUCTURAL CONCRETE
NORTH ABUT.FTG.	14.4	-
SOUTH ABUT.FTG.	14.4	-
BRIDGE DECK + ABUT.& PIER DIAPHRAGM	-	228.0
ABUTMENT WINGS	-	7.6
PIER NO. 1	29.6	43.2
PIER NO. 2	29.6	43.2
TOTAL (CU. YDS.)	88.0	322.0

SUMMARY OF REINFORCING STEEL

LOCATION	NON-COATED REINFORCING STEEL	STAINLESS STEEL REINFORCING STEEL	EPOXY COATED REINFORCING STEEL
BRIDGE DECK + ABUTMENT FOOTING *	165	49,849	7,011
ABUTMENT WINGS	-	-	792
BARRIER RAILS	-	15,043	166
BARRIER RAIL END SECTIONS	-	1,832	-
PIER NO. 1	11,342	-	-
PIER NO. 2	11,342	-	-
* INCLUDES ABUTMENT DIAPHRAGMS AND PIER DIAPHRAGMS			
TOTAL (LBS.)	22,849	66,724	7,969

SUMMARY OF EXCAVATION

LOCATION	CLASS 20 EXCAVATION
NORTH ABUTMENT	65
SOUTH ABUTMENT	65
PIER NO. 1	160
PIER NO. 2	140
TOTAL (CU. YDS.)	430

SUMMARY OF FOUNDATIONS

LOCATION	SUBSTRUCTURE TYPE	FOUNDATION TYPE	NUMBER	LENGTH (LIN. FT.)	TOTAL (LIN. FT.)
NORTH ABUTMENT	INTEGRAL ABUTMENT	HP 10x57	8	75	600
SOUTH ABUTMENT	INTEGRAL ABUTMENT	HP 10x57	8	75	600
PIER NO. 1	FRAME PIER	HP 10x57	14	105	1,470
PIER NO. 2	FRAME PIER	HP 10x57	14	100	1,400
TOTAL HP 10x57 (LIN. FT.)					4,070

SUMMARY OF PREBORED HOLES

LOCATION	NUMBER	LENGTH (LIN. FT.)	TOTAL
NORTH ABUTMENT	8	10	80
SOUTH ABUTMENT	8	10	80
TOTAL (LIN. FT.)			160

SUMMARY OF STRUCTURAL STEEL

LOCATION	TOTAL (LBS.)
DIAPHRAGMS	3,780
TOTAL (LBS.)	3,780

SUMMARY OF BEARINGS

LOCATION	BEARING TYPE	NUMBER	ASSOCIATED BID ITEM
NORTH ABUTMENT	3 x 3 BAR	4	PPC BEAMS
SOUTH ABUTMENT	3 x 3 BAR	4	PPC BEAMS
PIER NO. 1	PLAIN NEOPRENE 1"	8	PPC BEAMS
PIER NO. 2	PLAIN NEOPRENE 1"	8	PPC BEAMS



DESIGN FOR VARIABLE SKEW
RADIUS = 1,820'
214'-0" X 28'-0" PRETENSIONED
PRESTRESSED CONCRETE BEAM BRIDGE
66'-0" & 71'-0" END SPANS
77'-0" INTERIOR SPAN
SUMMARY QUANTITIES
STA. 32591+41.72 @ RAMP B
APRIL, 2022
POLK COUNTY
IOWA DEPARTMENT OF TRANSPORTATION
DESIGN SHEET NO. 2 OF 34 FILE NO. 31046 DESIGN NO. 2118

SPECIFICATIONS:

DESIGN: AASHTO LRFD 8TH ED, SERIES OF 2017, EXCEPT AS NOTED IN THE CURRENT IOWA BRIDGE DESIGN MANUAL.

CONSTRUCTION: IOWA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY AND BRIDGE CONSTRUCTION, SERIES 2015, PLUS APPLICABLE GENERAL SUPPLEMENTAL SPECIFICATIONS, DEVELOPMENTAL SPECIFICATIONS, SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS, INCLUDING THE FOLLOWING SHALL APPLY TO CONSTRUCTION WORK ON THIS PROJECT:

- “DEVELOPMENTAL SPECIFICATIONS FOR HIGH PERFORMANCE CONCRETE FOR STRUCTURES”,
- “DEVELOPMENTAL SPECIFICATIONS FOR MAINTENANCE WORK ON RAILROAD RIGHT-OF-WAY (UNION PACIFIC RAILROAD)”,
- “SPECIAL PROVISIONS FOR E-BUILDER”.

DESIGN STRESSES:

DESIGN STRESSES FOR THE FOLLOWING MATERIALS ARE IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 8TH ED, SERIES OF 2017, EXCEPT AS NOTED IN THE CURRENT IOWA BRIDGE DESIGN MANUAL.

- REINFORCING STEEL IN ACCORDANCE WITH AASHTO LRFD SECTION 5, GRADE 60 FOR EPOXY COATED AND NON-COATED, AND GRADE 60 OR 75 FOR STAINLESS.
- CONCRETE IN ACCORDANCE WITH LRFD AASHTO SECTION 5, f’c = 4.0 KSI, EXCEPT PRESTRESSED BEAM CONCRETE AS NOTED.
- PRESTRESSED CONCRETE BEAMS, SEE DESIGN SHEET I9.
- STRUCTURAL STEEL IN ACCORDANCE WITH AASHTO LRFD SECTION 6, ASTM A709 GRADE 50 (AASHTO M270 GRADE 50), EXCEPT AS NOTED.

GENERAL NOTES:

THIS BRIDGE IS DESIGNED FOR HL-93 LOADING, PLUS 20 LBS. PER SQUARE FOOT OF ROADWAY FOR FUTURE WEARING SURFACE.

THE CITY AND UTILITY COMPANIES WHOSE FACILITIES ARE SHOWN ON THE PLANS OR KNOWN TO BE WITHIN THE CONSTRUCTION LIMITS SHALL BE NOTIFIED BY THE BRIDGE CONTRACTOR OF THE CONSTRUCTION STARTING DATE.

THE BRIDGE CONTRACTOR SHALL WORK IN SUCH A MANNER THAT EQUIPMENT AND MATERIALS SHALL NOT BE ALLOWED TO INTERFERE WITH TRAIN TRAFFIC OR BE ALLOWED TO FALL ON THE RAILROAD TRACKS. INTERFERENCE ABOVE THE RAILROAD TRACK AREA SHALL BE COORDINATED WITH THE RAILROAD.

FAINT LINES ON PLANS INDICATE THE EXISTING STRUCTURE.

EXCAVATION QUANTITIES FOR THE PIERS ARE BASED ON THE ASSUMPTION THAT ABUTMENT FILLS ARE IN PLACE PRIOR TO STARTING CONSTRUCTION OF THE PIERS.

IT SHALL BE THE BRIDGE CONTRACTOR’S RESPONSIBILITY TO PROVIDE SITES FOR EXCESS EXCAVATED MATERIAL. NO PAYMENT FOR OVERHAUL WILL BE ALLOWED FOR MATERIAL HAULED TO THESE SITES.

THE BRIDGE CONTRACTOR SHALL PREBORE HOLES FOR ABUTMENT PILES. HOLES SHALL BE BORED TO THE ELEVATIONS SHOWN ON THE “LONGITUDINAL SECTION ALONG RAMP B” ON DESIGN SHEET 4. PILES SHALL BE DRIVEN THROUGH THE HOLES TO AT LEAST THE SPECIFIED DESIGN BEARING.

ABUTMENT PILES SHALL NOT BE DRIVEN FOR A MINIMUM OF 75 DAYS FOLLOWING COMPLETION OF APPROACH FILLS. THE TIME PERIOD BETWEEN COMPLETION OF FILLS AND DRIVING PILES MAY BE CHANGED AS ORDERED BY THE ENGINEER BASED UPON REVIEW OF SETTLEMENT PLATES.

THE APPROACH FILLS AS SHOWN ARE TO BE DONE AS PART OF THE TIED PROJECT IM-035-3(194)87--13-77 AND ARE TO BE IN PLACE BEFORE ABUTMENT PILES ARE DRIVEN. THE BRIDGE CONTRACTOR IS TO LEVEL OFF AND SHAPE THE BERMS TO THE ELEVATIONS AND DIMENSIONS SHOWN. DRESSING OF SLOPES OUTSIDE THE BRIDGE AREA NOT DISTURBED BY THE BRIDGE CONTRACTOR SHALL BE PAID FOR AS EXTRA WORK.

CONCRETE BARRIER RAILS PLACED USING THE SLIPFORM METHOD WILL REQUIRE THE USED OF A CLASS BR CONCRETE IN ACCORDANCE WITH ARTICLE 2513.03, A, 2 OF THE STANDARD SPECIFICATIONS. CAST-IN-PLACE BARRIER RAILS SHALL USE CLASS C MIX. CLASS D CONCRETE IS NOT PERMITTED FOR CONCRETE BARRIER RAILS (CAST-IN-PLACE OR SLIPFORMED METHOD).

ALL REINFORCING IS TO BE SECURELY WIRED IN PLACE BEFORE CONCRETE IS POURED.

ALL EXPOSED CONCRETE CORNERS, 90 DEGREES OR SHARPER TO BE FILLETED WITH A ¾” DRESSED AND BEVELED STRIP, UNLESS NOTED OTHERWISE.

THESE BRIDGE PLANS LABEL ALL REINFORCING STEEL WITH ENGLISH NOTATION (5a1 is 5 8 inch diameter bar). ENGLISH REINFORCING STEEL RECEIVED IN THE FIELD MAY DISPLAY THE FOLLOWING “BAR DESIGNATION”. THE “BAR DESIGNATION” IS THE STAMPED IMPRESSION ON THE REINFORCING BARS, AND IS EQUIVALENT TO THE BAR DIAMETER IN MILLIMETERS.

ENGLISH SIZE	3	4	5	6	7	8	9	10	11
BAR DESIGNATION	10	13	16	19	22	25	29	32	36

ALL REINFORCING BARS AND BARS NOTED AS DOWELS SUPPLIED FOR THIS STRUCTURE SHALL BE DEFORMED REINFORCEMENT UNLESS OTHERWISE NOTED OR SHOWN.

KEYWAY DIMENSIONS SHOWN ON THE PLANS ARE BASED ON NOMINAL DIMENSIONS UNLESS STATED OTHERWISE. IN ADDITION, THE BEVEL USED ON THE KEYWAY SHALL BE LIMITED TO A MAXIMUM OF 10 DEGREES FROM VERTICAL.

DURING CONSTRUCTION OF THIS PROJECT THE BRIDGE CONTRACTOR WILL BE REQUIRED TO COORDINATE OPERATIONS WITH THOSE OF OTHER CONTRACTORS WORKING WITHIN THE SAME AREA. OTHER WORK IN PROGRESS DURING THE SAME PERIOD OF TIME IS SHOWN ON SHEET J.1 OF THESE PLANS.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ENSURING STABILITY OF PRESTRESSED CONCRETE BEAMS DURING ERECTION AND CONSTRUCTION UP THROUGH THE CONCRETE BRIDGE DECK REACHING ITS FULL 28-DAY STRENGTH. THE CONTRACTOR SHALL PROVIDE SUFFICIENT TEMPORARY ANCHOR BRACING AT BEAM ENDS AND TEMPORARY INTERMEDIATE BRACING AS NEEDED TO ENSURE PRESTRESSED BEAM STABILITY. PARTIALLY OR FULLY INSTALLED PERMANENT BRACING AS SHOWN IN THESE DESIGN PLANS SHALL NOT BE ASSUMED SUFFICIENT TO BRACE PRESTRESSED BEAMS DURING ERECTION AND CONSTRUCTION. TEMPORARY BRACING SHALL NOT BE WELDED TO PRESTRESSED BEAM STIRRUPS.

LONGITUDINAL GROOVING SHOWN ELSEWHERE IN THESE PLANS.

SHOP DRAWING SUBMITTALS	
SHOP DRAWINGS SHALL BE SUBMITTED FOR THE FOLLOWING ITEMS SHOWN IN THE TABLE BELOW. (NOTE ADDITIONAL SHOP DRAWINGS MAY BE REQUIRED IN ACCORDANCE WITH ARTICLE 1105.03 OF THE STANDARD SPECIFICATIONS.)	
SUBMITTAL REQUIREMENTS FOR SHOP DRAWINGS SHOULD BE IN ACCORDANCE WITH 1105.03 OF THE STANDARD SPECIFICATIONS FOR HIGHWAY AND BRIDGE CONSTRUCTION OF THE IOWA DEPARTMENT OF TRANSPORTATION.	
SHOP DRAWINGS SHALL BE SUBMITTED WITH THE FOLLOWING NAMING CONVENTION: (Paren)_County_DesignNumber_Submittal Description.pdf Example: (70)_Polk_Design2218_SteelDiaphragms.pdf	
I	STEEL DIAPHRAGMS

BRIDGE DECK DIMENSIONS TABLE

	ITEM	UNITS	QUANTITY
1	DECK LENGTH	L.F.	217.0
2	MINIMUM DECK WIDTH	L.F.	31.2
3	MAXIMUM DECK WIDTH	L.F.	31.2
4	DECK AREA	S.F.	6,763

- DECK LENGTH IS MEASURED FROM FACE-TO-FACE OF PAVING NOTCHES ALONG THE BRIDGE BASELINE.
3. DECK WIDTHS ARE MEASURED FROM OUT-TO-OUT OF DECK PERPENDICULAR TO THE BRIDGE BASELINE.
- DECK AREA IS TO BE BASED ON THE FACE-TO-FACE PAVING NOTCH DISTANCE AND OUT-TO-OUT DECK DIMENSIONS.

FORMS FOR PIER CAPS ON PIER NOS. 1 AND 2 MAY BE REMOVED WITH THE APPROVAL OF THE ENGINEER WHEN THE FOLLOWING TWO CONDITIONS HAVE BEEN MET:

- PIER CAP CONCRETE HAS BEEN IN PLACE FOR A MINIMUM OF 2 CALENDAR DAYS EXCLUDING DAYS THAT THE CONCRETE SURFACE IS SUBJECTED TO TEMPERATURES AT OR BELOW 40°F AND
- THE PIER CAP CONCRETE STRENGTH IS AT LEAST 2.50 KSI.

CONCRETE STRENGTH SHALL BE VERIFIED BY FLEXURAL STRENGTH ACCORDING TO MATERIALS I.M. 316 WITH A MINIMUM FLEXURAL STRENGTH OF 0.343 KSI OR BY THE MATURITY METHOD ACCORDING TO MATERIALS I.M. 383. CURING OF PIER CAP CONCRETE SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. PIER CAP CONCRETE SHALL ATTAIN A MINIMUM CONCRETE STRENGTH OF 4.00 KSI BEFORE BEING SUBJECTED TO EXTERIOR LOADS. PIER CAP CONCRETE SHALL BE SUBJECTED TO EXTERIOR LOADS IN ACCORDANCE WITH ARTICLE 2403.03, N, OF THE STANDARD SPECIFICATIONS.

AT THE CONTRACTORS OPTION TRANSPARENT STAY-IN-PLACE DECK FORMS MAY BE USED FOR THIS PROJECT. THE STAY-IN-PLACE FORMS SHALL HAVE A MINIMUM AVERAGE TRANSPARENCY OF 70%. ALL STRUCTURAL STEEL MEMBERS USED IN THE FORM ASSEMBLY (INCLUDING COLD-FORMED AND ROLLED) SHALL BE CORROSION PROTECTED. THE FORM ASSEMBLY SHALL HAVE A MAXIMUM UNIT WEIGHT OF 3.5 PSF OVER THE FULL FORM PANEL AREA. SHOP DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED FOR THE ENGINEER’S REVIEW. THE TRANSPARENT STAY-IN-PLACE FORM MATERIAL AND INSTALLATION COST SHALL BE INCLUDED IN THE PAY ITEM FOR STRUCTURAL CONCRETE (BRIDGE), WITH NO ADDITIONAL COST TO THE STATE.

GENERAL NOTES FOR CONCRETE RUSTICATION:

STRIPS AND PANELS USED AS INSERTS WITHIN CONCRETE FORMS TO CREATE THE RUSTICATION FEATURES MAY BE MADE OF WOOD, STEEL, PLASTIC OR OTHER NONPOROUS MATERIAL CAPABLE OF WITHSTANDING ANTICIPATED CONCRETE POUR PRESSURES WITHOUT PHYSICAL DEFECTS. WOOD INSERTS, IF USED, SHALL BE FREE OF WARP, TWIST, CHECKS OR CRACKS, AND SHALL BE PRESOAKED PRIOR TO PLACEMENT OF CONCRETE IN THE FORMS.

RUSTICATION INSERTS SHALL EASILY ATTACH TO FORMS AND SHALL NOT ALLOW LEAKAGE OF CONCRETE BETWEEN THE FORM AND THE INSERT. WHEN STEEL FORMS ARE USED, RUSTICATION STRIPS MAY BE RIGIDLY ATTACHED TO THE INSIDE SURFACES OF THE FORMS. WHEN STEEL FORMS ARE NOT USED, RUSTICATION STRIPS AND OTHER INSERTS FOR SMALL RECESSES ON EXPOSED CONCRETE SURFACES SHALL BE FASTENED TO THE FORMS IN A MANNER THAT WILL PERMIT THEM TO REMAIN IN PLACE WHEN THE FORMS ARE REMOVED. LEAVE INSERTS IN PLACE UNTIL THEY CAN BE REMOVED WITHOUT DAMAGE TO THE SURROUNDING CONCRETE.

THE INSERTS SHALL BE DESIGNED TO FORM SURFACES AND FEATURES CONFORMING TO THE DESIGN INTENT INCLUDING THE SHAPE, LINES, DEPTHS AND DIMENSIONS SHOWN IN THE PLANS. CREATE INSERTS USING A MINIMUM NUMBER OF SPLICE JOINTS IN THEIR LENGTH. SPLICES, IF USED, SHALL BE TIGHTLY JOINED SO AS NOT TO ALLOW GAPS OR LEAKS, AND SHALL NOT CREATE ANY CHANGE IN ALIGNMENT OR SHAPE OF THE RUSTICATION FEATURE. DO NOT LOCATE FORM TIES WITHIN CONCRETE RUSTICATIONS.

FOR RUSTICATION FEATURES FOLLOWING THE PERIMETER OF ROUNDED SURFACES, IT MAY BE NECESSARY TO USE MULTIPLE LAYERS OF INSERT MATERIAL IN ORDER TO ACHIEVE THE RADIUS CURVE. THIS IS ACCEPTABLE, PROVIDED THAT THE FINAL SHAPE, LINE, DEPTH, AND DIMENSION OF THE FEATURES ARE MAINTAINED IN THE FINAL RESULT.

DURING LOADING OF FORMS WITH CONCRETE, TAKE EXTRA CARE TO ENSURE PROPER CONSOLIDATION OF CONCRETE AROUND ALL RUSTICATION INSERTS TO PRESERVE THE SHAPE, LINE AND DEPTH OF ALL INTENDED FEATURES IN THE FINAL CONCRETE SURFACE. FOLLOWING REMOVAL OF FORMS, REPAIR ALL DEFECTS TO ACHIEVE THE RUSTICATION FEATURES AS SPECIFIED IN THE PLANS. PATCH VOIDS, HONEYCOMB AREAS, ETC., IN ACCORDANCE WITH THE STANDARED SPECIFICATIONS. IF SURFACES WILL NOT RECEIVE A COLORED SEALER COATING, ADD WHITE CEMENT TO THE PATCHING MORTAR TO LIGHTEN IT IN ORDER TO MATCH OR BE SLIGHTLY LIGHTER THAN SURROUNDING CONCRETE WHEN DRY. COMPLETED SURFACE SHALL BE FREE FROM BLEMISHES, SURFACE VOIDS AND CONSPICUOUS FORM MARKS TO THE SATISFACTION OF THE ENGINEER. THE CONTRACTOR SHALL CORRECT ANY SURFACE DEFECTS TO THE SATISFACTION OF THE ENGINEER AT NO ADDITIONAL COST TO THE PROJECT.

ALL COSTS ASSOCIATED WITH CONCRETE RUSTICATION ARE TO BE INCLUDED IN THE BID ITEM “HIGH PERFORMANCE STRUCTURAL CONCRETE”.

ALL PLAN DIMENSIONS ARE HORIZONTAL UNLESS NOTED OTHERWISE.

THE TIED ROAD PLANS, PROJECT NO. IM-035-3(194)87--13-77 CONTAIN THE POLLUTION PREVENTION PLAN.

TRAFFIC CONTROL PLAN:
THE STRUCTURE IS BEING CONSTRUCTED ON A RELOCATION AND THE ROAD WILL NOT BE OPEN TO TRAFFIC UNTIL AFTER COMPLETION OF CONSTRUCTION. SEE TIED PROJECT IM-035-3(194)87--13-77 FOR THE TRAFFIC CONTROL PLAN.

NO CONSTRUCTION ACTIVITY IS ALLOWED IN THE AREA SOUTH OF THE EXISTING UPRR TRACK UNTIL AFTER JANUARY 1ST, 2023. THIS INCLUDES GRADING AND ANY WORK RELATED TO THE CONSTRUCTION OF THE SOUTH ABUTMENT AND PIER NO. 2.

DESIGN FOR VARIABLE SKEW RADIUS = 1,820’

214’-0 X 28’-0 PRETENSIONED

PRESTRESSED CONCRETE BEAM BRIDGE

66’-0 & 71’-0 END SPANS 77’-0 INTERIOR SPAN

GENERAL NOTES

STA. 32591+41.72 (RAMP B) APRIL, 2022

POLK COUNTY

IOWA DEPARTMENT OF TRANSPORTATION

DESIGN SHEET NO. 3 OF 34 FILE NO. 31046 DESIGN NO. 2118



970		970
960		960
950		950
940		940
930		930
920		920
910		910
900		900

LONGITUDINAL SECTION ALONG CL RAMP B

TYPICAL APPROACH SECTION

CL FUTURE RAMP G

RAILROAD ROW LINE

EXISTING WIRE FENCE

FUTURE 204'-0" x 42'-0" PPCB BRIDGE

RAILROAD ROW LINE

CL EXISTING UPRR TRACK

GUARDRAIL (TYP.)

MACADAM STONE SLOPE PROTECTION (TYP.)

CL RAMP B & P.G.L.

2.5:1 MAX.

29.4' MIN. HORIZ. CL.

28.7' MIN. HORIZ. CL.

27.0' MIN. HORIZ. CL.

26.3' MIN. HORIZ. CL.

69°29'48"*

MIN. VERT. CL. (PT. A)

25'-0"

1°08'00"○

1°17'27"○

3°22'07"○

3°22'07"○

STA. 32591+37.39 CL RAMP B = CL U.P.R.R.

SPAN NO. 1 = 66'-0"

SPAN NO. 2 = 77'-0"

SPAN NO. 3 = 71'-0"

214'-0" CL TO CL ABUT. BRGS.

217'-0" FACE TO FACE OF PAVING NOTCHES

CL N. ABUT. BRG STA. 32590+34.72

CL PIER NO. 1 STA. 32591+00.72

CL PIER NO. 2 STA. 32591+77.72

CL S. ABUT. BRG STA. 32592+48.72

MEASURED ALONG CL RAMP B

1'-6 1/8

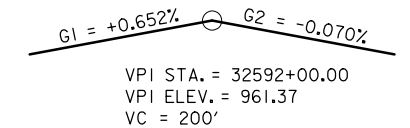
1'-6 1/8

UTILITIES

POWER POLE

A map of the study area showing the location of the bridge. The map includes a network of roads and a river. A line points from the text 'BRIDGE LOCATION' to a specific point on the river.

RAMP B (E.B. TO S.B.)
OVER UPRR
T-79N R-23W
SECTION 18
DELAWARE TOWNSHIP
POLK COUNTY
FHWA NO. 700895
LATITUDE: 41.647150°
LONGITUDE: -93.577130°



B_L RAMP B
CURVE DATA

PI STA. 32590+41.99
 $\Delta = 31^{\circ}51'07.08''$ (RT.)
 T = 519.33'
 L = 1,011.78'
 E = 72.65'
 R = 1,820.00'
 PC STA. 32585+22.66
 PT STA. 32595+34.44

OVERHEAD STATION = 32591+08.26, 6.06' RT. (B RAMP B)
OVERHEAD ELEVATION = 960.50
DEPTH OF SUPERSTRUCTURE = 3.83'
UNDERPASS ELEVATION = 932.50
PROVIDED MIN. VERTICAL CLEARANCE = 24.17'

POINTS	NORTH ABUTMENT			SOUTH ABUTMENT		
	STATION	OFFSET	ELEV.	STATION	OFFSET	ELEV.
A1	32590+93.40	24.58' LT.	933.04	32592+14.11	24.58' LT.	943.64
A2	32590+92.54	12.58' RT.	933.00	32592+15.59	12.58' RT.	944.58
B1	32590+40.59	24.58' LT.	954.40	32592+42.84	24.58' LT.	955.44
B2	32590+38.51	12.58' RT.	954.40	32592+44.93	12.58' RT.	955.44
W1	32590+27.48	24.58' LT.	961.12	32592+55.96	24.58' LT.	962.17
W2	32590+25.70	12.58' RT.	959.75	32592+57.74	12.58' RT.	960.81

BERM SLOPE ELEVATIONS REFLECT THE GRADING SURFACE

● POWER POLE - MIDAMERICAN

NOTES:

ALL UNITS ARE IN FEET UNLESS NOTED OTHERWISE.
 * MEASURED TO LOCAL TANGENT OF $\frac{1}{4}$ RAMP B.
 ○ MEASURED PERPENDICULAR TO LOCAL TANGENT OF $\frac{1}{4}$ RAMP B.

DESIGN FOR VARIABLE SKEW RADIUS = 1,820'
**214'-0" X 28'-0" PRETENSIONED
 PRESTRESSED CONCRETE BEAM BRIDGE**
 66'-0" & 71'-0" END SPANS 77'-0" INTERIOR SPAN

STA. 32591+41.72 (B RAMP B)

APRIL, 2022

IOWA DEPARTMENT OF TRANSPORTATION

DESIGN SHEET NO. 4 OF 34 FILE NO. 31046 DESIGN NO. 2118

HDR

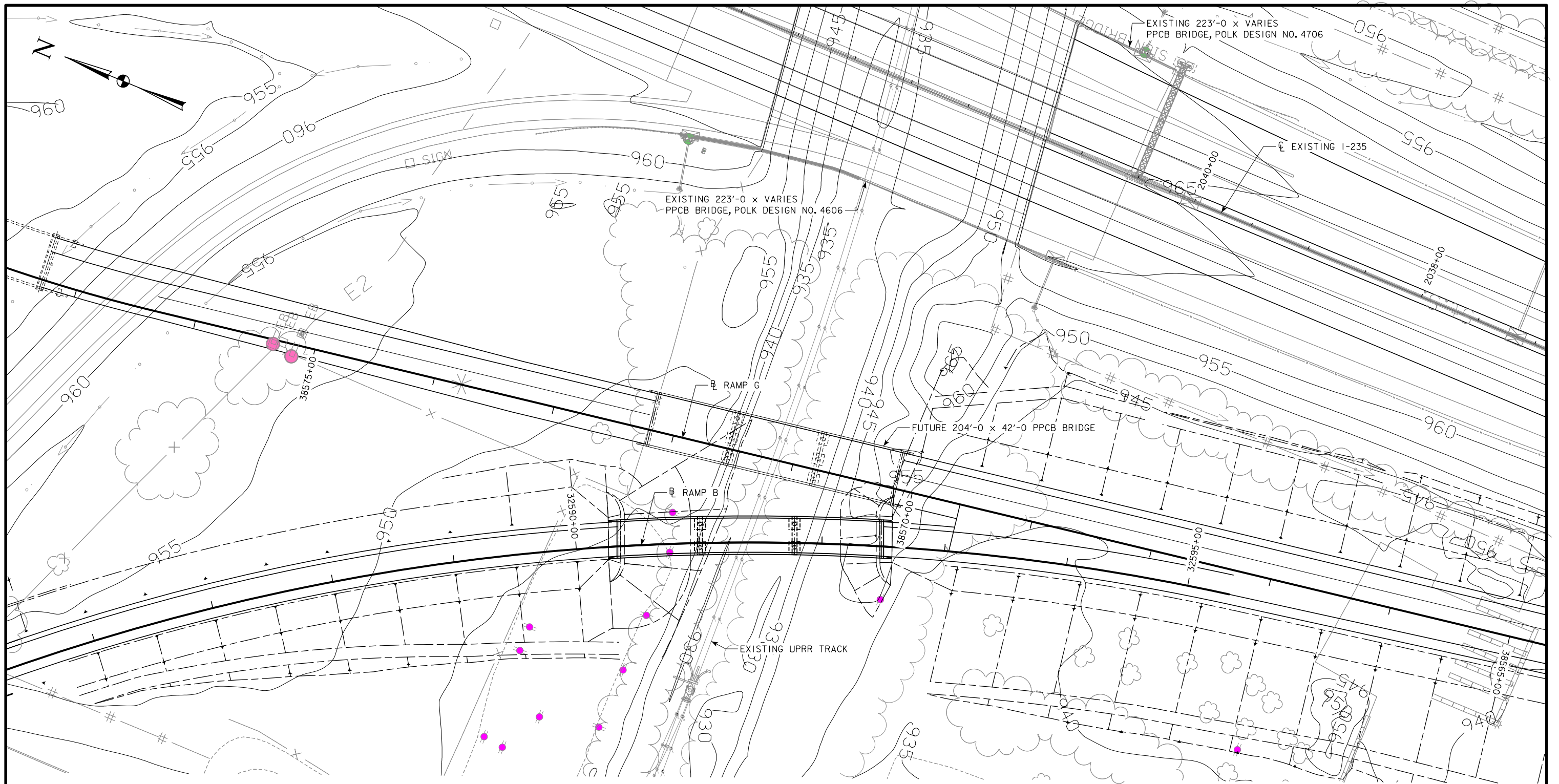
DESIGN TEAM HDR

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POLK COUNTY

PROJECT NUMBER	IM-035-3(203)87--13-77
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SHEET NUMBER 5

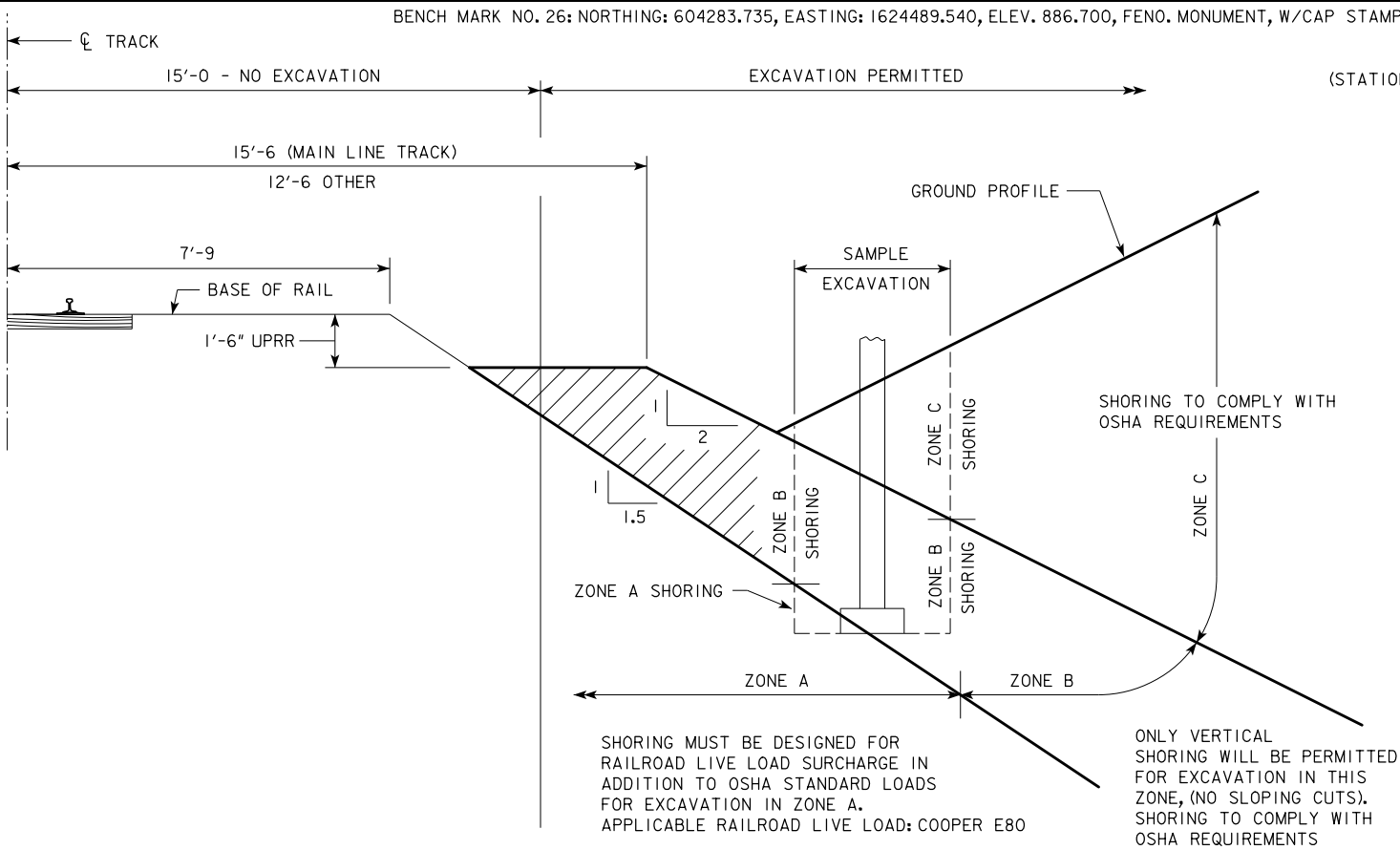


SITE PLAN

DESIGN FOR VARIABLE SKEW		RADIUS = 1,820'
214'-0 X 28'-0 PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE		
66'-0 & 71'-0 END SPANS		77'-0 INTERIOR SPAN
SITUATION PLAN - SITE		
STA. 32591+41.72 (RAMP B)		APRIL, 2022
POLK COUNTY		
IOWA DEPARTMENT OF TRANSPORTATION		
DESIGN SHEET NO. 5	OF 34	FILE NO. 31046
DESIGN NO. 2118		



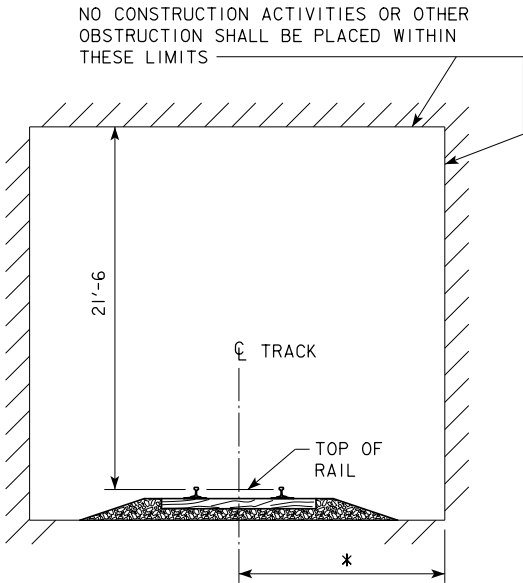
REVISED 09-13 - RAILROAD CHANGED THE MINIMUM VERTICAL CONSTRUCTION CLEARANCE TO 21'-6. DISTANCE FROM MAINLINE TRACK TO EDGE OF EMBANKMENT CHANGED TO 15'-6. ENGLISHMISCELLANEOUSBRIDGES.DGN - 1067 - THIS SHEET ISSUED 12-08.



GENERAL EXCAVATION ZONES

GENERAL SHORING NOTES:

1. ALL DIMENSIONS ARE MEASURED PERPENDICULAR TO TRACK.
2. PRIOR TO COMMENCING ANY WORK, THE CONTRACTOR SHALL SUBMIT FOR APPROVAL BY THE RAILROAD DETAILED PLANS INDICATING THE NATURE AND EXTENT OF THE TRACK PROTECTION SHORING PROPOSED. THE CONTRACTOR SHALL INSTALL THE TEMPORARY SHORING SYSTEM PER THE APPROVED PLANS. DESIGN OF THE TEMPORARY SHORING SYSTEM TO COMPLY WITH GUIDELINES FOR TEMPORARY SHORING.
3. FOR EXCAVATIONS WHICH ENCROACH INTO ZONE A OR B, SHORING PLANS SHALL BE ACCOMPANIED BY DESIGN CALCULATIONS. PLANS AND CALCULATIONS MUST BE SIGNED AND STAMPED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF IOWA.



MINIMUM CONSTRUCTION CLEARANCE ENVELOPE

(NORMAL TO RAILROAD)

* 15'-0" FOR UPRR

NOTE:
UPRR = UNION PACIFIC RAILROAD

MAIN LINE UPRR			
ALIGNMENT: LEFT RAIL		ALIGNMENT: RIGHT RAIL	
STATION	ELEVATION	STATION	ELEVATION
03+77.02	926.94	03+75.41	926.92
04+25.71	927.58	04+23.95	927.52
04+73.02	928.15	04+71.26	928.02
05+16.50	928.79	05+15.05	928.70
05+69.44	929.26	05+67.59	929.35
06+32.30	929.76	06+30.50	929.69
06+81.56	930.13	06+79.77	930.14
07+30.59	930.60	07+28.66	930.61
07+80.50	930.89	07+78.70	930.90
08+03.03	931.08	08+01.10	931.09
08+23.34	931.25	08+21.06	931.26
08+50.47	931.47	08+48.68	931.47
08+85.15	931.73	08+83.34	931.73
09+35.24	932.13	09+33.34	932.17
09+87.12	932.53	09+85.29	932.60
① 10+00.00	932.65	② 10+00.00	932.75
10+40.71	933.03	10+39.06	933.14
10+90.64	933.50	10+88.97	933.61
11+36.11	933.88	11+34.18	934.04
11+86.69	934.34	11+85.24	934.58
12+37.31	934.77	12+35.68	935.00
12+87.08	935.13	12+85.79	935.32
13+38.50	935.32	13+36.96	935.63
13+91.30	935.66	13+89.90	936.01
14+40.28	936.26	14+38.98	936.41
14+63.06	936.44	14+60.58	936.55

① EXISTING TRACK STA. 10+00 = STA. 32591+34.79 @ RAMP B

② EXISTING TRACK STA. 10+00 = STA. 32591+39.99 @ RAMP B

RAILROAD GENERAL NOTES:

1. ALL PERMANENT CLEARANCES SHALL BE VERIFIED BEFORE PROJECT CLOSING.
2. THE CONTRACTOR MUST SUBMIT A PROPOSED METHOD OF EROSION AND SEDIMENT CONTROL AND HAVE THE METHOD APPROVED BY THE RAILROAD.
3. REGARDLESS OF UNDERLYING LAND OWNERSHIP, ALL SHORING SYSTEMS WITHIN RAILROAD RIGHT-OF-WAY OR THAT MAY IMPACT THE RAILROAD'S OPERATIONS AND/OR SUPPORTS THE RAILROAD'S EMBANKMENT SHALL BE DESIGNED AND CONSTRUCTED PER CURRENT RAILROAD GUIDELINES FOR TEMPORARY SHORING.
4. THE CONTRACTOR SUBMIT AND PROVIDE SUFFICIENT SAFETY MEASURES TO PROTECT UNATTENDED EXCAVATIONS TO THE RAILROAD FOR APPROVAL.
5. ALL DEMOLITIONS/REMOVALS WITHIN THE RAILROAD'S RIGHT-OF-WAY AND/OR THAT MAY IMPACT THE RAILROAD'S TRACKS OR OPERATIONS SHALL BE IN COMPLIANCE WITH THE CURRENT RAILROAD'S DEMOLITION GUIDELINES.
6. RAILROAD REQUIREMENTS DO NOT ALLOW WORK WITHIN 50 FEET OF TRACK CENTERLINE WHEN A TRAIN PASSES THE WORK SITE AND ALL PERSONNEL MUST CLEAR THE AREA WITHIN 25 FEET OF THE TRACK CENTERLINE AND SECURE ALL EQUIPMENT.
7. CALL BEFORE YOU DIG. PRIOR TO EXCAVATION, DISRUPTING, OR WORKING ON THE RAILROAD PROPERTY THE CONTRACTOR SHALL LOCATE AND PROTECT UPRR FACILITIES BY CALLING THE UPRR "CALL BEFORE YOU DIG" (CBYD) PHONE NUMBER: 1-800-336-9193.
8. CONSTRUCTION ACTIVITIES, INCLUDING FALSEWORK/FORMWORK, ARE NOT ALLOWED WITHIN THE "MINIMUM CONSTRUCTION CLEARANCE ENVELOPE" AS THEY WOULD OTHERWISE DISRUPT RAILROAD OPERATIONS.
9. RAILROAD REVIEW AND APPROVAL OF SHORING, ERECTION, DEMOLITION, AND FALSEWORK IS REQUIRED. ALLOW A MINIMUM OF FOUR WEEKS FOR THE REVIEW AND APPROVAL OF EACH SUBMITTAL.
10. THE PROPOSED GRADE SEPARATION PROJECT SHALL NOT INCREASE THE QUANTITY AND/OR CHARACTERISTICS OF THE FLOW IN THE RAILROAD'S DITCHES AND/OR DRAINAGE STRUCTURES.
11. THE ELEVATION OF THE EXISTING TOP-OF-RAIL PROFILE SHALL BE VERIFIED BEFORE BEGINNING CONSTRUCTION. ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE RAILROAD PRIOR TO CONSTRUCTION.
12. ERECTION OVER THE RAILROAD'S RIGHT-OF-WAY SHALL BE DESIGNED TO CAUSE NO INTERRUPTION TO THE RAILROAD'S OPERATION, ENABLING THE TRACK(S) TO REMAIN OPEN TO TRAFFIC PER THE RAILROAD'S REQUIREMENTS.
13. ALL CONSTRUCTION PHASING THAT MAY IMPACT THE RAILROAD OPERATIONS SHALL BE DESIGNED TO CAUSE NO INTERRUPTION TO THE RAILROAD'S OPERATION, ENABLING THE TRACK(S) TO REMAIN OPEN TO TRAFFIC PER THE RAILROAD'S REQUIREMENTS.
14. FALSE-WORK CLEARANCES SHALL COMPLY WITH MINIMUM CONSTRUCTION CLEARANCES.
15. FOR RAILROAD COORDINATION PLEASE REFER TO THE RAILROAD COORDINATION REQUIREMENTS AS PART OF SPECIAL PROVISIONS.

DESIGN FOR VARIABLE SKEW RADIUS = 1,820'

**214'-0 X 28'-0 PRETENSIONED
PRESTRESSED CONCRETE BEAM BRIDGE**

66'-0 & 71'-0 END SPANS 77'-0 INTERIOR SPAN

UPRR SHORING DETAILS

STA. 32591+41.72 (@ RAMP B) APRIL, 2022

POLK COUNTY

IOWA DEPARTMENT OF TRANSPORTATION

DESIGN SHEET NO. 6 OF 34 FILE NO. 31046 DESIGN NO. 2118

DESIGN TEAM HDR

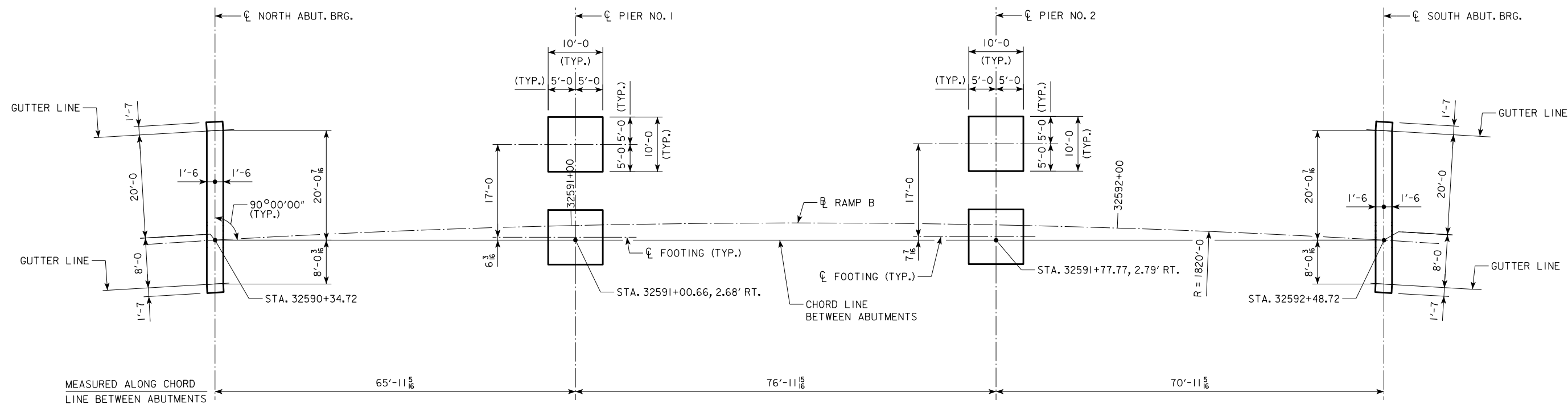
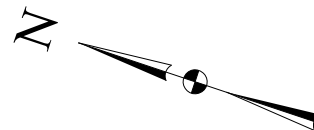
BNSF & UPRR GENERAL NOTES & SHORING

STANDARD SHEET 1067
(MODIFIED)

POLK COUNTY

PROJECT NUMBER IM-035-3(203)87--13-77

SHEET NUMBER **7**



STAKING DIAGRAM

BRIDGE COORDINATES				
LOCATION	CL N. ABUT. BRG.	CL PIER NO. 1	CL PIER NO. 2	CL S. ABUT. BRG.
LEFT EDGE OF DECK	E = 1619312.373 N = 600237.421	E = 1619340.094 N = 600177.530	E = 1619369.700 N = 600106.456	E = 1619394.343 N = 600039.876
CL RAMP B	E = 1619292.404 N = 600229.135	E = 1619320.154 N = 600169.256	E = 1619349.761 N = 600098.182	E = 1619374.374 N = 600031.589
RIGHT EDGE OF DECK	E = 1619283.536 N = 600225.455	E = 1619311.300 N = 600165.582	E = 1619340.907 N = 600094.508	E = 1619365.507 N = 600027.910

NOTE: AN ELECTRONIC FILE CONTAINING THE BRIDGE COORDINATE DATA IS AVAILABLE AS PART OF THE E-FILES SUPPLIED WITH THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL VERIFY THESE COORDINATES WITH THE PROJECT HORIZONTAL CONTROL INFORMATION PROVIDED IN THE ROAD PLANS.

DESIGN FOR VARIABLE SKEW RADIUS = 1,820'

214'-0 X 28'-0 PRETENSIONED
PRESTRESSED CONCRETE BEAM BRIDGE

66'-0 & 71'-0 END SPANS 77'-0 INTERIOR SPAN

STAKING DIAGRAM

STA. 32591+41.72 (CL RAMP B) APRIL, 2022

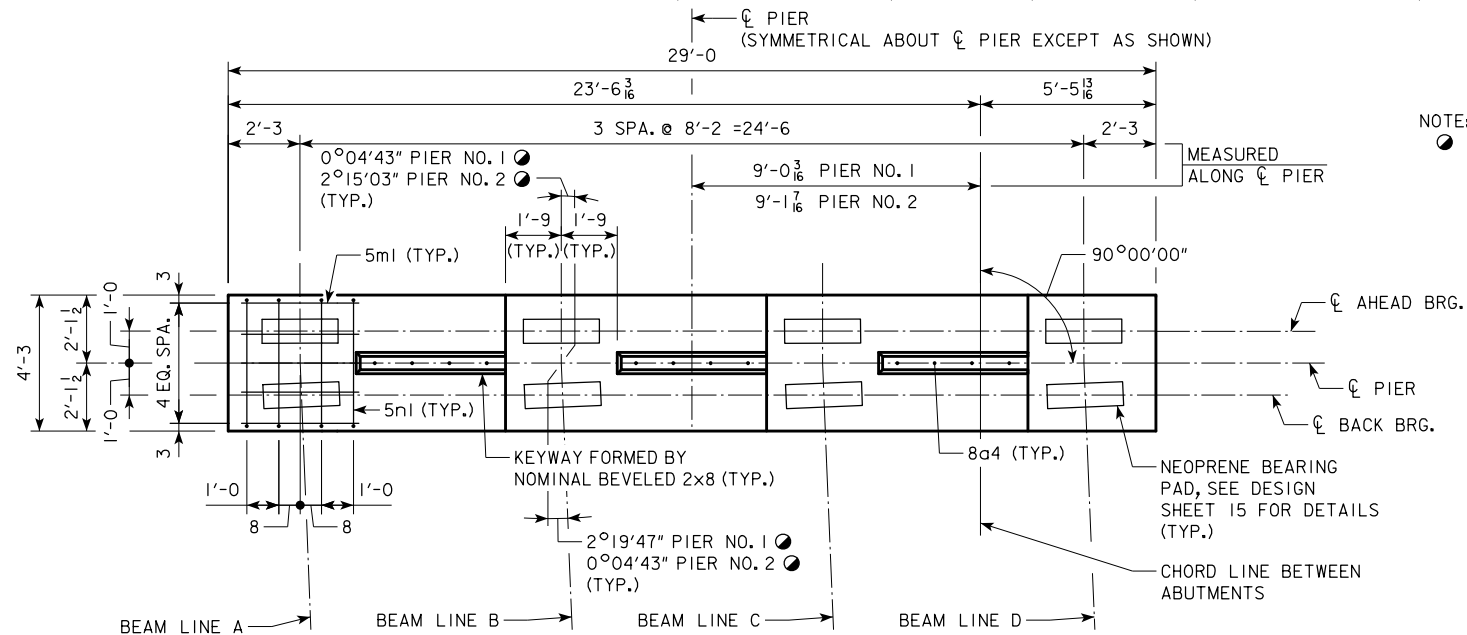
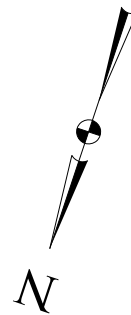
POLK COUNTY

IOWA DEPARTMENT OF TRANSPORTATION

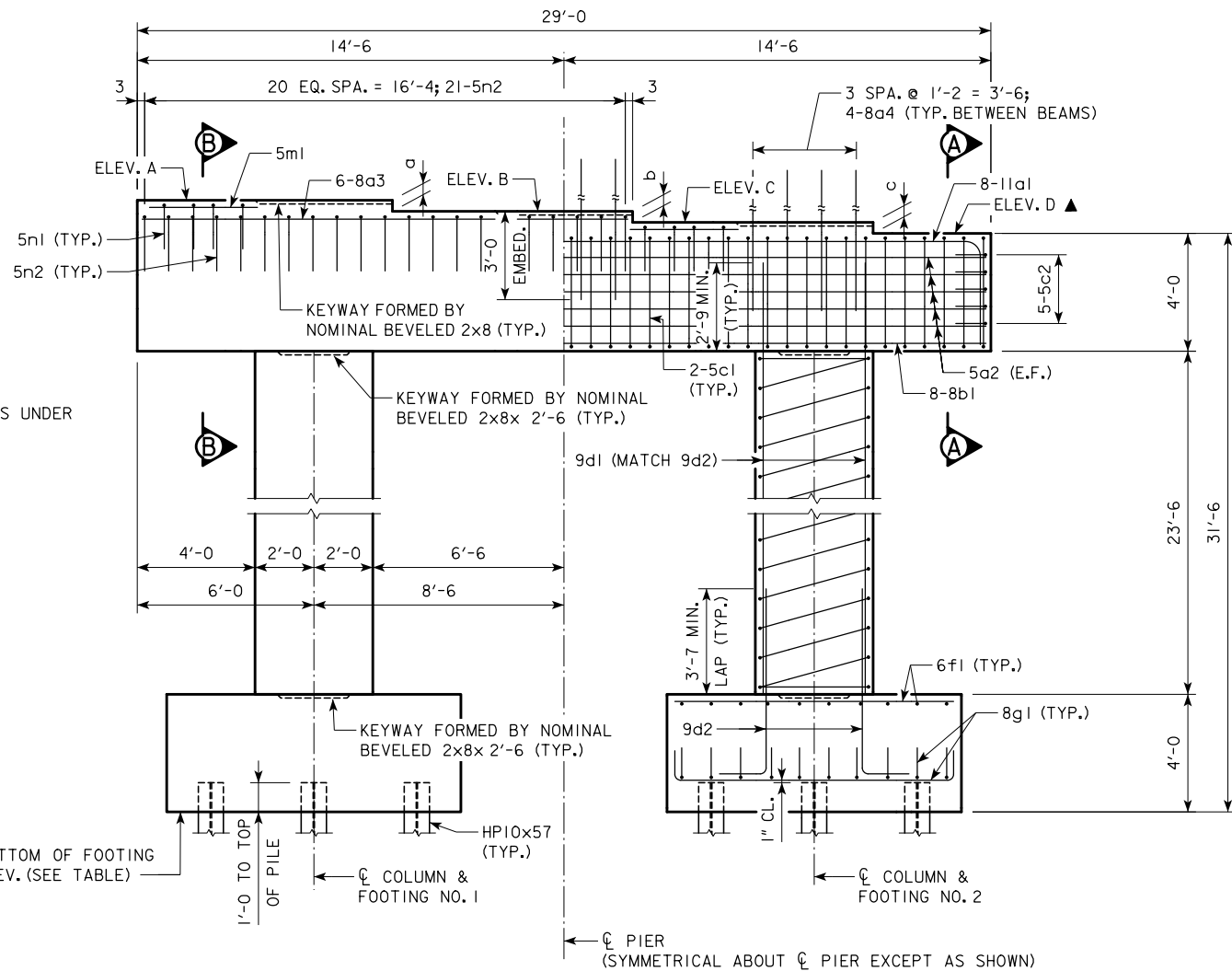
DESIGN SHEET NO. 7 OF 34 FILE NO. 31046 DESIGN NO. 2118



BENCH MARK NO. 26: NORTHING: 604283.735, EASTING: 1624489.540, ELEV. 886.700, FENO. MONUMENT, W/CAP STAMPED "026", 6" DEEP, IN THE NORTH ROW/SHOULDER OF NE 54TH AVE., 9' NORTH OF EDGE AC SLAB; 128' FROM P.I. OF NE 54TH AVE. WITH OF RR CROSSING.

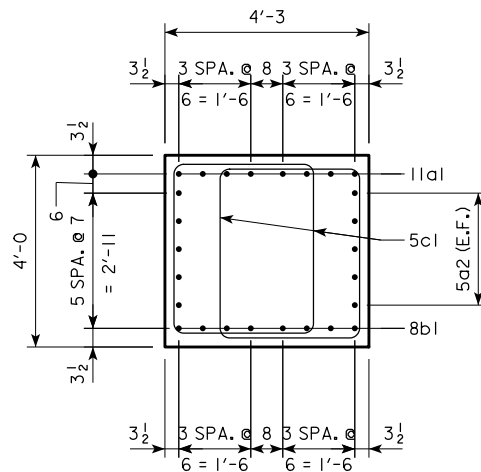


PLAN OF PIER CAP

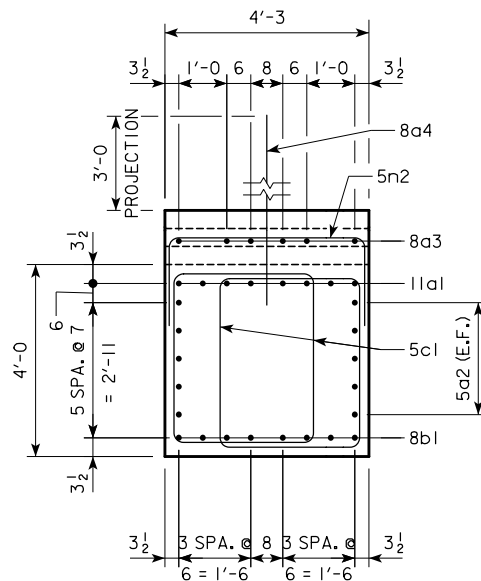


PIER ELEVATION
(LOOKING SOUTH)

NOTE:
● MEASURED PERPENDICULAR TO CL OF PIER.



SECTION A-A



SECTION B-B

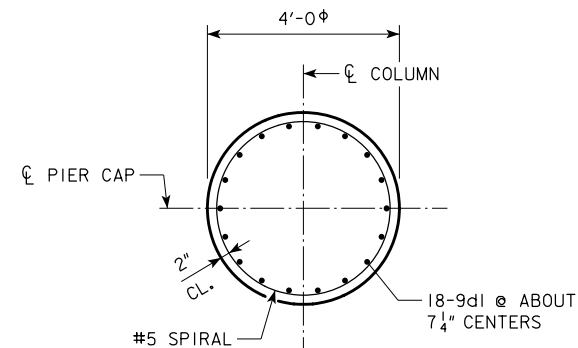
TABLE OF PIER ELEVATIONS

ELEVATION	PIER NO. 1	PIER NO. 2
ELEV. A	957.61	957.99
ELEV. B	957.23	957.61
ELEV. C	956.86	957.24
ELEV. D	▲ 956.48	▲ 956.86
BOTTOM OF FOOTING	924.98	925.36

▲ LOW STEP ELEVATION

TABLE OF PIER STEPS

STEP	PIER NO. 1	PIER NO. 2
a	4 1/2	4 1/2
b	4 1/2	4 1/2
c	4 1/2	4 1/2



TYPICAL SECTION
THRU COLUMN

PIER NOTES:

MINIMUM CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR IS TO BE 2" UNLESS OTHERWISE NOTED OR SHOWN.

SPIRAL REINFORCING IS TO BE NO. 5 BAR WITH 3'-7 3/8" DIAMETER, 12" PITCH WITH 4 EQUALLY SPACED L 7/8 x 7/8 x 1/8 SPACERS PUNCHED TO HOLD SPIRALS. SPIRALS ARE TO HAVE 1 1/2 EXTRA TURNS AT TOP AND BOTTOM COLUMNS.

THE SPIRAL REINFORCING MAY BE SPLICED BY LAPPING 22". THE LENGTH OF THE SPIRAL SHOWN DOES NOT INCLUDE THE LAPPED LENGTH OF THE SPLICES. THE COST OF THE LAPS AT SPLICES IS TO BE INCLUDED IN THE PRICE BID FOR OTHER REINFORCEMENT.

COLUMN TIES SPACED AT 12" CENTERS MAY BE SUBSTITUTED FOR THE SPIRAL REINFORCEMENT. PAYMENT WILL BE BASED ON THE WEIGHT OF SPIRAL REINFORCEMENT. NO ADJUSTMENTS IN REINFORCING STEEL PAY WEIGHT WILL BE ALLOWED. SEE BENT BAR DETAILS FOR SPLICE LAP LENGTH.

DESIGN FOR VARIABLE SKEW RADIUS = 1,820'
**214'-0 X 28'-0 PRETENSIONED
PRESTRESSED CONCRETE BEAM BRIDGE**
66'-0 & 71'-0 END SPANS 77'-0 INTERIOR SPAN

PIER DETAILS

STA. 32591+41.72 (RAMP B)

APRIL, 2022

POLK COUNTY

IOWA DEPARTMENT OF TRANSPORTATION

DESIGN SHEET NO. 8 OF 34 FILE NO. 31046 DESIGN NO. 2118



PIER PILE NOTES:

THE CONTRACT LENGTH OF 105 FEET FOR THE PIER NO.1 PILES, AND 100 FEET FOR THE PIER NO.2 PILES IS BASED ON A COHESIVE SOIL CLASSIFICATION, A TOTAL FACTORED AXIAL LOAD PER PILE (PU) OF 204 KIPS, AND A GEOTECHNICAL RESISTANCE FACTOR (PHI) OF 0.65. THE FACTORED AXIAL LOAD INCLUDES A FACTORED DEAD LOAD OF 6 KIPS PER PILE TO ACCOUNT FOR FUTURE RAILROAD PIER PROTECTION.

THE NOMINAL AXIAL BEARING RESISTANCE FOR CONSTRUCTION CONTROL WAS DETERMINED FROM A COHESIVE SOIL CLASSIFICATION AND A GEOTECHNICAL RESISTANCE FACTOR (PHI) OF 0.76. PILES ARE ASSUMED TO BE DRIVEN FROM A START ELEVATION AT THE BOTTOM OF FOOTING.

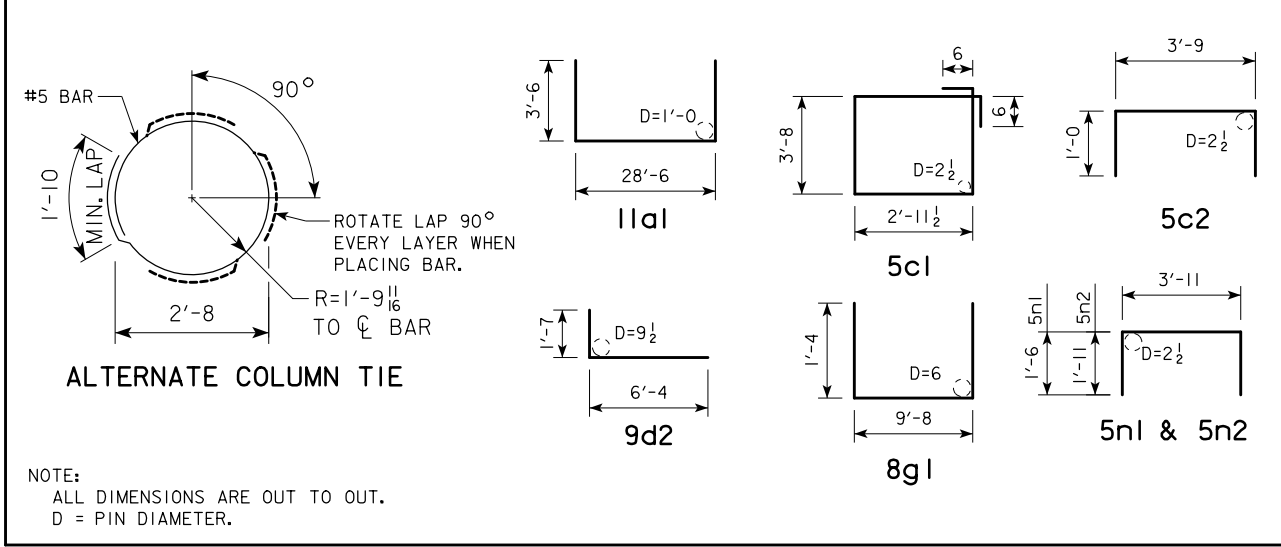
THE REQUIRED NOMINAL AXIAL BEARING RESISTANCE FOR PIER PILES IS 134 TONS AT END OF DRIVE. IF RETAPS ARE NECESSARY TO ACHIEVE BEARING THE REQUIRED NOMINAL AXIAL BEARING RESISTANCE IS 157 TONS AT ONE-DAY RETAP. THE PILE CONTRACT LENGTH SHALL BE DRIVEN AS PER PLAN UNLESS PILES REACH REFUSAL. CONSTRUCTION CONTROL REQUIRES A WEAP ANALYSIS WITH BEARING GRAPH.

PILE DIMENSIONS ARE AT BOTTOM OF FOOTING. BATTER PILES 1:4 IN DIRECTION SHOWN.

ALL BATTERED PILE SHALL BE TRIMMED TO A HORIZONTAL LINE TO AID IN THE PLACEMENT OF REINFORCING.

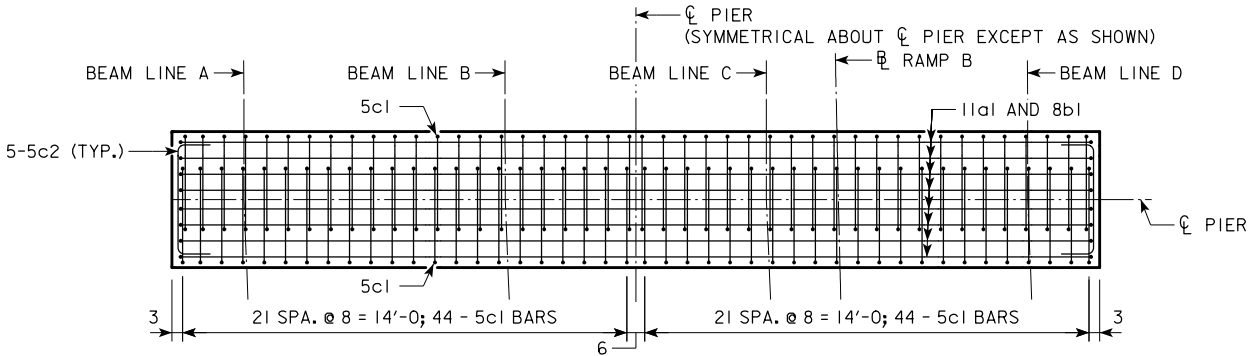
7 - HPI0x57 STEEL BEARING PILING ARE REQUIRED FOR EACH FOOTING AT PIER NOS. 1 AND 2.

BENT BAR DETAILS



REINFORCING BAR LIST - ONE PIER

BAR	LOCATION	SHAPE	NO.	LENGTH	WEIGHT
11a1	CAP, TOP, LONGITUDINAL	U	8	35'-6"	1,509
5a2	CAP, SIDE, LONGITUDINAL	—	10	28'-8"	299
8a3	CAP, TOP, LONGITUDINAL	—	6	16'-6"	264
8a4	CAP TO DIAPHRAGM DOWEL	—	12	6'-0"	192
8b1	CAP, BOTTOM, LONGITUDINAL	—	8	28'-8"	612
5c1	CAP, HOOPS	□	88	14'-3"	1,308
5c2	CAP, ENDS	U	10	5'-9"	60
9d1	COLUMN, VERTICAL	—	36	26'-3"	3,213
9d2	FOOTING TO COLUMN DOWEL	—	36	7'-11"	969
6f1	FOOTING, TOP	—	40	9'-8"	581
8g1	FOOTING, BOTTOM	U	40	12'-4"	1,317
5m1	CAP, PEDESTAL, LONGITUDINAL	—	10	3'-8"	38
5n1	CAP, PEDESTAL, TRANSVERSE	U	8	6'-11"	58
5n2	CAP, PEDESTAL, TRANSVERSE	U	21	7'-9"	170
#5	COLUMN SPIRAL	WW	2	298'-2"	622
	COLUMN SPIRAL SPACERS L 7/8 x 7/8 x 1/8	—	8	23'-2"	130
REINFORCING STEEL - TOTAL (LBS.)					11,342



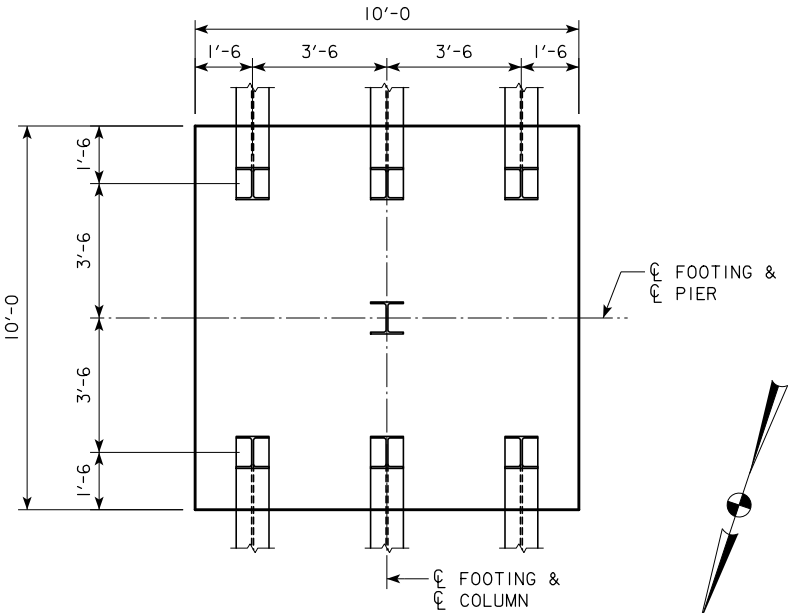
NOTE:
8a3, 5m1, 5n1 AND 5n2 BARS
NOT SHOWN FOR CLARITY.

PIER CAP REINFORCING PLAN

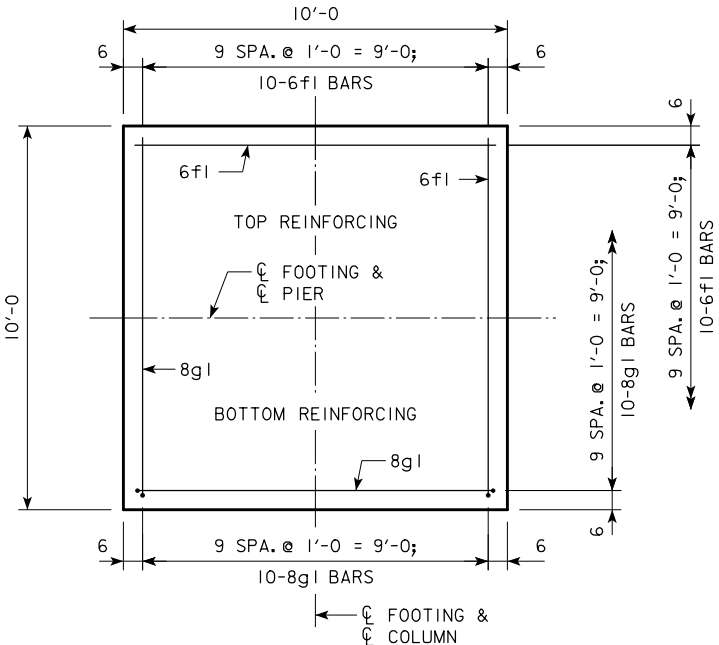
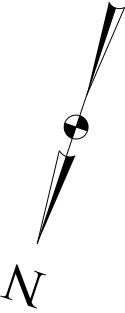
CONCRETE PLACEMENT QUANTITIES

LOCATION	PIER NO. 1	PIER NO. 2	QUANTITY
CAP & STEPS (HIGH PERFORMANCE CONCRETE)	21.3	21.3	42.6
COLUMNS (HIGH PERFORMANCE CONCRETE)	21.9	21.9	43.8
FOOTINGS	29.6	29.6	59.2
TOTAL CU. YDS.	72.8	72.8	145.6

NOTE:
CONCRETE AND REINFORCING STEEL QUANTITIES ARE INCLUDED ON THE SUMMARY OF ITEMIZED QUANTITIES SHEET.



PILE LAYOUT
(TYP. FOR ALL PIER FOOTINGS)



REINFORCING LAYOUT
(TYP. FOR ALL PIER FOOTINGS)

DESIGN FOR VARIABLE SKEW RADIUS = 1,820'

**214'-0 X 28'-0 PRETENSIONED
PRESTRESSED CONCRETE BEAM BRIDGE**

66'-0 & 71'-0 END SPANS 77'-0 INTERIOR SPAN

PIER DETAILS

STA. 32591+41.72 (CL RAMP B) APRIL, 2022

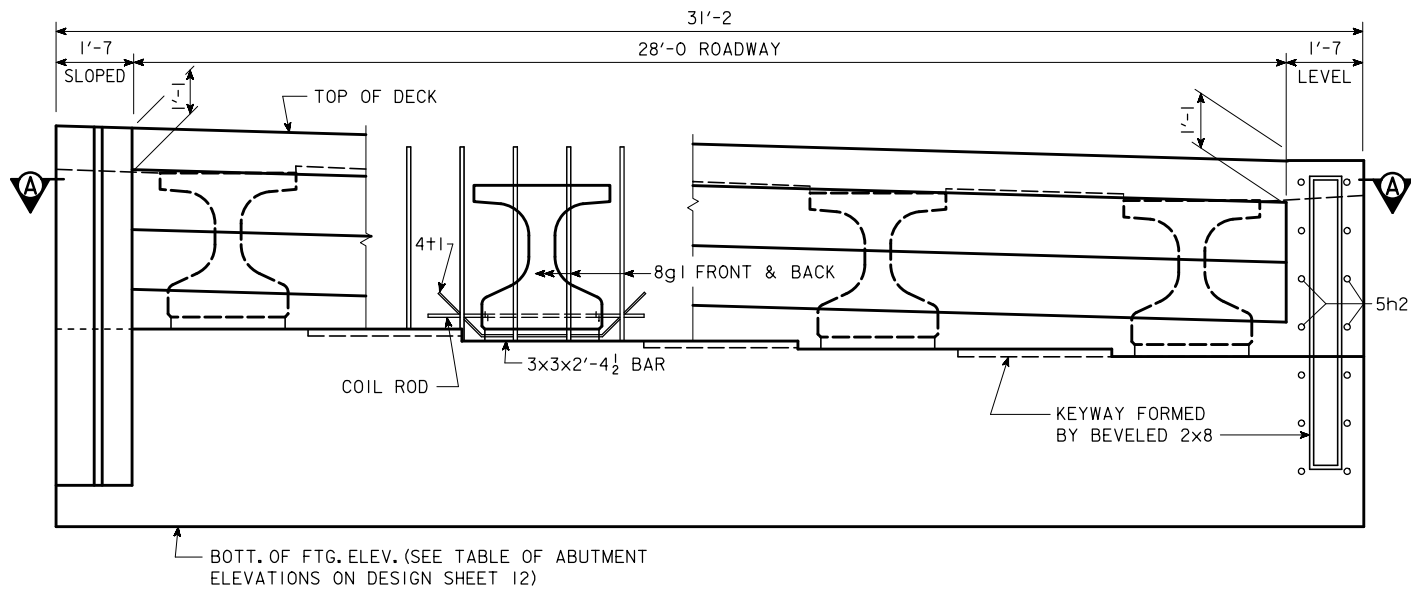
POLK COUNTY

IOWA DEPARTMENT OF TRANSPORTATION

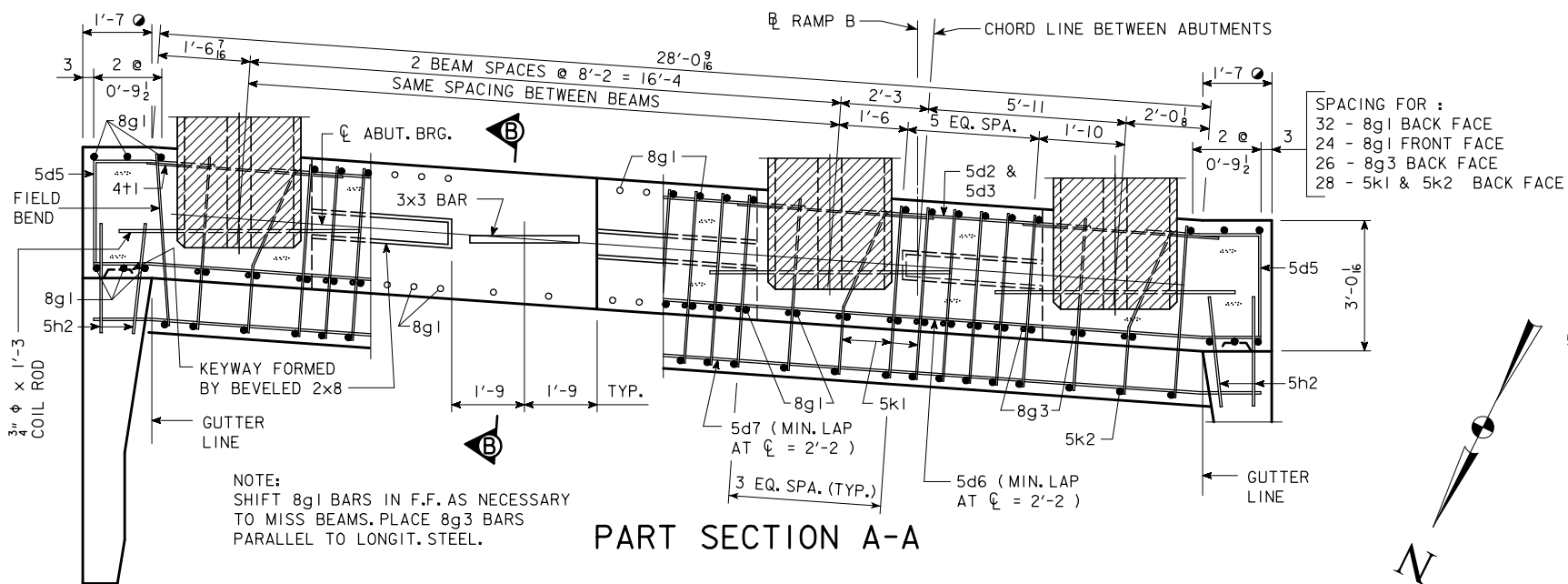
DESIGN SHEET NO. 9 OF 34 FILE NO. 31046 DESIGN NO. 2118



CORRECTION 04-14 - ADDED CONCRETE QUANTITY TABLE & REFERRAL NOTE TO SUMMARY QUANTITY SHEET. REMOVED DESIGN BEARING NOTE FOR ABUT. PILING FROM ABUTMENT NOTES.
ENGLISHBTINTEGRALBRIDGES.DGN - 2079-BTB - THIS SHEET ISSUED 02-08.



PART REAR ELEVATION AT NORTH ABUTMENT



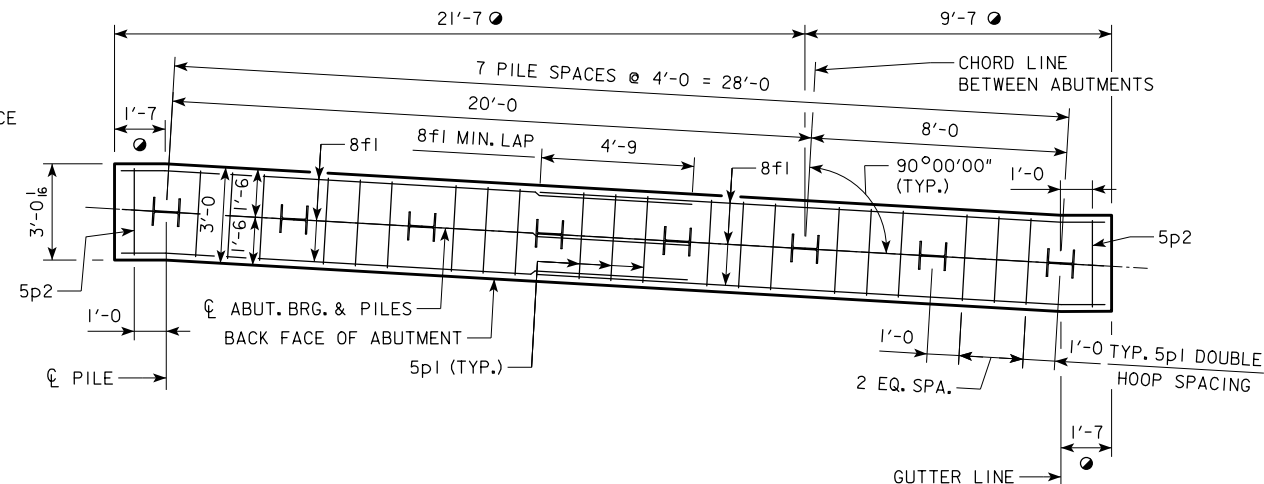
PART SECTION A-A

NOTE: PLACE 5h2 BAR AT 1:6 SLOPE TO MATCH TRAFFIC SIDE OF ABUTMENT WING FACE (BOTH SIDES TYPICAL).

NORTH ABUTMENT PILE NOTES:

THE CONTRACT LENGTH OF 75 FEET FOR THE NORTH ABUTMENT PILES IS BASED ON A COHESIVE SOIL CLASSIFICATION, A TOTAL FACTORED AXIAL LOAD PER PILE (PU) OF 113 KIPS, AND A GEOTECHNICAL RESISTANCE FACTOR (PHI) OF 0.65. THE NOMINAL AXIAL BEARING RESISTANCE FOR CONSTRUCTION CONTROL WAS DETERMINED FROM A COHESIVE SOIL CLASSIFICATION AND A GEOTECHNICAL RESISTANCE FACTOR (PHI) OF 0.78. PILES ARE ASSUMED TO BE DRIVEN FROM A START ELEVATION AT THE BOTTOM OF PREBORE. THE REQUIRED NOMINAL AXIAL BEARING RESISTANCE FOR NORTH ABUTMENT PILES IS 73 TONS AT END OF DRIVE. IF RETAPS ARE NECESSARY TO ACHIEVE BEARING THE REQUIRED NOMINAL AXIAL BEARING RESISTANCE IS 87 TONS AT ONE-DAY RETAP. THE PILE CONTRACT LENGTH SHALL BE DRIVEN AS PER PLAN UNLESS PILES REACH REFUSAL. CONSTRUCTION CONTROL REQUIRES A WEAP ANALYSIS WITH BEARING GRAPH.

NOTES:
8 - HP10x57 STEEL BEARING PILING REQUIRED AT THE NORTH ABUTMENT.
FOR PART SECTION B-B AND ABUTMENT STEP DIAGRAM, SEE DESIGN SHEET 12.
MEASURED RADIAL TO RAMP B.



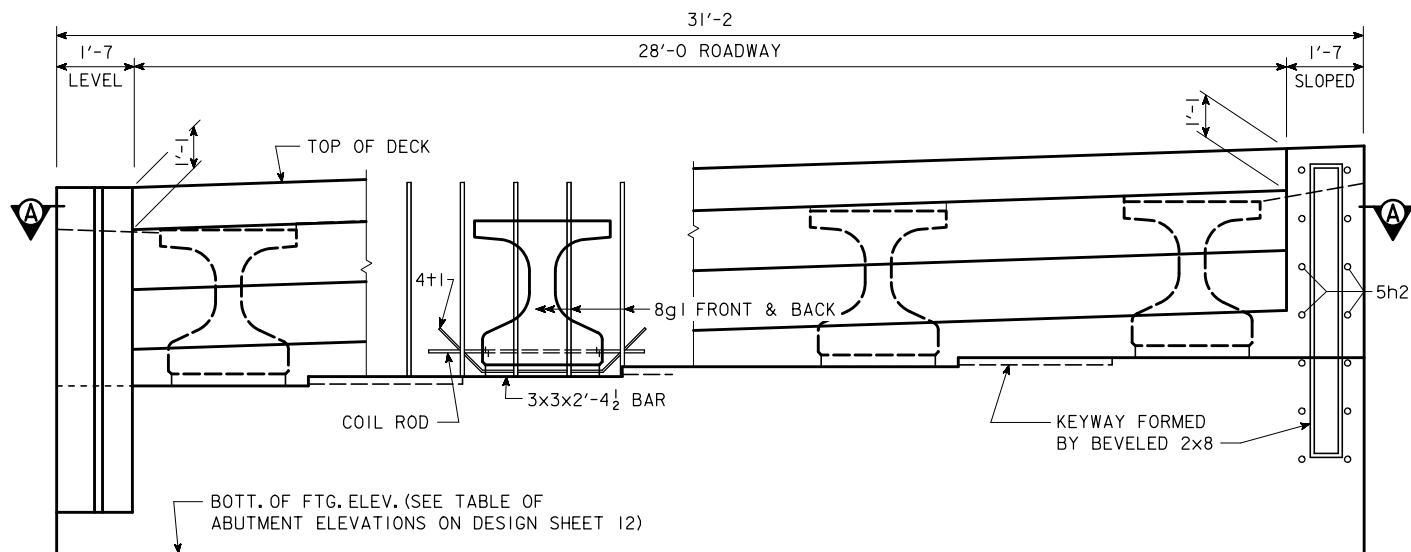
NORTH ABUTMENT PILE PLAN

ABUTMENT NOTES:

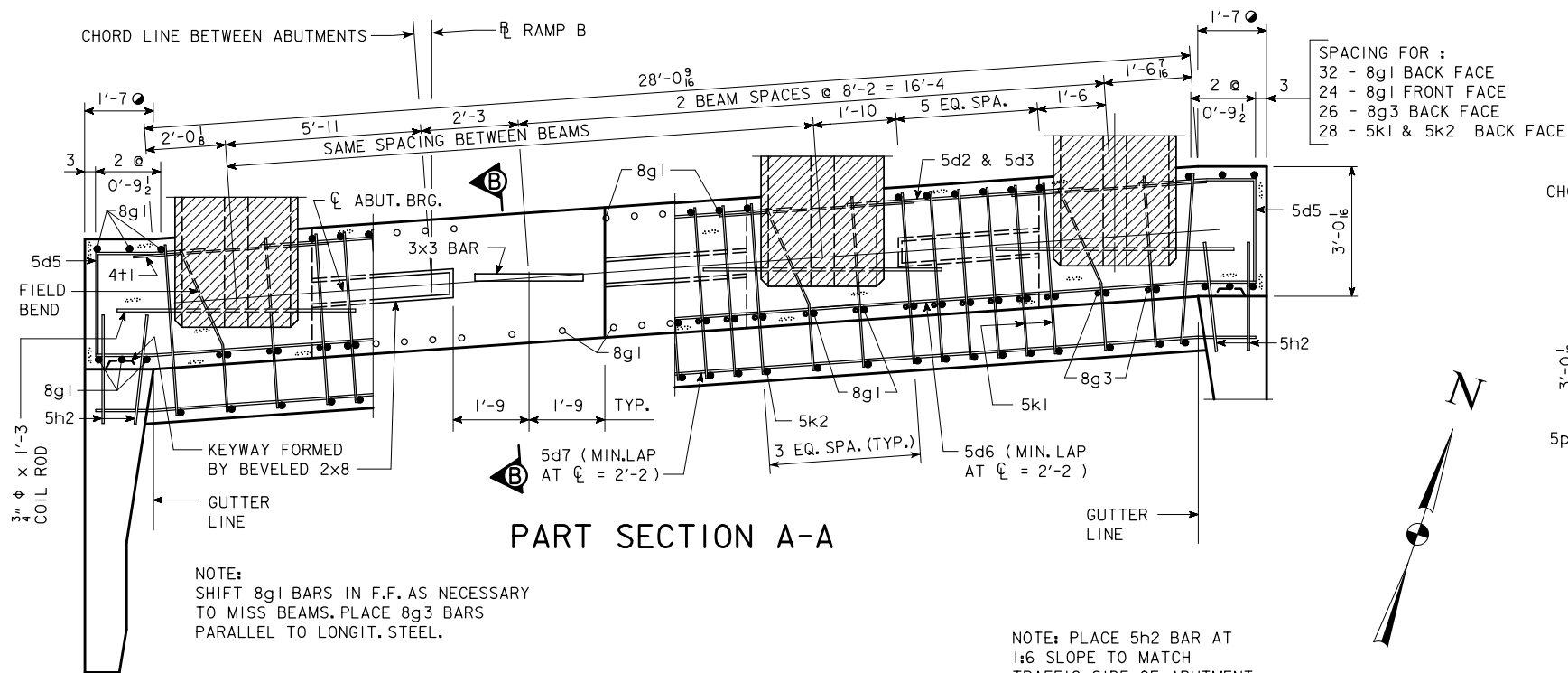
MINIMUM CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR IS TO BE 2" UNLESS OTHERWISE NOTED OR SHOWN. IF NECESSARY TO PREVENT DAMAGE TO THE END OF THE BRIDGE DECK OR BACKWALL FROM CONSTRUCTION EQUIPMENT, AN APPROPRIATE METHOD OF PROTECTION APPROVED BY THE ENGINEER SHALL BE PROVIDED BY THE BRIDGE CONTRACTOR AT NO EXTRA COST TO THE STATE. BARRIER RAIL NOT SHOWN IN DETAILS.

DESIGN FOR VARIABLE SKEW	RADIUS = 1,820'
214'-0" X 28'-0" PRETENSIONED	
PRESTRESSED CONCRETE BEAM BRIDGE	
66'-0" & 71'-0" END SPANS	77'-0" INTERIOR SPAN
NORTH ABUTMENT DETAILS	
STA. 32591+41.72 (RAMP B)	APRIL, 2022
POLK COUNTY	
IOWA DEPARTMENT OF TRANSPORTATION	
DESIGN SHEET NO. 10 OF 34	FILE NO. 31046
DESIGN NO. 2118	

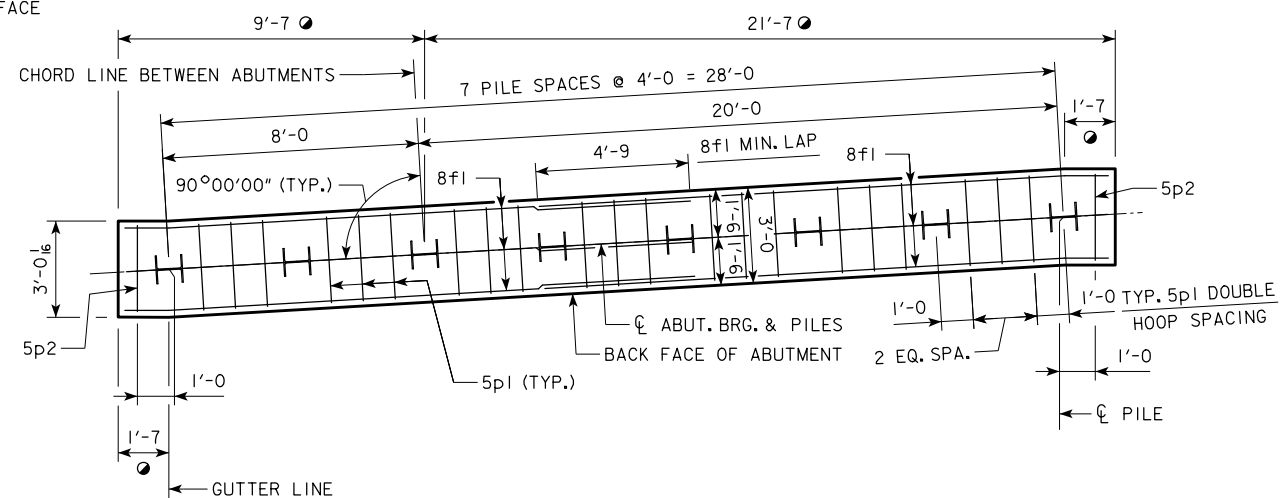
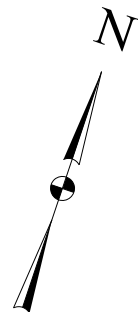
CORRECTION 04-14 - ADDED CONCRETE QUANTITY TABLE & REFERRAL NOTE TO SUMMARY QUANTITY SHEET. REMOVED DESIGN BEARING NOTE FOR ABUT. PILING FROM ABUTMENT NOTES.
ENGLISHBTINTEGRALBRIDGES.DGN - 2080-BTB - THIS SHEET ISSUED 02-08.



PART REAR ELEVATION AT SOUTH ABUTMENT



PART SECTION A-A



SOUTH ABUTMENT PILE PLAN

SOUTH ABUTMENT PILE NOTES:

THE CONTRACT LENGTH OF 75 FEET FOR THE SOUTH ABUTMENT PILES IS BASED ON A COHESIVE SOIL CLASSIFICATION, A TOTAL FACTORED AXIAL LOAD PER PILE (PU) OF 113 KIPS, AND A GEOTECHNICAL RESISTANCE FACTOR (PHI) OF 0.65.

THE NOMINAL AXIAL BEARING RESISTANCE FOR CONSTRUCTION CONTROL WAS DETERMINED FROM A COHESIVE SOIL CLASSIFICATION AND A GEOTECHNICAL RESISTANCE FACTOR (PHI) OF 0.78. PILES ARE ASSUMED TO BE DRIVEN FROM A START ELEVATION AT THE BOTTOM OF PREBORE.

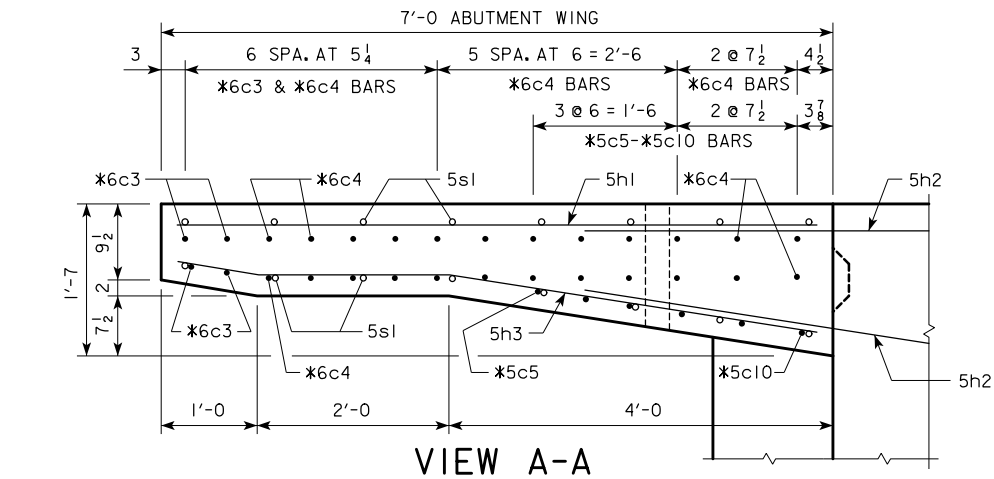
THE REQUIRED NOMINAL AXIAL BEARING RESISTANCE FOR SOUTH ABUTMENT PILES IS 73 TONS AT END OF DRIVE. IF RETAPS ARE NECESSARY TO ACHIEVE BEARING THE REQUIRED NOMINAL AXIAL BEARING RESISTANCE IS 87 TONS AT ONE-DAY RETAP. THE PILE CONTRACT LENGTH SHALL BE DRIVEN AS PER PLAN UNLESS PILES REACH REFUSAL. CONSTRUCTION CONTROL REQUIRES A WEAP ANALYSIS WITH BEARING GRAPH.

NOTES:

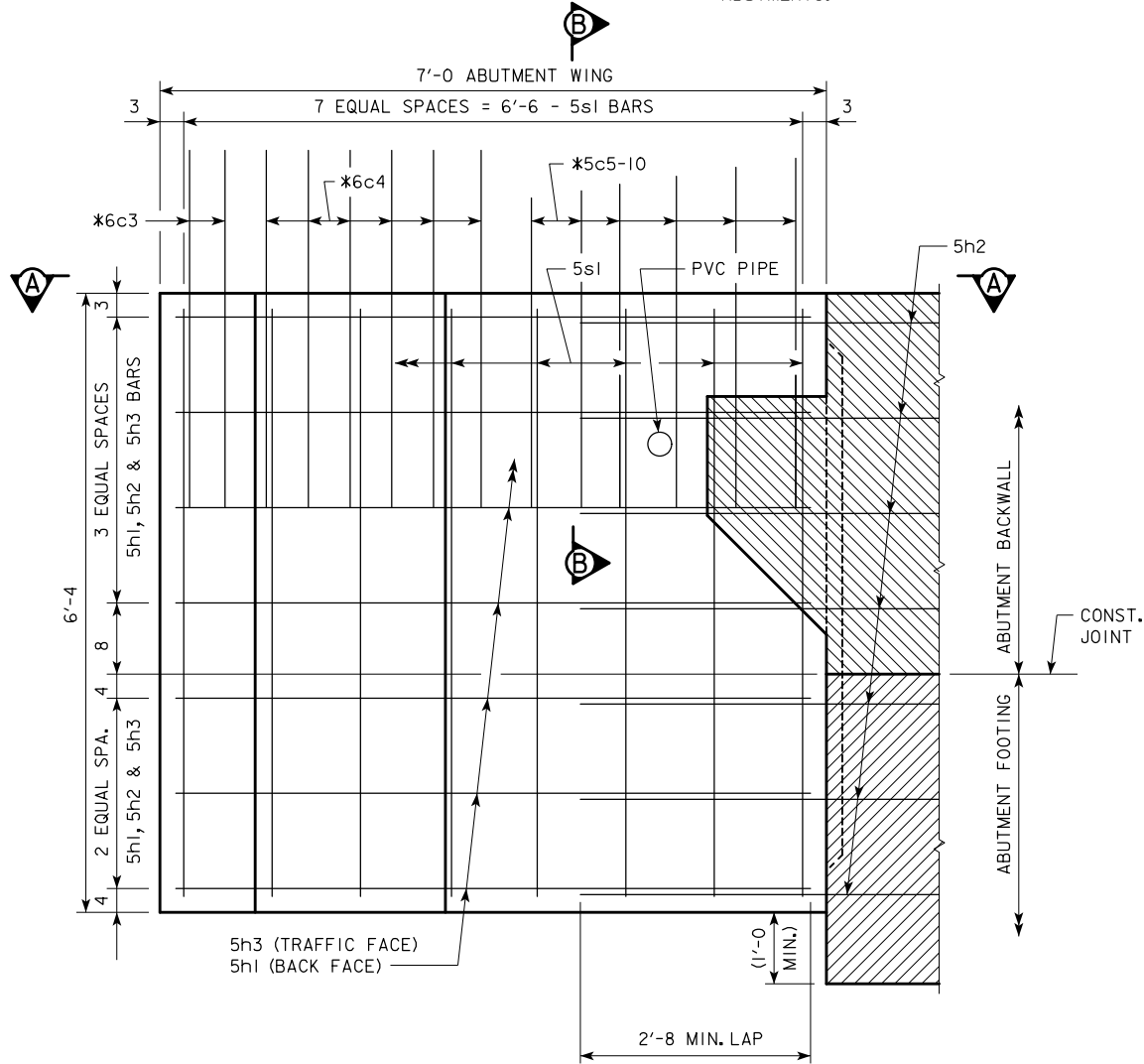
- FOR ABUTMENT NOTES, SEE DESIGN SHEET 10.
- 8 - HP10x57 STEEL BEARING PILING REQUIRED AT THE SOUTH ABUTMENT.
- FOR PART SECTION B-B AND ABUTMENT STEP DIAGRAM, SEE DESIGN SHEET 12.
- MEASURED RADIAL TO CL RAMP B.

DESIGN FOR VARIABLE SKEW		RADIUS = 1,820'	
214'-0 X 28'-0 PRETENSIONED			
PRESTRESSED CONCRETE BEAM BRIDGE			
66'-0 & 71'-0 END SPANS		77'-0 INTERIOR SPAN	
SOUTH ABUTMENT DETAILS			
STA. 32591+41.72 (RAMP B)		APRIL, 2022	
POLK COUNTY			
IOWA DEPARTMENT OF TRANSPORTATION			
DESIGN SHEET NO. 11 OF 34		FILE NO. 31046	
		DESIGN NO. 2118	

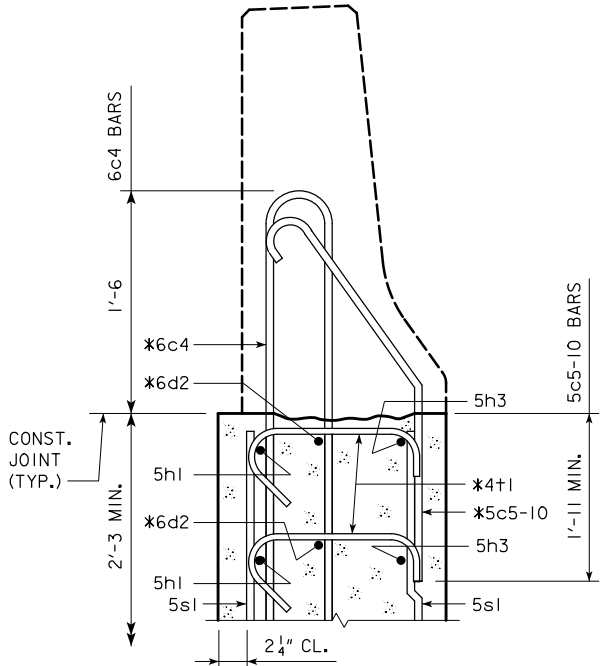
CORRECTION 04-14 - ADDED REFERRAL NOTE TO SUMMARY QUANTITIES SHEET.
ENGLISHMISCELLANEOUSBRIDGES.DGN - 2111 - THIS SHEET ISSUED 02-08.



NOTE:
PLUG 3"Ø PVC PIPE WITH
EXPANDING FOAM PRIOR
TO BACKFILLING BEHIND
ABUTMENTS.



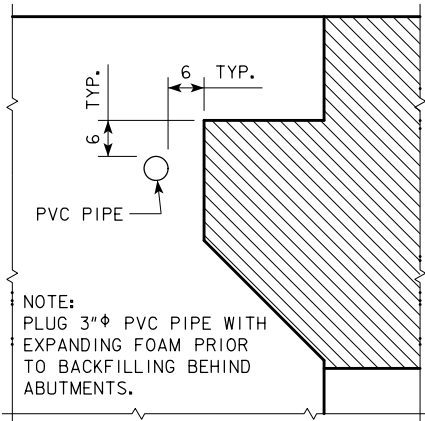
ABUTMENT WING - ELEVATION VIEW



SECTION B-B

* BARRIER RAIL END SECTION
BARS TO BE PLACED WITH
ABUTMENT WING.

SEE BARRIER RAIL END SECTION
SHEET IN THESE PLANS FOR
DETAILS OF REINFORCING BARS
6c3, 6c4, 5c5-10, 6d2 & 4+1.



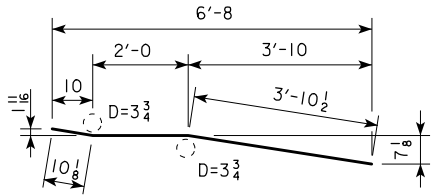
PVC PIPE LOCATION

REINFORCING BAR LIST - ONE ABUT. WING

BAR	LOCATION	SHAPE	NO.	LENGTH	WEIGHT
5h1	HORIZONTAL BACK FACE		7	6'-8	49
5h3	HORIZONTAL TRAFFIC FACE		7	6'-9	49
5sl	VERTICAL BOTH FACES		16	6'-0	100

REINFORCING STEEL EPOXY COATED - TOTAL (LBS.)

198



5h3

NOTE: ALL DIMENSIONS ARE OUT TO OUT. D = PIN DIAMETER.

BENT BAR DETAILS

CONCRETE PLACEMENT SUMMARY

CONCRETE	TOTAL
ONE ABUTMENT WING	1.9
TOTAL (CU. YDS.)	1.9

NOTE:

CONCRETE AND REINFORCING STEEL QUANTITIES ARE INCLUDED ON THE
SUMMARY QUANTITIES SHEET.

DESIGN FOR VARIABLE SKEW RADIUS = 1,820'

**214'-0 X 28'-0 PRETENSIONED
PRESTRESSED CONCRETE BEAM BRIDGE**

66'-0 & 71'-0 END SPANS 77'-0 INTERIOR SPAN

ABUTMENT WING DETAILS

STA. 32591+41.72 (RAMP B) APRIL, 2022

POLK COUNTY

IOWA DEPARTMENT OF TRANSPORTATION

DESIGN SHEET NO. 13 OF 34 FILE NO. 31046 DESIGN NO. 2118

THE BRIDGE DECK DECK AS SHOWN INCLUDES $3\frac{3}{4}$ " INTEGRAL WEARING SURFACE. THE PIER AND ABUTMENT DIAPHRAGM CONCRETE IS TO BE PLACED MONOLITHICALLY WITH THE BRIDGE DECK.

COST OF ALL RESILIENT JOINT FILLER MATERIAL IS TO BE INCLUDED IN THE PRICE BID FOR "HIGH PERFORMANCE STRUCTURAL CONCRETE".

ALL BEAMS ARE TO BE SET VERTICAL.

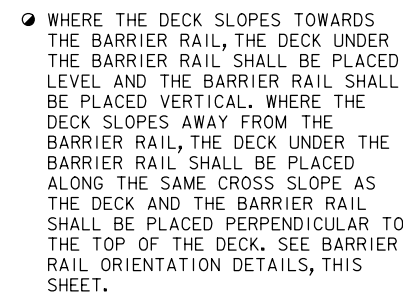
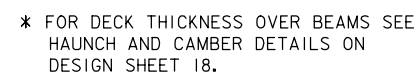
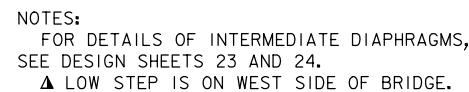
FORMS FOR THE DECK AND BARRIER RAIL ARE TO BE SUPPORTED BY THE PRESTRESSED CONCRETE BEAMS.

CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR SHALL BE 2 INCHES UNLESS OTHERWISE NOTED OR SHOWN.

ALL DECK AND DIAPHRAGM REINFORCING IS TO BE WIRED IN PLACE AND ADEQUATELY SUPPORTED BEFORE CONCRETE IS PLACED.

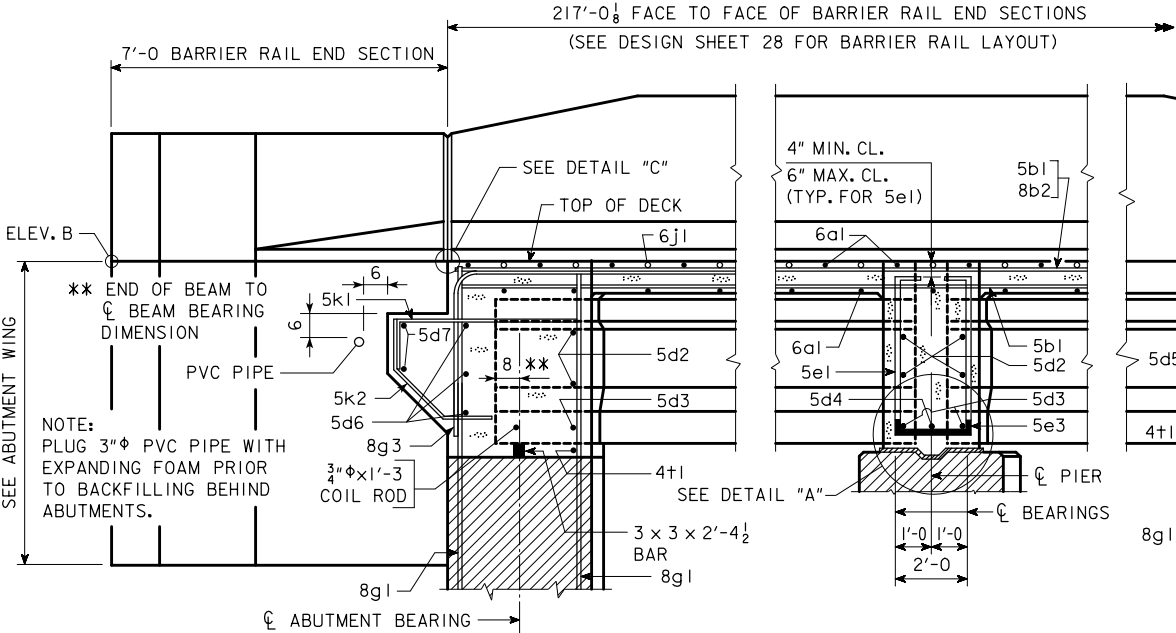
TOP TRANSVERSE REINFORCING STEEL IS TO BE PARALLEL TO AND $2\frac{3}{4}$ " CLEAR BELOW TOP OF DECK. BOTTOM TRANSVERSE REINFORCING STEEL IS TO BE PARALLEL TO AND $1\frac{1}{4}$ " CLEAR ABOVE BOTTOM OF DECK. TOP AND BOTTOM REINFORCING STEEL IS TO BE SUPPORTED BY INDIVIDUAL BAR CHAIRS SPACED AT NOT MORE THAN 3'-0" CENTERS LONGITUDINALLY AND TRANSVERSELY, OR BY CONTINUOUS ROWS OF BAR HIGH CHAIRS OR DECK BOLSTERS SPACED 4'-0" APART. I.M. 451.01 REQUIREMENTS SHALL APPLY FOR BAR CHAIRS, BAR HIGH CHAIRS, AND DECK BOLSTERS.

COST OF BEARING MATERIAL IS TO BE INCLUDED IN THE PRICE BID FOR "PRETENSIONED PRESTRESSED CONCRETE BEAMS".

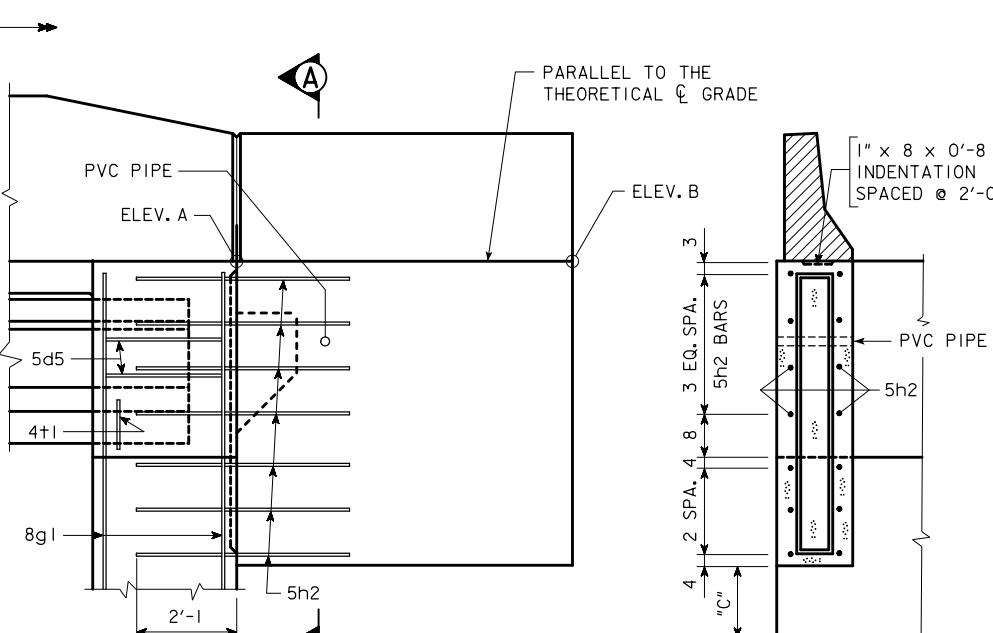


HDR

BENCH MARK NO. 26: NORTHING: 604283.735, EASTING: 1624489.540, ELEV. 886.700, FENO. MONUMENT, W/CAP STAMPED "026", 6" DEEP, IN THE NORTH ROW/SHOULDER OF NE 54TH AVE., 9' NORTH OF EDGE AC SLAB; 128' FROM P.I. OF NE 54TH AVE. WITH OF RR CROSSING.

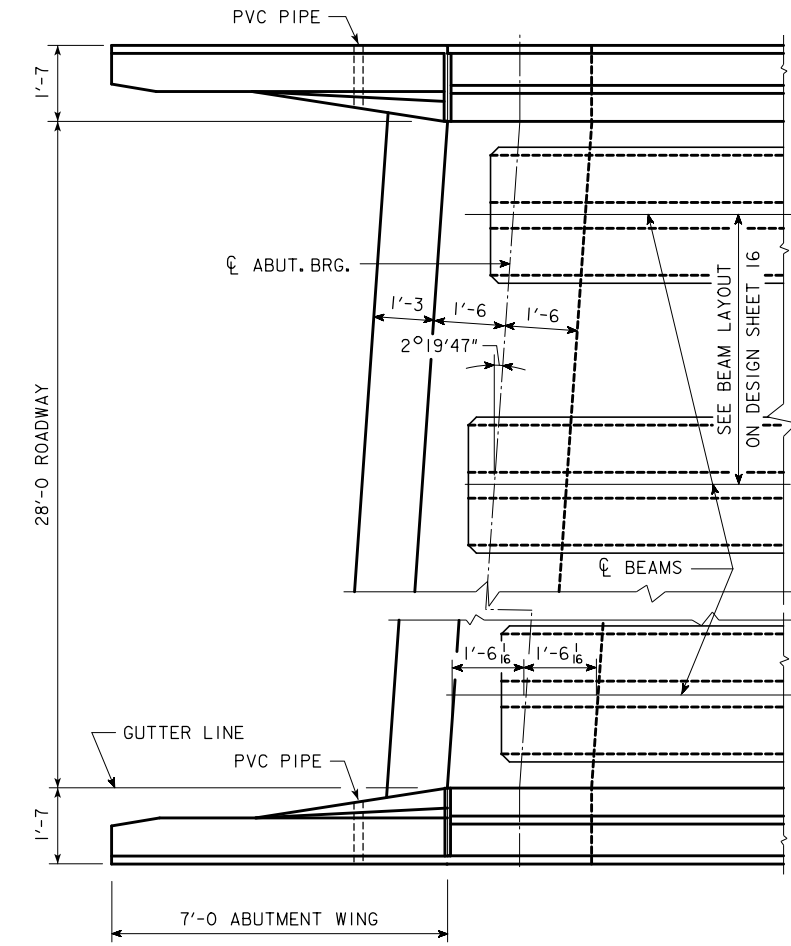
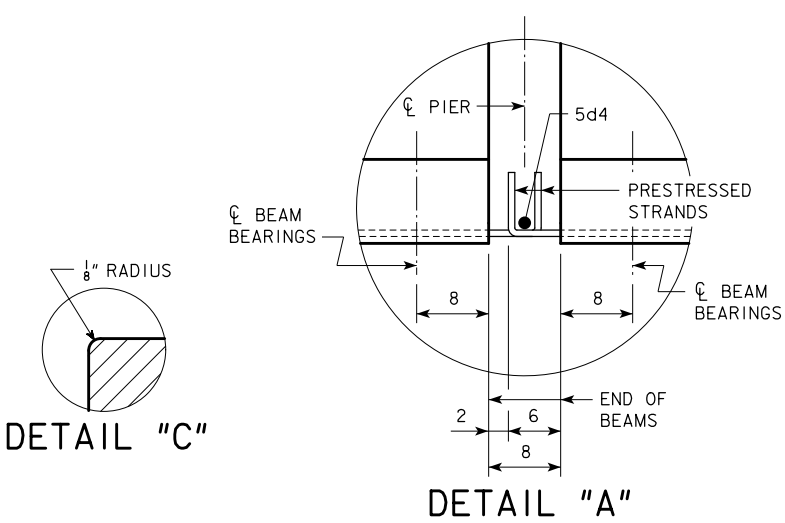


PART LONGITUDINAL SECTION NEAR GUTTER
(FOR DETAILS OF INTERMEDIATE DIAPHRAGM SEE DESIGN SHEETS 23 & 24)

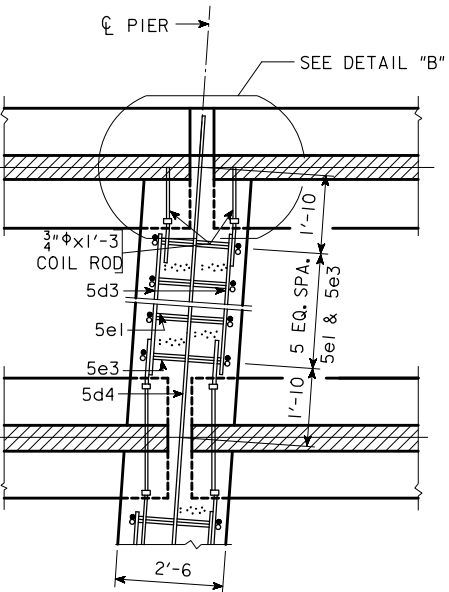


PART END VIEW AT ABUTMENT

SECTION A-A



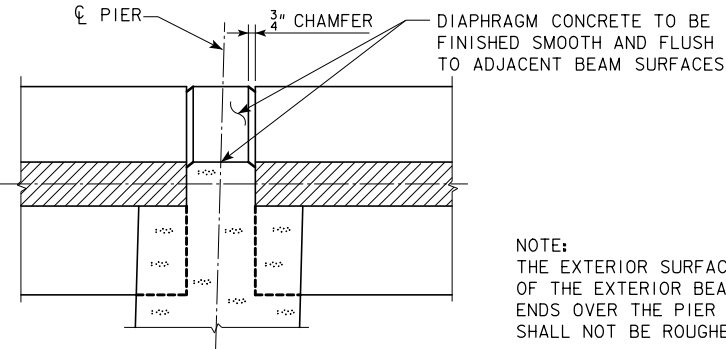
PART PLAN



PART SECTION AT PIER

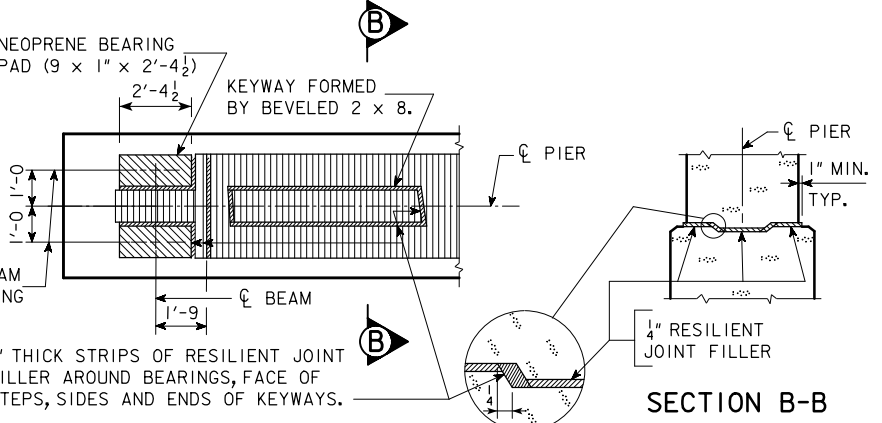
TABLE OF WING ELEVATIONS			
LOCATION	DIM "C"	ELEV. A	ELEV. B
SOUTH ABUT. @ WEST GUTTER LINE	1'-1 1/8"	960.93	960.93
NORTH ABUT. @ WEST GUTTER LINE	1'-1 1/8"	959.92	959.87
SOUTH ABUT. @ EAST GUTTER LINE	2'-5 3/8"	962.21	962.22
NORTH ABUT. @ EAST GUTTER LINE	2'-5 1/4"	961.21	961.17

DETAILS SHOWN ON THIS SHEET ARE APPLICABLE TO THE NORTH ABUTMENT AND PIER NO. 1. THE SOUTH ABUTMENT AND PIER NO. 2 ARE SIMILAR BUT OPPOSITE HAND.



DETAIL "B"

PART PLAN AT ENDS OF PIER DIAPHRAGM



PART PLAN

TOP OF PIER DETAILS

DESIGN FOR VARIABLE SKEW RADIUS = 1,820'

214'-0 X 28'-0 PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGE

66'-0 & 71'-0 END SPANS 77'-0 INTERIOR SPAN

ABUT. & PIER DIAPHRAGM DETAILS

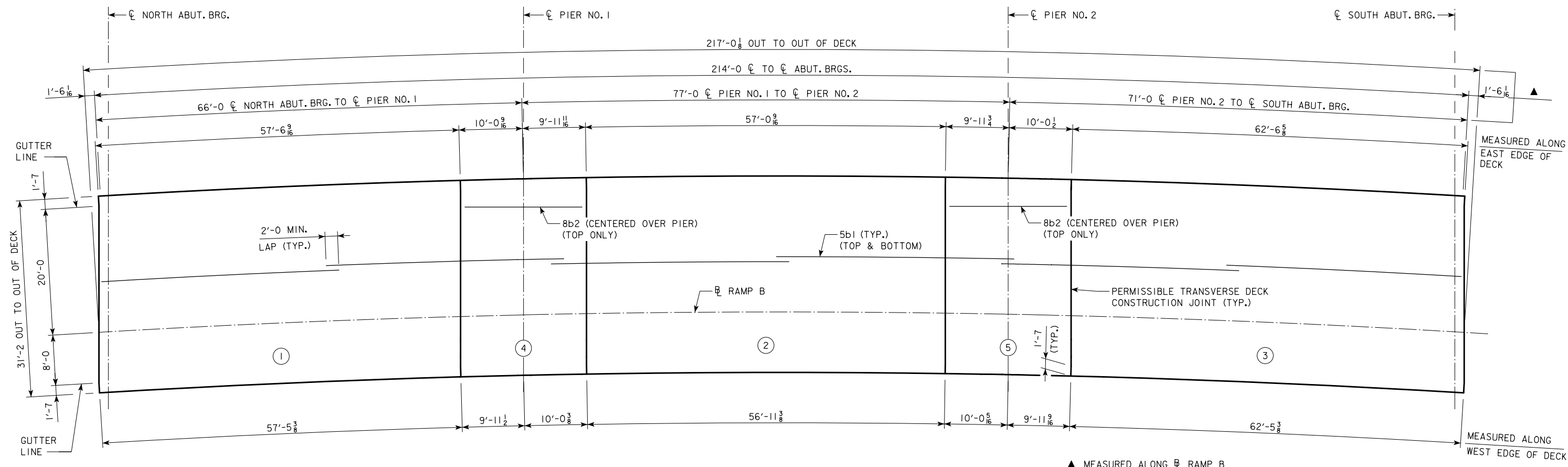
STA. 32591+41.72 (CL RAMP B)

POLK COUNTY

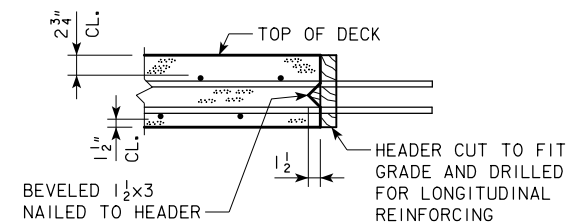
IOWA DEPARTMENT OF TRANSPORTATION

DESIGN SHEET NO. 15 OF 34 FILE NO. 31046 DESIGN NO. 2118

ENGLISHBTINTEGRALBRIDGES.DGN - 4501-BTB - THIS SHEET ISSUED 02-08.



CONCRETE PLACEMENT DIAGRAM & LONGITUDINAL REINFORCING LAYOUT



PERMISSIBLE TRANSVERSE DECK CONSTRUCTION JOINT

NOTES:
CONCRETE DECK SHALL BE PLACED IN SECTIONS AND SEQUENCES INDICATED. (AN APPROVED ALTERNATE PROCEDURE IS TO PLACE THE CONCRETE DECK IN ONE CONTINUOUS POUR BEGINNING AT ONE END OF THE BRIDGE.) ALTERNATE PROCEDURES FOR PLACING DECK CONCRETE MAY BE SUBMITTED FOR APPROVAL TOGETHER WITH A STATEMENT OF THE PROPOSED METHOD AND EVIDENCE THAT THE CONTRACTOR POSSESSES THE NECESSARY EQUIPMENT AND FACILITIES TO ACCOMPLISH THE REQUIRED RESULTS. THE BRIDGE ENGINEER SHALL REVIEW ANY ALTERNATE PROCEDURES. THE COST OF ANY ADDITIONAL ANALYSIS AND PLAN MODIFICATIONS SHALL BE PAID FOR BY THE CONTRACTOR. THE ENGINEER SHALL DETERMINE IF A RETARDING ADMIXTURE IS REQUIRED TO MAINTAIN PLASTICITY OF THE CONCRETE DECK DURING PLACEMENT.
DECK CONCRETE SECTIONS SHALL CURE FOR A MINIMUM OF 48 HOURS AND SHALL ACHIEVE A MINIMUM STRENGTH OF 75% OF THE 28 DAY DECK CONCRETE STRENGTH PRIOR TO REMOVING DECK HEADERS AND BEGINNING AN ADJACENT POUR.

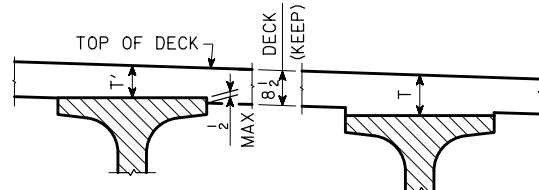
DESIGN FOR VARIABLE SKEW RADIUS = 1,820'
**214'-0 X 28'-0 PRETENSIONED
PRESTRESSED CONCRETE BEAM BRIDGE**
66'-0 & 71'-0 END SPANS 77'-0 INTERIOR SPAN
DECK PLAN & REINFORCING
STA. 32591+41.72 (RAMP B) APRIL, 2022
POLK COUNTY
IOWA DEPARTMENT OF TRANSPORTATION
DESIGN SHEET NO. 17 OF 34 FILE NO. 31046 DESIGN NO. 2118

HDR TOP TRANSVERSE REINFORCING LAYOUT

BOTTOM TRANSVERSE REINFORCING LAYOUT

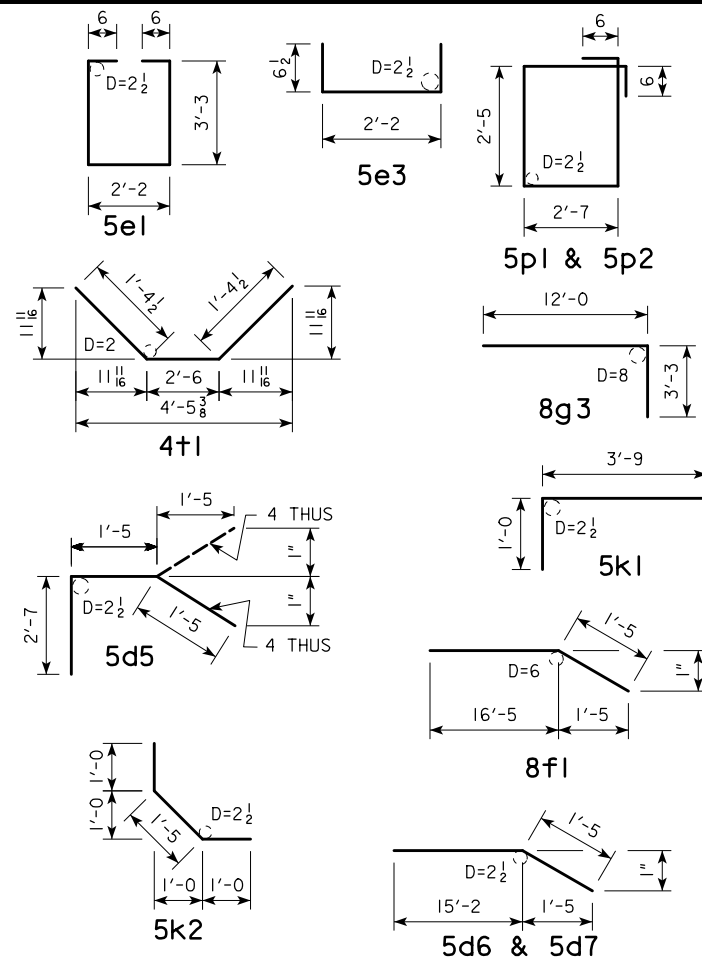
CONCRETE PLACEMENT QUANTITIES	
LOCATION	QUANTITY
SECTION 1, DECK & ABUT. DIAPH.	63.0
SECTION 2, DECK	49.5
SECTION 3, DECK & ABUT. DIAPH.	67.0
SECTION 4, DECK & PIER DIAPH.	24.2
SECTION 5, DECK & PIER DIAPH.	24.3
TOTAL (CU. YDS.)	228.0

NOTE:
CONCRETE AND REINFORCING STEEL QUANTITIES ARE INCLUDED
ON THE SUMMARY QUANTITIES SHEET.



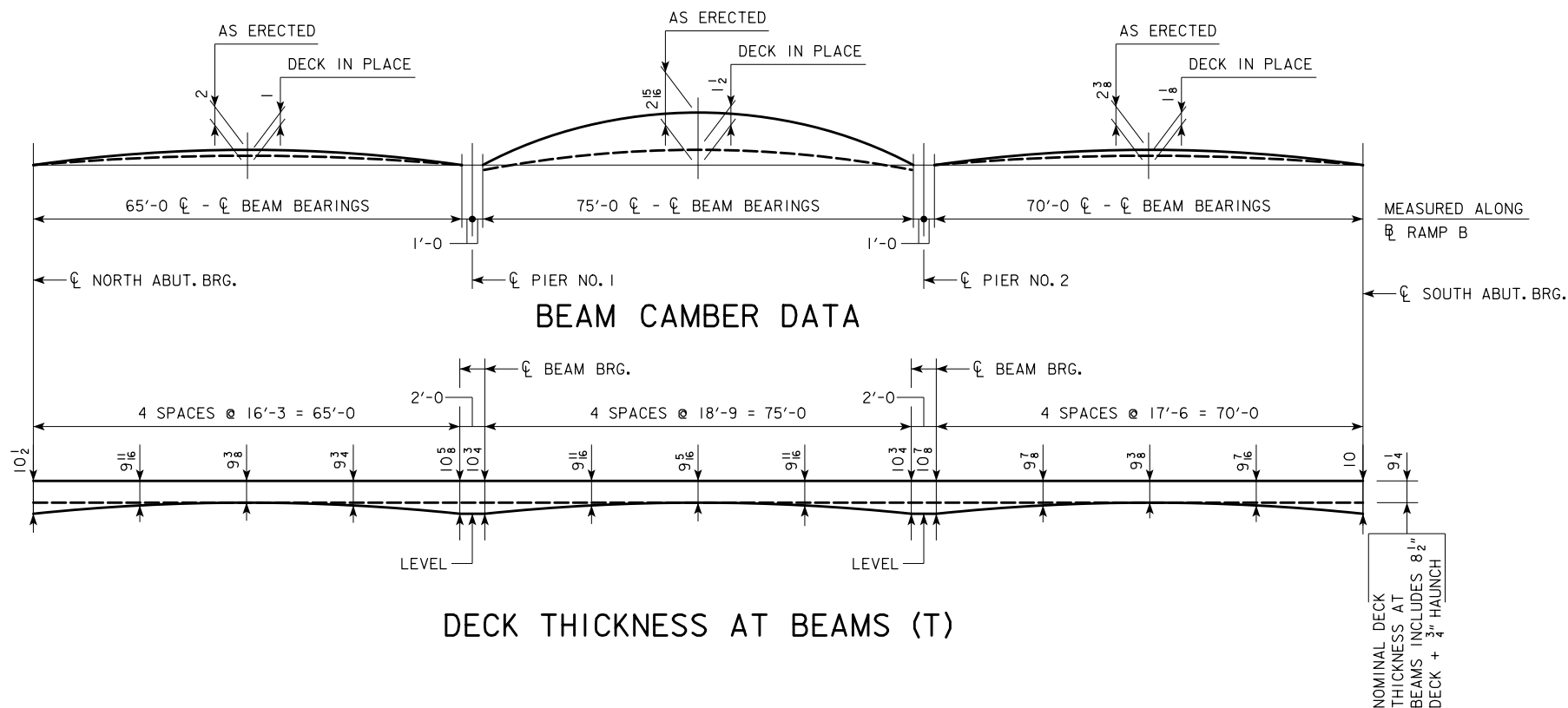
DECK THICKNESS DETAILS

NOTE:
THE DECK THICKNESS (T) AT BEAMS IS BASED ON THE ANTICIPATED BEAM CAMBER AND DEFLECTIONS. THESE VALUES ARE USED BY THE DESIGNER TO SET BEAM ELEVATIONS AND ESTIMATE CONCRETE QUANTITIES. REFER TO THE HAUNCH DATA DETAILS SHEET FOR ADDITIONAL INFORMATION TO AID THE CONTRACTOR IN SETTING THE FIELD HAUNCHES REQUIRED FOR CONSTRUCTION.



NOTE: ALL DIMENSIONS ARE OUT TO OUT. D= PIN DIAMETER.

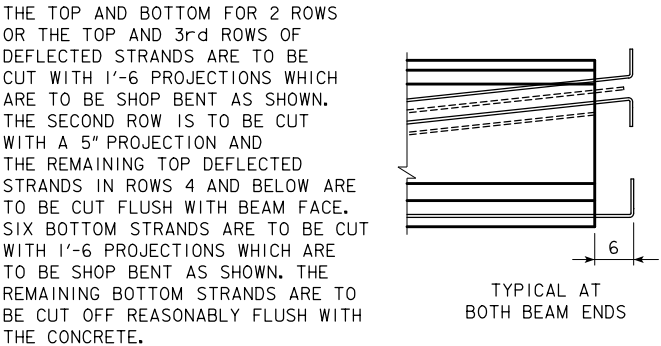
BENT BAR DETAILS



REINFORCING BAR LIST

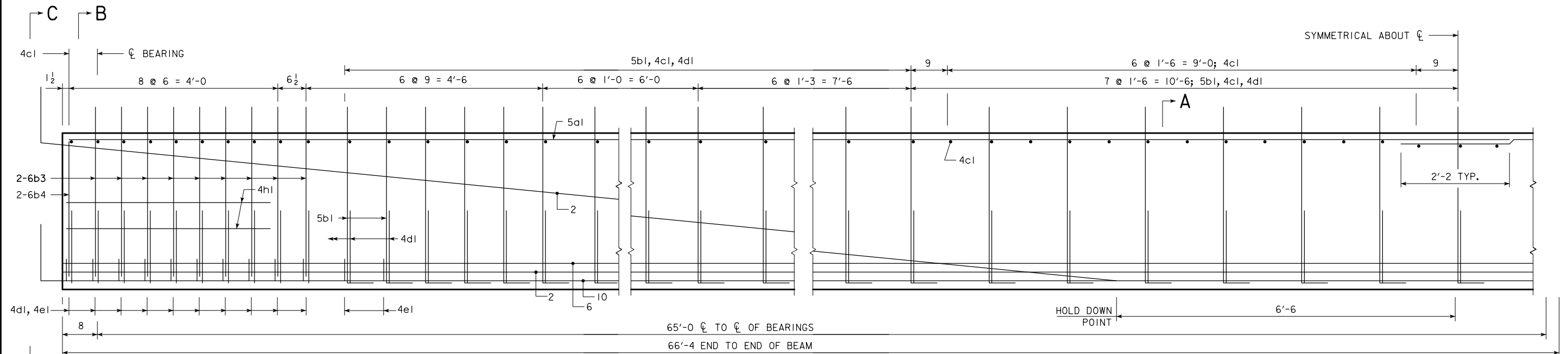
STAINLESS	BAR	LOCATION	NO.	LENGTH	WEIGHT
	6a1	DECK TRANSV. TOP & BOTT.	—	495	30'-10 22,924
	5b1	DECK LONGIT. TOP & BOTT.	—	426	37'-10 16,810
	8b2	DECK LONGIT. TOP AT PIERS	—	64	19'-8 3,361
	8g3	ABUT. DIAPH. VERT. B.F.	┌	52	15'-3 2,117
	6j1	TOP OF DECK TRANSV. (AT RAIL)	—	494	6'-3 4,637
	REINFORCING STEEL STAINLESS - TOTAL (LBS.)				49,849
EPOXY COATED REINFORCING	5d2	PIER & ABUT. DIAPH. LONGIT.	—	36	7'-3 272
	5d3	PIER & ABUT. DIAPH. LONGIT.	—	18	5'-5 102
	5d4	PIER DIAPH. LONGIT.	—	2	26'-8 56
	5d5	ABUT. DIAPH. ENDS	┐	8	5'-5 45
	5d6	ABUT. DIAPH. LONGIT. B.F.	┐	12	16'-7 208
	5d7	PAVING NOTCH LONGIT.	┐	8	16'-7 138
	5e1	PIER DIAPH. HOOPS	□	36	9'-8 363
	5e3	PIER DIAPH. TIES	┐	36	3'-3 122
	8f1	ABUT. FOOTING LONGIT. BOTH F.	┐	36	17'-10 1,714
	8g1	ABUT. VERT. BOTH F.	—	112	7'-2 2,143
	5h2	ABUT. TO WING ANCHOR	—	56	4'-11 287
	5k1	PAVING NOTCH	┌	56	4'-9 277
	5k2	PAVING NOTCH	┐	56	3'-5 200
	5p1	ABUT. HOOPS	□	84	11'-0 964
	5p2	ABUT. HOOPS AT ENDS	□	8	11'-0 92
	4t1	UNDER BEAMS AT ABUTMENTS	┐	8	5'-3 28
	REINFORCING STEEL EPOXY COATED - TOTAL (LBS.)				7,011
NON-COATED	#2	PILE SPIRAL	〰	16	38'-6 103
		SPIRAL SPACERS, L $\frac{7}{8} \times \frac{7}{8} \times \frac{1}{8} \times 0.70$	—	48	1'-10 62
REINFORCING STEEL - TOTAL (LBS.)				165	

DESIGN FOR VARIABLE SKEW $RADIUS = 1,820'$
**214'-0" X 28'-0" PRETENSIONED
PRESTRESSED CONCRETE BEAM BRIDGE**
66'-0" & 71'-0" END SPANS 77'-0" INTERIOR SPAN
SUPERSTRUCTURES DETAILS
STA. 32591+41.72 (RAMP B) APRIL, 2022
POLK COUNTY
IOWA DEPARTMENT OF TRANSPORTATION
DESIGN SHEET NO. 18 OF 34 FILE NO. 31046 DESIGN NO. 2118

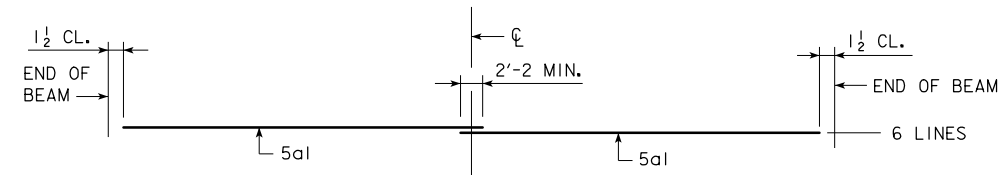


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ENGLISHBEAMS.DGN - 4758 - THIS SHEET ISSUED 02-08.

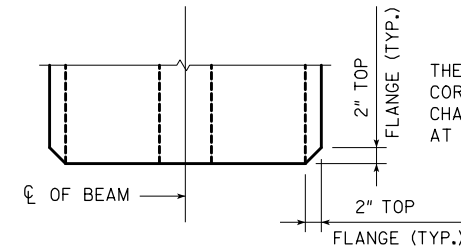


BTB65



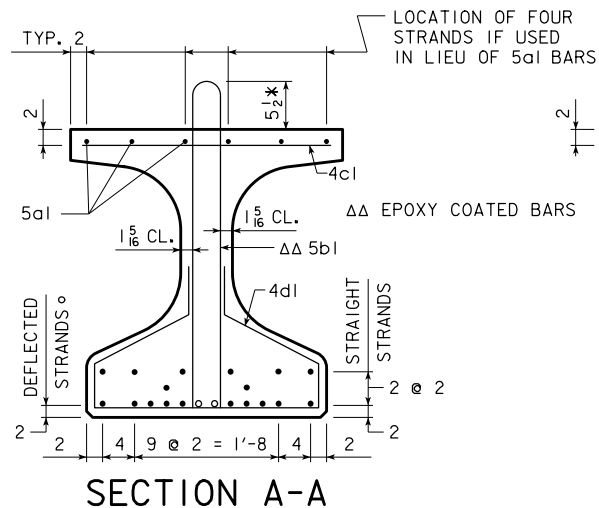
TOP FLANGE LONGITUDINAL BAR LAYOUT

NOTE: STIRRUP EXTENSION
* THE STANDARD 5" PROJECTION HAS BEEN INCREASED.

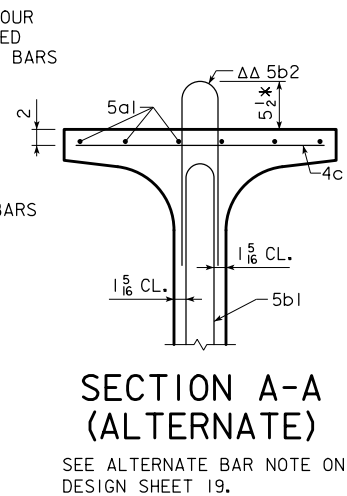


TOP VIEW

THE TOP FLANGE BEAM CORNERS ARE TO BE CHAMFERED 2" AS SHOWN AT BOTH ENDS OF THE BEAM.

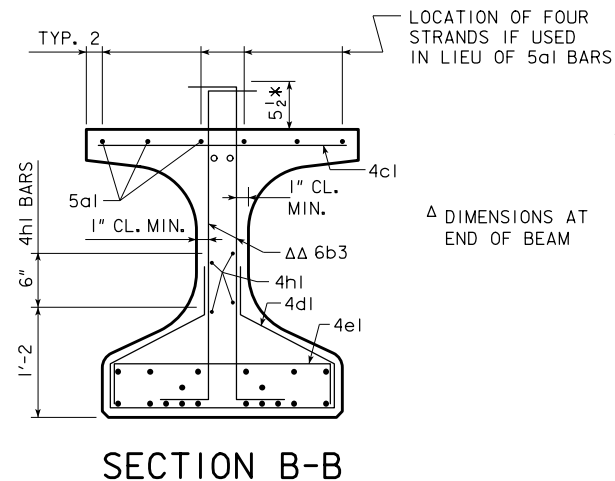


SECTION A-A



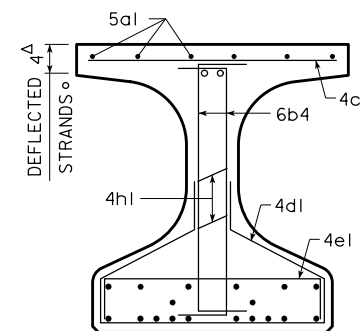
SECTION A-A (ALTERNATE)

SEE ALTERNATE BAR NOTE ON DESIGN SHEET 19.



SECTION B-B

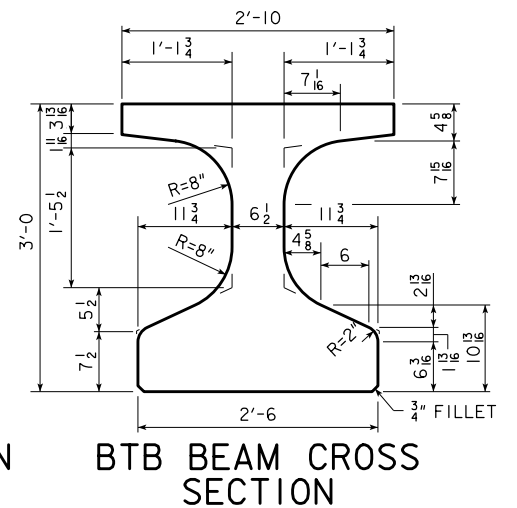
A DIMENSIONS AT END OF BEAM



SECTION C-C

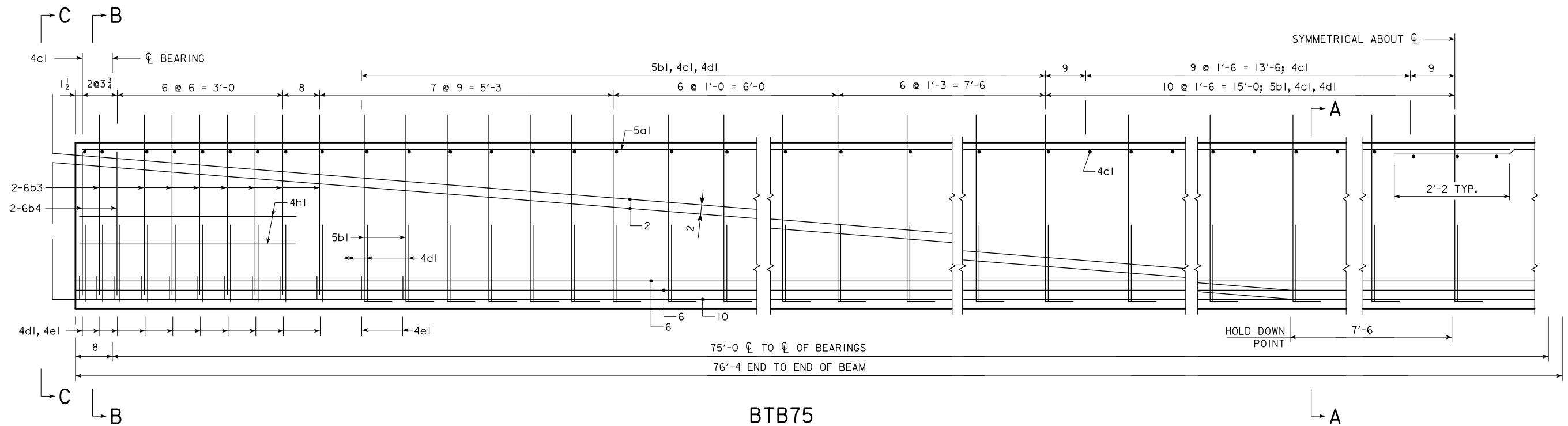
BEAM SECTION PROPERTIES

AREA = 631.7 in²
y_b = 17.14 in.
I = 99,980 in⁴

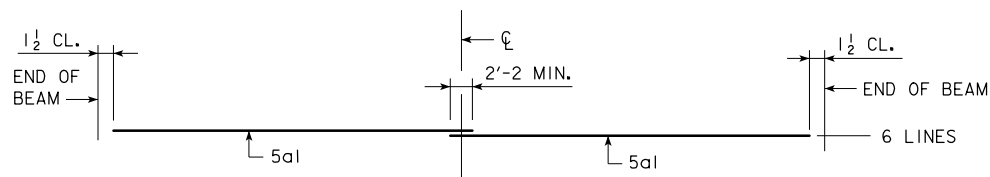


DESIGN FOR VARIABLE SKEW RADIUS = 1,820'
**214'-0 X 28'-0 PRETENSIONED
PRESTRESSED CONCRETE BEAM BRIDGE**
66'-0 & 71'-0 END SPANS 77'-0 INTERIOR SPAN
BTB65 BEAM DETAILS
STA. 32591+41.72 (CL RAMP B) APRIL, 2022
POLK COUNTY
IOWA DEPARTMENT OF TRANSPORTATION
DESIGN SHEET NO. 20 OF 34 FILE NO. 31046 DESIGN NO. 2118

ENGLISHBEAMS.DGN - 4760 - THIS SHEET ISSUED 02-08.

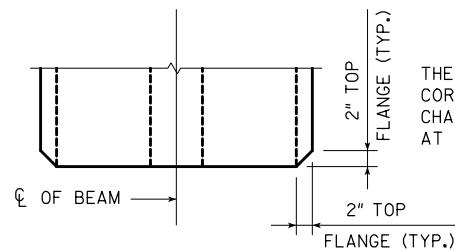


BTB75



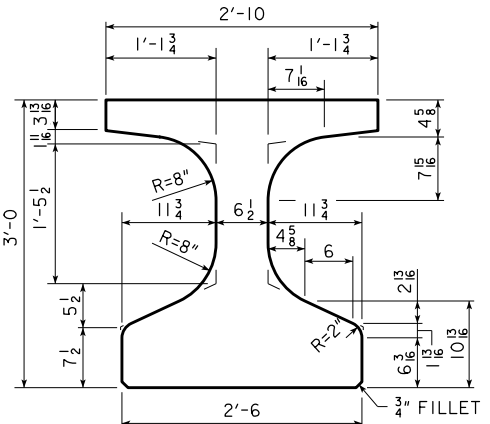
TOP FLANGE LONGITUDINAL BAR LAYOUT

NOTE: STIRRUP EXTENSION
* THE STANDARD 5" PROJECTION HAS BEEN INCREASED.



TOP VIEW

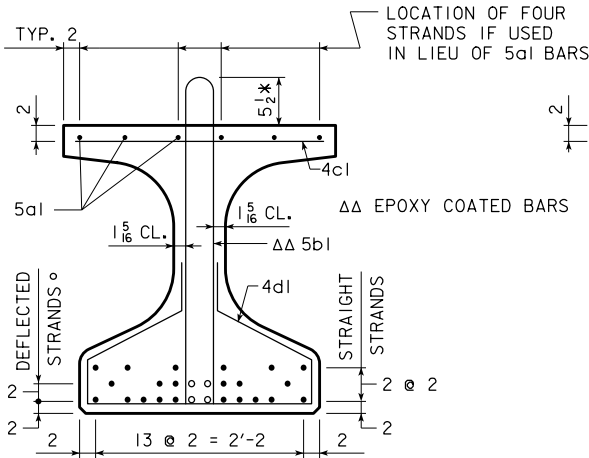
THE TOP FLANGE BEAM CORNERS ARE TO BE CHAMFERED 2" AS SHOWN AT BOTH ENDS OF THE BEAM.



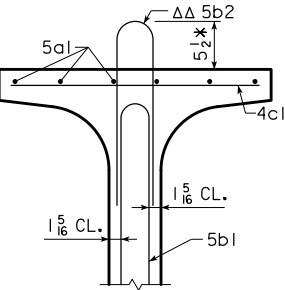
BTB BEAM CROSS SECTION

AREA = 631.7 in²
y_b = 17.14 in.
I = 99,980 in⁴

BEAM SECTION PROPERTIES

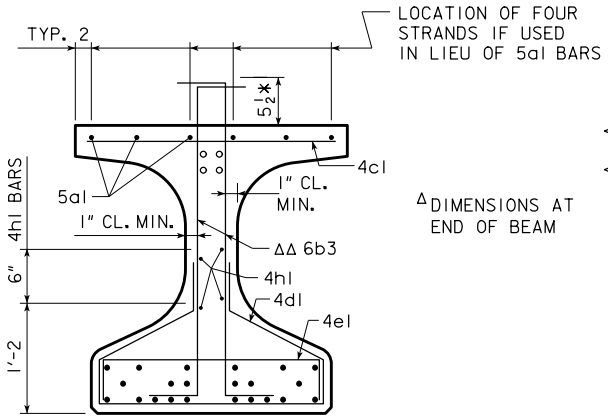


SECTION A-A

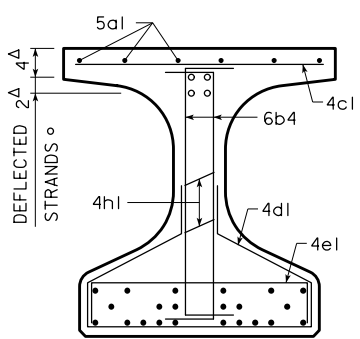


SECTION A-A (ALTERNATE)

SEE ALTERNATE BAR NOTE ON DESIGN SHEET 19.



SECTION B-B



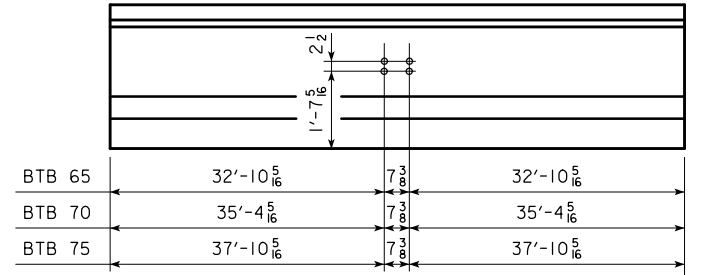
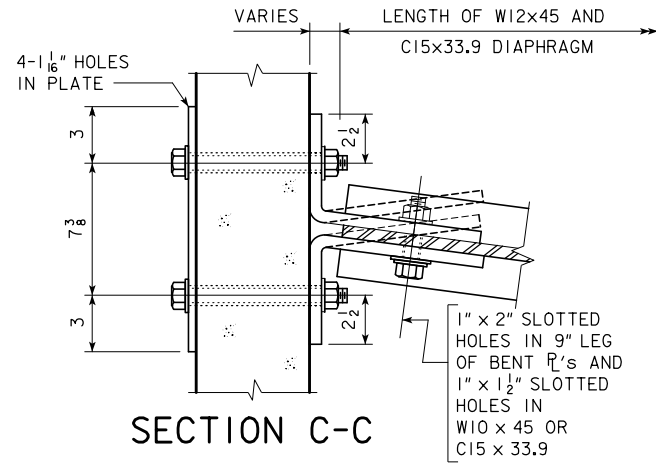
SECTION C-C

DESIGN FOR VARIABLE SKEW RADIUS = 1,820'
214'-0 X 28'-0 PRETENSIONED
PRESTRESSED CONCRETE BEAM BRIDGE
66'-0 & 71'-0 END SPANS 77'-0 INTERIOR SPAN
BTB75 BEAM DETAILS
STA. 32591+41.72 (CL RAMP B) APRIL, 2022
POLK COUNTY
IOWA DEPARTMENT OF TRANSPORTATION
DESIGN SHEET NO. 22 OF 34 FILE NO. 31046 DESIGN NO. 2118

BULB TEE "B" BEAM INTERMEDIATE DIAPHRAGM STRUCTURAL STEEL			
ONE BEAM CONNECTION (DETAIL "D", "E", "F", OR "G")			WEIGHT
		NO. OF BEAM CONNECTIONS	
4 - $\frac{7}{8}$ " ϕ \times 9 $\frac{1}{4}$ H.S.BOLTS WITH NUTS & WASHERS = 9.6 LBS.		12	115
ONE DETAIL "E"	2 - BENT \overline{C} 9 \times 6 \times $\frac{1}{2}$ \times 0'-11 = 46.8 LBS.	2	94
	2 - BENT \overline{C} 9 \times 6 \times $\frac{1}{2}$ \times 0'-5 = 21.3 LBS.	2	43
ONE DETAIL "D"	1 - BACKING \overline{C} 5 \times $\frac{3}{8}$ \times 1'-1 $\frac{3}{8}$ = 7.1 LBS.	2	14
	2 - BENT \overline{C} 9 \times 6 \times $\frac{1}{2}$ \times 0'-5 = 21.3 LBS.	2	43
ONE DETAIL "G"	4 - BENT \overline{C} 9 \times 6 \times $\frac{1}{2}$ \times 0'-11 = 93.6 LBS.	4	374
ONE DETAIL "F"	1 - BACKING \overline{C} 5 \times $\frac{3}{8}$ \times 1'-1 $\frac{3}{8}$ = 7.1 LBS.	4	28
	2 - BENT \overline{C} 9 \times 6 \times $\frac{1}{2}$ \times 0'-11 = 46.8 LBS.	4	187
ONE DIAPHRAGM			
		NUMBER OF DIAPHRAGMS	
6 - $\frac{7}{8}$ " ϕ \times 3" H.S.BOLTS WITH NUTS & WASHERS = 7.8 LBS.		7	55
4 - $\frac{7}{8}$ " ϕ \times 3" H.S.BOLTS WITH NUTS & WASHERS = 5.2 LBS.		2	10
18 - $\frac{7}{8}$ " ϕ \times 2" H.S.BOLTS WITH NUTS & WASHERS = 16.8 LBS.		2	34
4 - $\frac{7}{8}$ " ϕ \times 2" H.S.BOLTS WITH NUTS & WASHERS = 4.0 LBS.		2	8
2 - \overline{C} 6 $\frac{1}{2}$ \times $\frac{3}{8}$ \times 1'-2 = 19.3 LBS.		2	39
4 - \overline{C} 6 $\frac{1}{2}$ \times $\frac{3}{8}$ \times 1'-2 = 12.9 LBS.		2	26
	LENGTH OF MEMBER		
1 - W12 \times 45 = 45 LBS./FT.	6'-11 $\frac{3}{4}$	2	628
1 - C15 \times 33.9 = 33.9 LBS./FT.	6'-11 $\frac{3}{4}$	7	1,656
1 - W14 \times 38 = 38 LBS./FT.	5'-7 $\frac{1}{4}$	2	426
INTERMEDIATE DIAPHRAGM STRUCTURAL STEEL - TOTAL (LBS.)			3,780

STRUCTURAL STEEL	
WEIGHT	3,780 LBS.

NOTE: STRUCTURAL STEEL WEIGHT
IS INCLUDED ON THE
SUMMARY QUANTITIES SHEET.



INTERMEDIATE DIAPHRAGM BOLT HOLE LOCATIONS

NOTES:

ALL DIAPHRAGM MATERIALS, INCLUDING BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED.

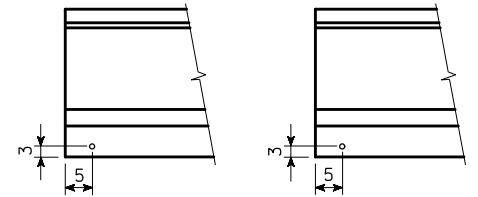
SHOP DRAWINGS OF THE STEEL DIAPHRAGMS SHOWING LAYOUT AND DETAILS OF THE DIAPHRAGMS SHALL BE SUBMITTED FOR APPROVAL.

ALL COSTS FOR FURNISHING AND INSTALLING STEEL
INTERMEDIATE DIAPHRAGMS SHALL BE INCLUDED IN THE PRICE
BID FOR STRUCTURAL STEEL.

THE $1\frac{1}{2}"\phi$ HOLES FOR THE $\frac{7}{8}"\phi$ H.S. BOLTS
SHALL BE CAST INTO THE WEB. DRILLING IS NOT ALLOWED.

THE $\frac{7}{8}"$ H.S. BOLTS THROUGH THE WEB SHALL HAVE A
THREAD LENGTH OF 3" MIN. AND 4" MAX. AND SHALL MEET
THE REQUIREMENTS OF ASTM A449.

ALL BOLTS ARE TO BE TIGHTENED PRIOR TO PLACING
BRIDGE FLOOR CONCRETE.



INTEGRAL ABUT. FIXED PIER

BEAM COIL TIE LOCATIONS

NOTE:
FOR LOCATION OF SECTION C-C, SEE DESIGN SHEET 24.

DESIGN FOR VARIABLE SKEW RADIUS = 1,820'

214'-0 X 28'-0 PRETENSIONED
PRESTRESSED CONCRETE BEAM BRIDGE

66'-0 & 71'-0 END SPANS 77'-0 INTERIOR SPAN

STEEL DIAPHRAGM DETAILS

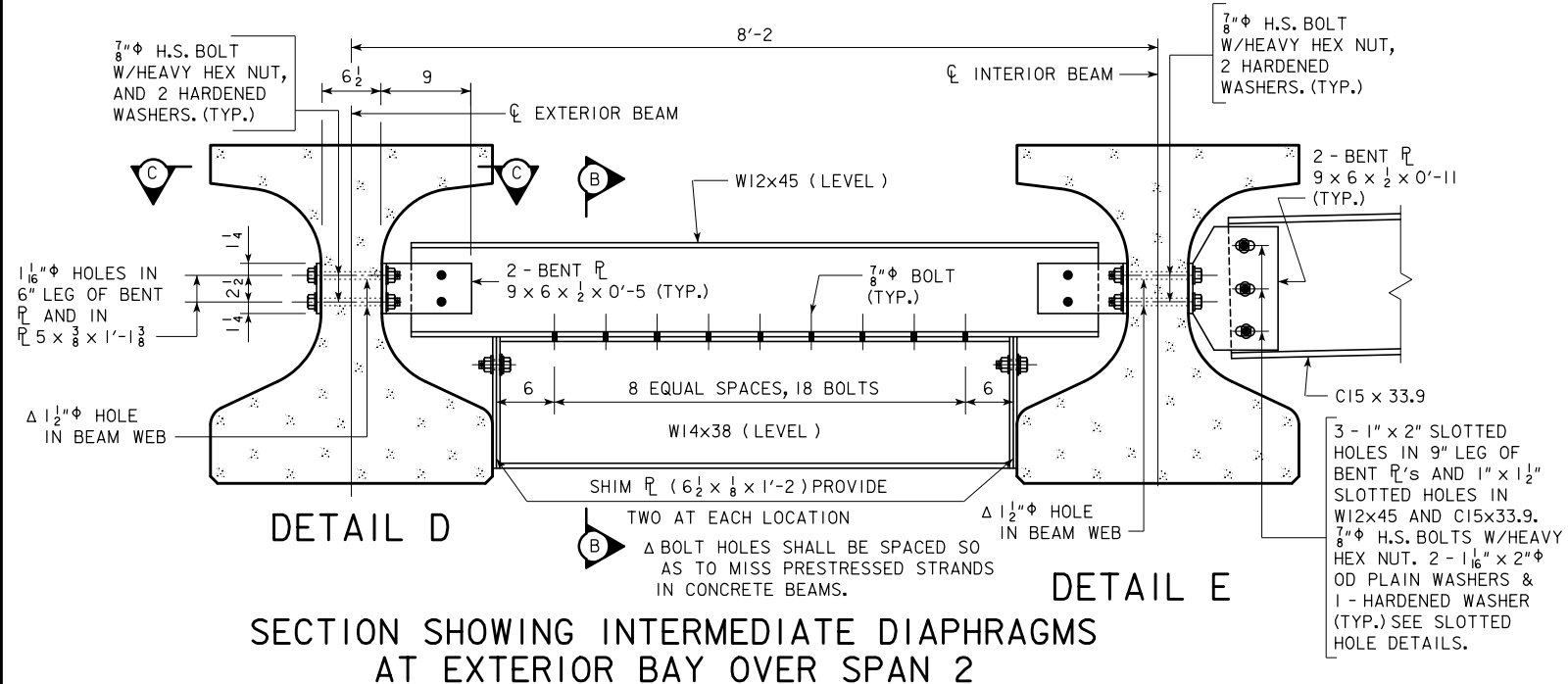
STA. 32591+41.72 @ RAMP B APRIL, 2022

POLK COUNTY

IOWA DEPARTMENT OF TRANSPORTATION

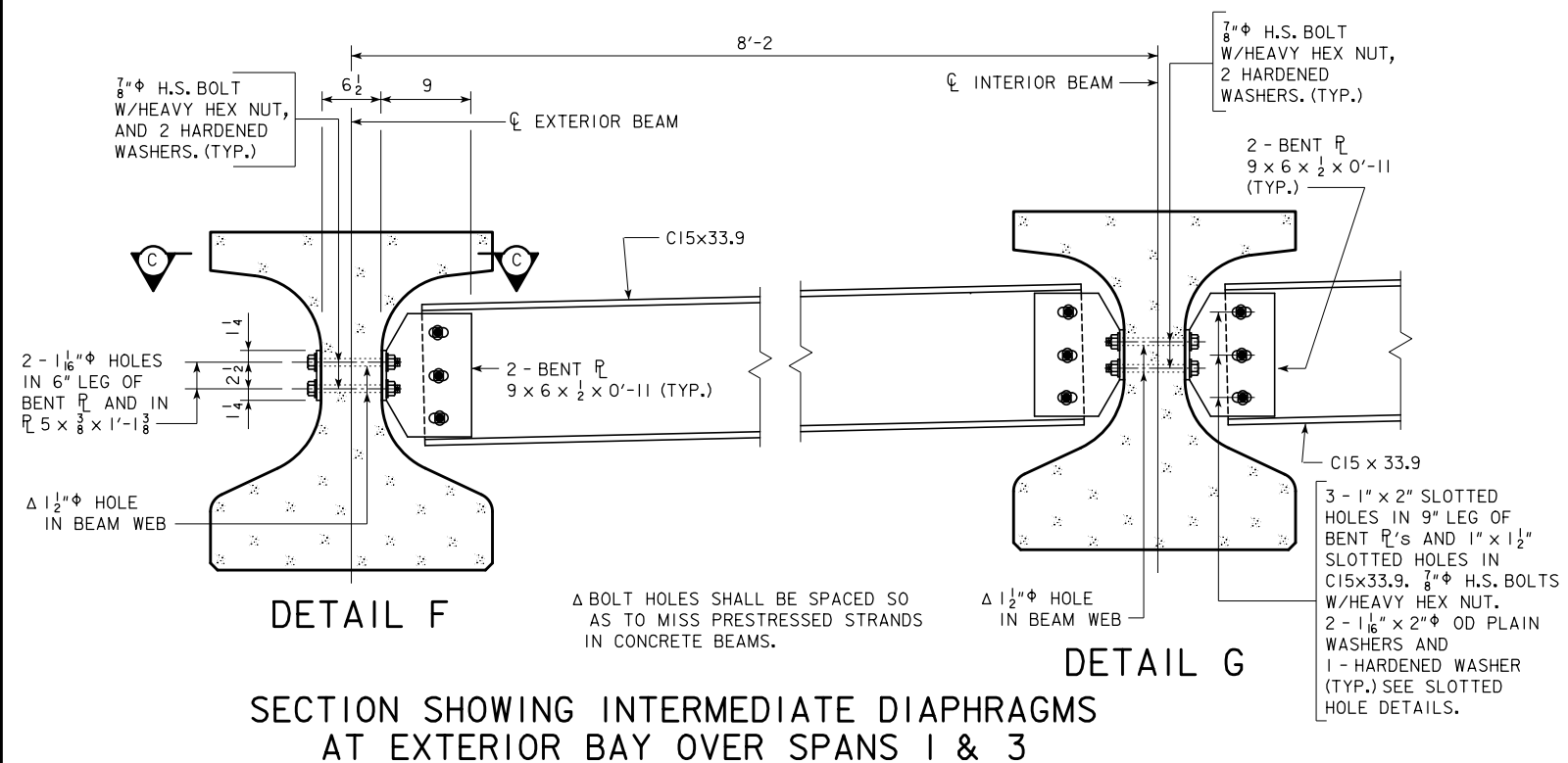
DESIGN SHEET NO. 23 OF 34 FILE NO. 31046 DESIGN NO. 2118

ENGLISHBEAMS.DGN - 1036-2-BTB - THIS SHEET ISSUED 06-14, SHEET 2 OF 2.

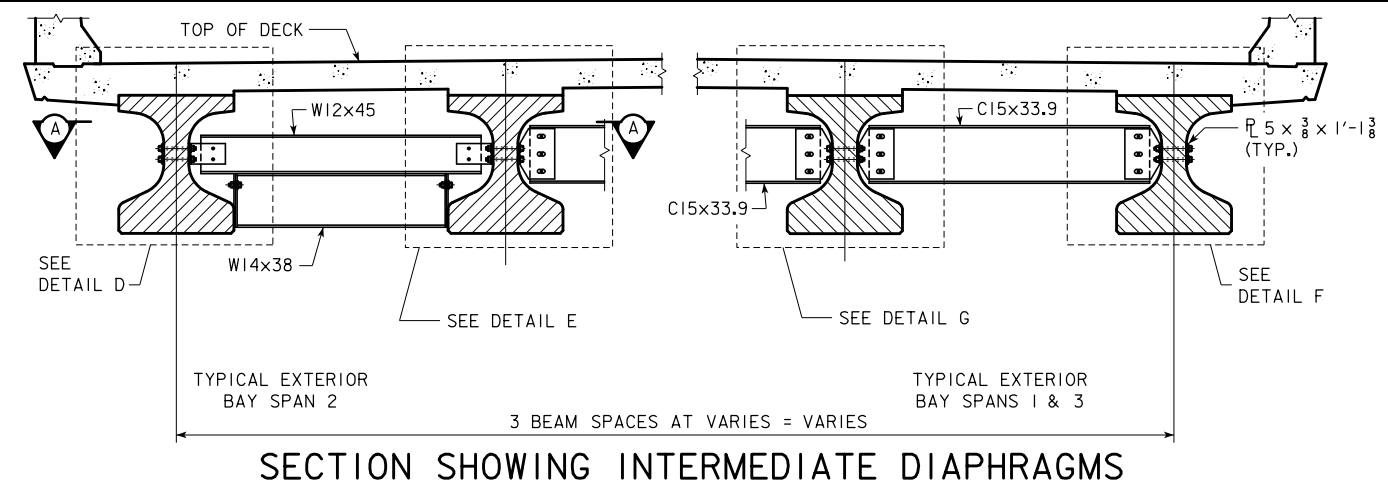


SECTION SHOWING INTERMEDIATE DIAPHRAGMS
AT EXTERIOR BAY OVER SPAN 2

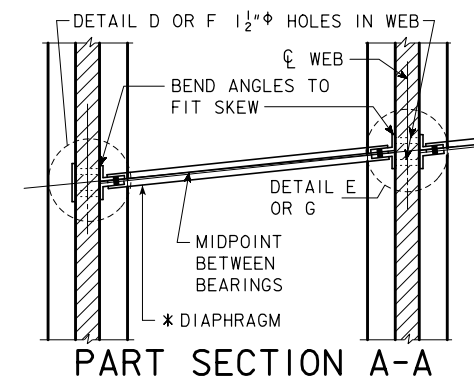
NOTE:
FOR SECTION C-C, SEE DESIGN SHEET 23.



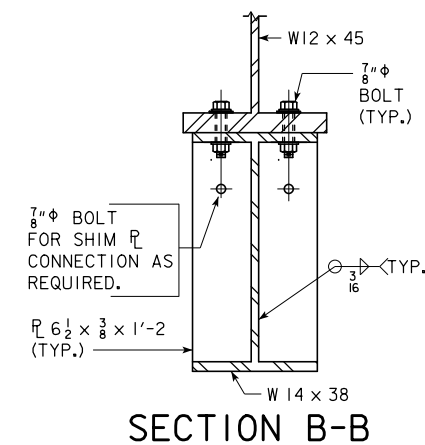
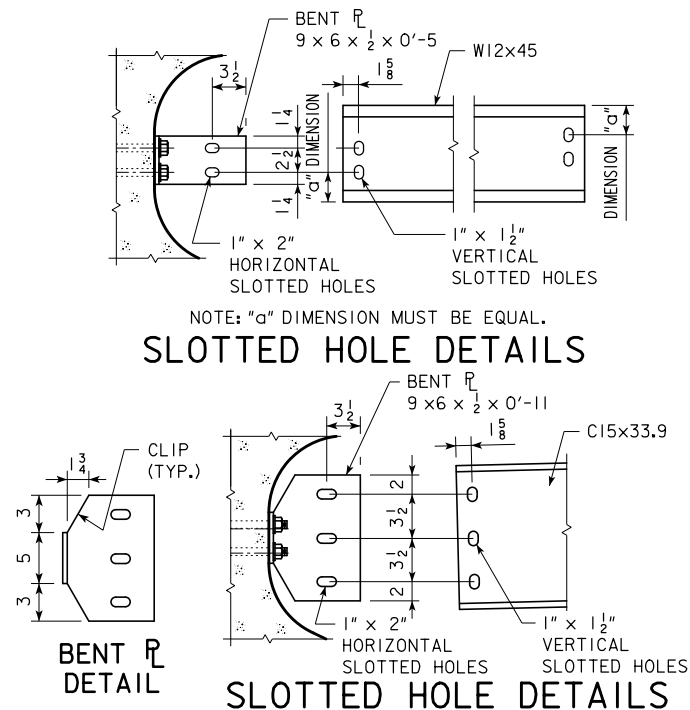
SECTION SHOWING INTERMEDIATE DIAPHRAGMS
AT EXTERIOR BAY OVER SPANS 1 & 3



SECTION SHOWING INTERMEDIATE DIAPHRAGMS

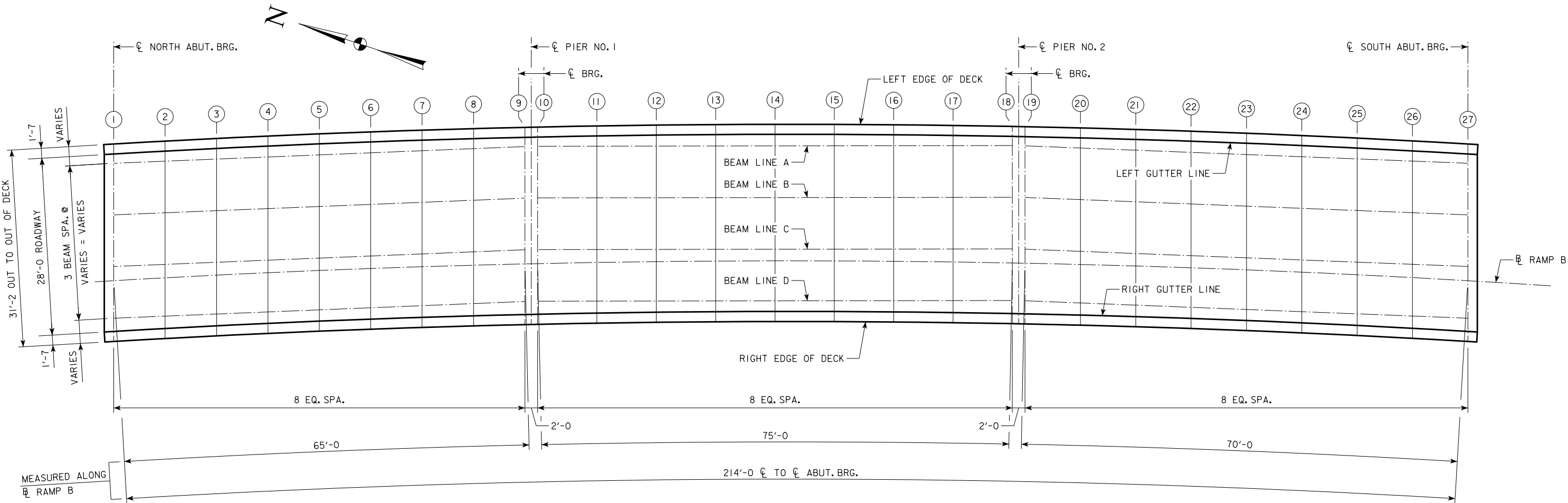


* NOTE: THE W 14 x 38 WILL REQUIRE
BEVELED ENDS TO MATCH THE SKEW.



DESIGN FOR VARIABLE SKEW RADIUS = 1,820'
214'-0 X 28'-0 PRETENSIONED
PRESTRESSED CONCRETE BEAM BRIDGE
66'-0 & 71'-0 END SPANS 77'-0 INTERIOR SPAN
STEEL DIAPHRAGM DETAILS
STA. 32591+41.72 (RAMP B) APRIL, 2022
POLK COUNTY
IOWA DEPARTMENT OF TRANSPORTATION
DESIGN SHEET NO. 24 OF 34 FILE NO. 31046 DESIGN NO. 2118

TABLE OF TOP OF DECK ELEVATIONS																											
	CL NORTH ABUT. BRG.								CL PIER NO. 1 BEARINGS								CL PIER NO. 2 BEARINGS								CL SOUTH ABUT. BRG.		
LINE \ POINT	(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(13)	(14)	(15)	(16)	(17)	(18)	(19)	(20)	(21)	(22)	(23)	(24)	(25)	(26)	(27)
LEFT EDGE OF DECK	961.30	961.35	961.40	961.45	961.51	961.56	961.61	961.66	961.72	961.73	961.79	961.84	961.89	961.94	961.99	962.03	962.07	962.11	962.11	962.15	962.17	962.20	962.22	962.24	962.26	962.27	962.28
LEFT GUTTER LINE	961.22	961.28	961.33	961.38	961.43	961.49	961.54	961.59	961.64	961.66	961.71	961.77	961.82	961.87	961.92	961.96	962.00	962.03	962.04	962.07	962.10	962.13	962.15	962.17	962.19	962.20	962.21
BEAM LINE A	961.16	961.20	961.25	961.30	961.35	961.41	961.46	961.52	961.58	961.59	961.64	961.69	961.74	961.79	961.83	961.88	961.92	961.97	961.98	962.00	962.02	962.05	962.07	962.09	962.11	962.13	962.14
BEAM LINE B	960.78	960.83	960.87	960.92	960.98	961.03	961.08	961.14	961.20	961.21	961.26	961.31	961.36	961.41	961.46	961.50	961.55	961.59	961.60	961.62	961.65	961.67	961.69	961.71	961.73	961.75	961.77
BEAM LINE C	960.40	960.45	960.50	960.55	960.60	960.65	960.71	960.76	960.82	960.84	960.89	960.94	960.99	961.03	961.08	961.13	961.17	961.22	961.23	961.25	961.27	961.30	961.32	961.34	961.36	961.38	961.40
CL RAMP B	960.30	960.35	960.40	960.46	960.51	960.56	960.61	960.67	960.72	960.73	960.79	960.85	960.90	960.95	961.00	961.04	961.08	961.12	961.12	961.15	961.18	961.21	961.23	961.25	961.27	961.28	961.29
BEAM LINE D	960.02	960.07	960.12	960.17	960.22	960.27	960.33	960.39	960.45	960.46	960.51	960.56	960.61	960.66	960.71	960.75	960.80	960.84	960.85	960.88	960.90	960.92	960.94	960.96	960.98	961.00	961.02
RIGHT GUTTER LINE	959.92	959.98	960.03	960.08	960.14	960.19	960.24	960.30	960.35	960.36	960.42	960.48	960.53	960.58	960.63	960.67	960.71	960.75	960.76	960.79	960.81	960.84	960.86	960.88	960.90	960.91	960.93
RIGHT EDGE OF DECK	959.92	959.98	960.03	960.08	960.14	960.19	960.24	960.30	960.35	960.36	960.42	960.48	960.53	960.58	960.63	960.67	960.71	960.75	960.76	960.79	960.82	960.84	960.86	960.88	960.90	960.91	960.93



TOP OF DECK ELEVATIONS & HAUNCH LOCATIONS

DESIGN FOR VARIABLE SKEW RADIUS = 1,820'

214'-0 X 28'-0 PRETENSIONED
PRESTRESSED CONCRETE BEAM BRIDGE

66'-0 & 71'-0 END SPANS 77'-0 INTERIOR SPAN

TOP OF DECK ELEVATIONS

STA. 32591+41.72 (CL RAMP B) APRIL, 2022

POLK COUNTY

IOWA DEPARTMENT OF TRANSPORTATION

DESIGN SHEET NO. 25 OF 34 FILE NO. 31046 DESIGN NO. 2118



REVISED 06-12 - THE ALLOWABLE FIELD HAUNCH MAX. & MIN. WAS CHANGED TO INCHES & DECIMALS OF FEET. NOTE & NOTE I WERE CHANGED. THE SLAB HAUNCH LOCATIONS EXAMPLE WAS REPLACED WITH A NOTE.
REVISED 07-2019: CHANGED ALL REFERENCES OF "SLAB" TO "DECK".
ENGLISHMICELLANEOUSBRIDGES.DGN - 1066 - THIS SHEET ISSUED 02-08.

[illegible]

NOTE: BRIDGE SEAT ELEVATIONS ARE SET BASED ON THEORETICAL CAMBER AND BEAM DEFLECTIONS. THESE BRIDGE SEATS WILL PROVIDE A THEORETICAL BEAM HAUNCH WITHIN DESIGN PARAMETERS. FIELD HAUNCHES ARE DETERMINED USING SURVEYED TOP OF BEAM ELEVATIONS AND "BEAM LINE HAUNCH ELEVATION" DATA. ALLOWABLE MAXIMUM AND MINIMUM "FIELD HAUNCH" VALUES ARE GIVEN IN INCHES AND DECIMALS OF FEET IN THE "MISCELLANEOUS DATA" TABLE. "CROSS SLOPE ADJUSTMENT" VALUES WILL AID THE CONTRACTOR IN DETERMINING ACTUAL FORMED HAUNCH DIMENSIONS AT THE EDGES OF THE TOP FLANGE.

DESIGN FOR VARIABLE SKEW RADIUS = 1,820'

214'-0 X 28'-0 PRETENSIONED
PRESTRESSED CONCRETE BEAM BRIDGE

66'-0 & 71'-0 END SPANS 77'-0 INTERIOR SPAN

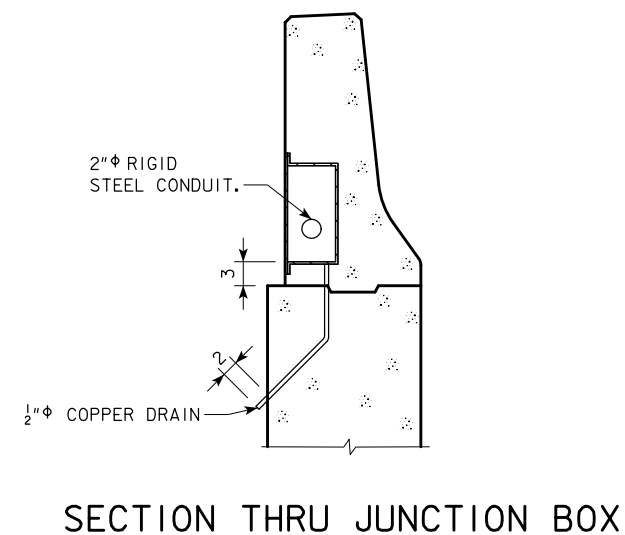
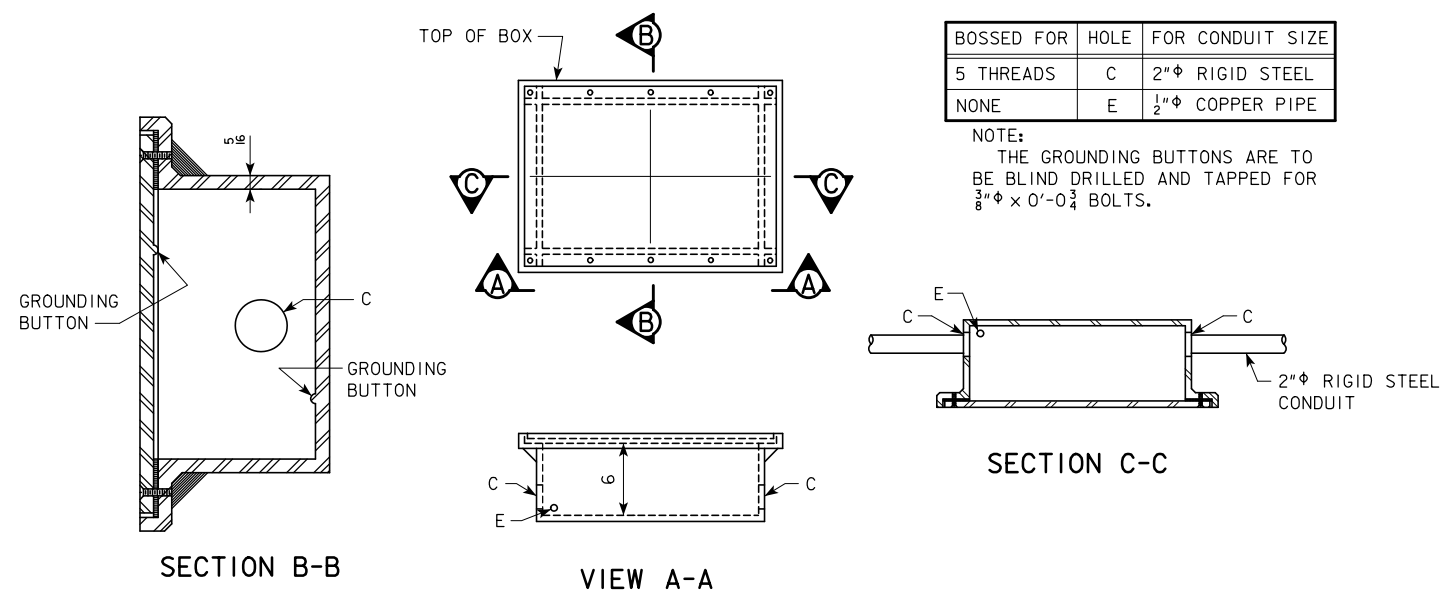
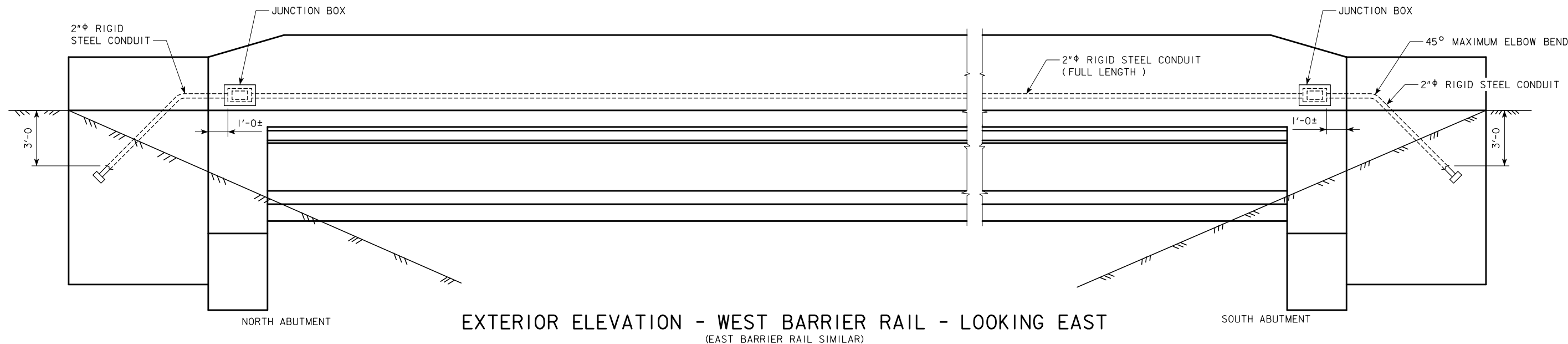
HAUNCH DATA DETAILS

STA. 32591+41.72 (RAMP B) APRIL, 2022

POLK COUNTY

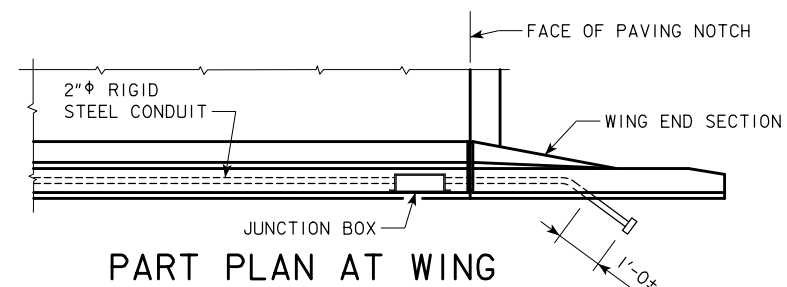
IOWA DEPARTMENT OF TRANSPORTATION

DESIGN SHEET NO. 26 OF 34 FILE NO. 31046 DESIGN NO. 2118



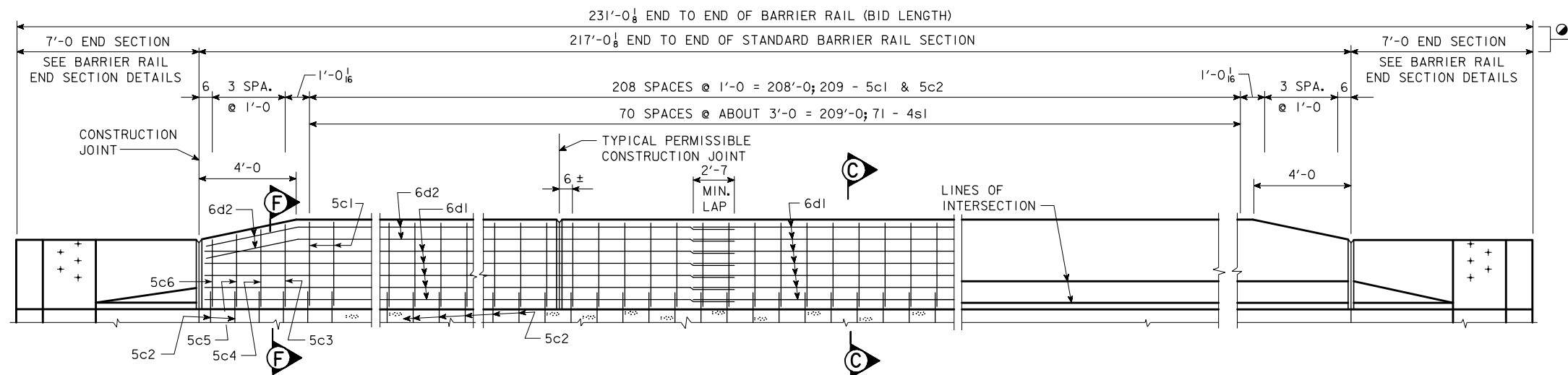
LIGHTING NOTES:

SEE LI-104 STANDARD ROAD PLAN FOR ADDITIONAL INFORMATION ON JUNCTION BOXES.
CONSTRUCTION SHALL CONFORM TO THE CURRENT IOWA D.O.T. STANDARD AND SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS.
CONDUIT INSTALLATION SHALL BE IN ACCORDANCE WITH ARTICLE 2523.03, N, OF THE STANDARD SPECIFICATIONS.
ALL "C" ENTRANCE HOLES IN JUNCTION BOXES SHALL BE DRILLED AND TAPPED FOR THE SPECIFIED CONDUIT SIZE. ALL OTHER HOLES SHALL HAVE A CONCRETE - TIGHT SLIP FIT. CONDUIT ENDS SHALL NOT PROTRUDE INTO JUNCTION BOX MORE THAN 1/4". DRAIN PIPE END SHALL BE FLUSH WITH INSIDE SURFACE OF BOX. GROUNDING BUTTONS SHALL BE LOCATED APPROXIMATELY 3" FROM THE INSIDE SURFACE OF THE BOX WALL, AND NOT CLOSER THAN 3" TO THE EDGE OF ANY HOLE IN THE BOX FLOOR. HOLES FOR DRAIN PIPE SHALL BE PLACED IN THE LOW CORNER OF THE BOX, WITH A MINIMUM CLEARANCE OF 1" BETWEEN THE EDGE OF THE HOLE AND THE INSIDE SURFACE OF THE BOX WALL. TYPICAL DETAILS ARE SHOWN ON THIS SHEET.
THE RIGID STEEL CONDUIT, JUNCTION BOXES AND FITTINGS INCLUDING LABOR AND ANY ADDITIONAL WORK TO DO THE INSTALLATION IS CONSIDERED INCIDENTAL TO THE COST OF THE RAILING.
STAINLESS-STEEL REINFORCEMENT SHALL NOT BE ALLOWED TO BE IN CONTACT WITH THE UNCOATED REINFORCEMENT, BARE METAL FORMING HARDWARE, OR TO GALVANIZED ATTACHMENTS OR GALVANIZED CONDUIT.



DESIGN FOR VARIABLE SKEW RADIUS = 1,820'
**214'-0 X 28'-0 PRETENSIONED
PRESTRESSED CONCRETE BEAM BRIDGE**
66'-0 & 71'-0 END SPANS 77'-0 INTERIOR SPAN
CONDUIT DETAILS
STA. 32591+41.72 (RAMP B) APRIL, 2022
POLK COUNTY
IOWA DEPARTMENT OF TRANSPORTATION
DESIGN SHEET NO. 27 OF 34 FILE NO. 31046 DESIGN NO. 2118

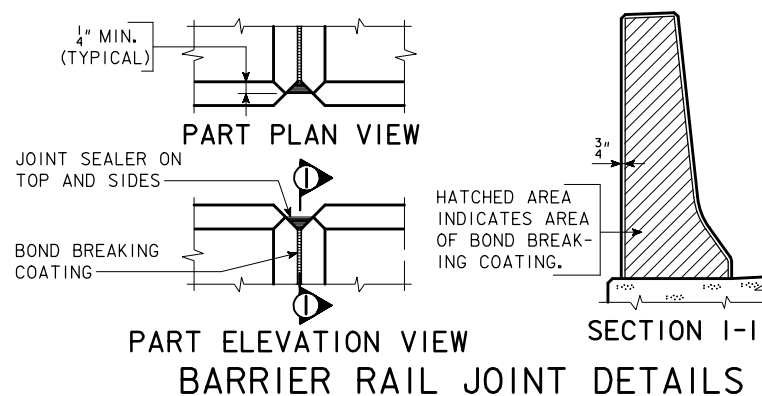




ELEVATION OF BARRIER RAIL

NOTE:
 MEASURED ALONG GUTTER LINE.

NOTE: REINFORCING STEEL QUANTITIES ARE INCLUDED ON THE SUMMARY QUANTITIES SHEET.



BARRIER RAIL NOTES:

MINIMUM CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REIN-
FORCING BAR IS TO BE 2" UNLESS OTHERWISE NOTED OR SHOWN.

THE PERMISSIBLE CONSTRUCTION JOINTS ARE TO BE PLACED BETWEEN
VERTICAL BARS AT A MINIMUM SPACING OF 20 FEET. CONSTRUCTION JOINT
CONTACT SURFACES ARE TO BE COATED WITH AN APPROVED BOND BREAKER.

COST OF THE JOINT SEALER AND BOND BREAKER SHALL BE CONSIDERED
INCIDENTAL TO OTHER CONSTRUCTION.

ALL BARRIER RAIL REINFORCING STEEL IS TO BE EITHER EPOXY COATED OR
STAINLESS STEEL AS SHOWN. THE STAINLESS STEEL REINFORCING STEEL SHALL BE
DEFORMED BAR GRADE 60 MEETING THE REQUIREMENTS OF MATERIALS I.M. 452.

THE CONCRETE BARRIER RAIL IS TO BE BID ON A LINEAL FOOT BASIS.

THE NUMBER OF LINEAL FEET OF BARRIER RAIL INSTALLED WILL BE PAID
FOR AT THE CONTRACT PRICE PER LINEAL FOOT BASED ON PLAN QUANTITIES.

PRICE BID FOR 3'-8 CONCRETE BARRIER RAILING SHALL BE FULL COMPENSATION
FOR FURNISHING ALL MATERIAL, EXCLUDING REINFORCING STEEL, AND ALL OF
THE EQUIPMENT AND LABOR REQUIRED TO ERECT THE RAIL IN ACCORDANCE
WITH THESE PLANS AND CURRENT SPECIFICATIONS. IF CONDUIT IS REQUIRED
IN THIS PLAN THE RIGID STEEL CONDUIT, JUNCTION BOXES AND FITTINGS
INCLUDING LABOR AND ANY ADDITIONAL WORK TO DO THE INSTALLATION IS
CONSIDERED INCIDENTAL TO THE COST OF THE RAILING.

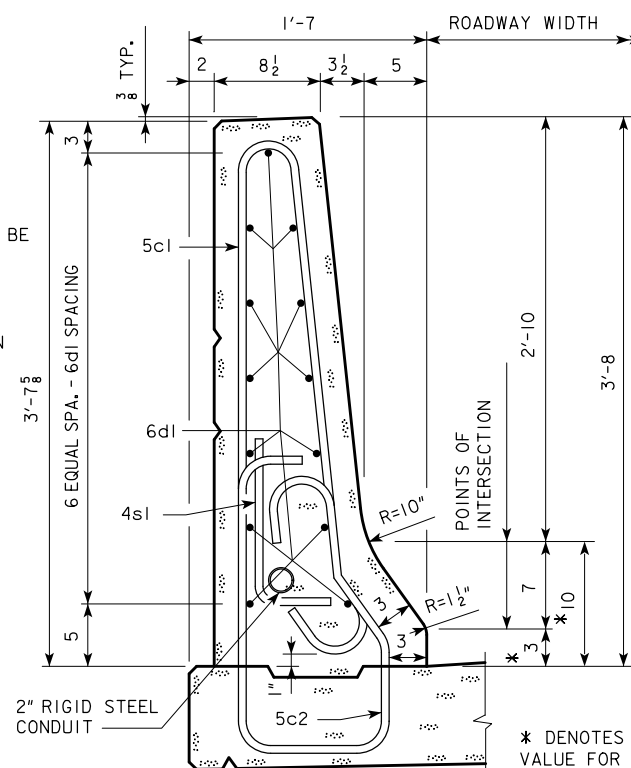
THE JOINT SEALER SHALL BE LIGHT GRAY NONSAG LATEX CAULKING SEALER
MARKETED FOR OUTDOOR USE. NO TESTING OR CERTIFICATION IS REQUIRED.

TOP OF THE BARRIER RAIL IS TO BE PARALLEL TO THE THEORETICAL \bar{C}
GRADE, EXCEPT AT THE SPECIAL SECTIONS.

CROSS SECTIONAL AREA OF THE STANDARD SECTION OF THE BARRIER RAIL =
3.46 SQUARE FEET EXCEPT THE 4'-0 SLOPED ENDS AT THE END SECTIONS.

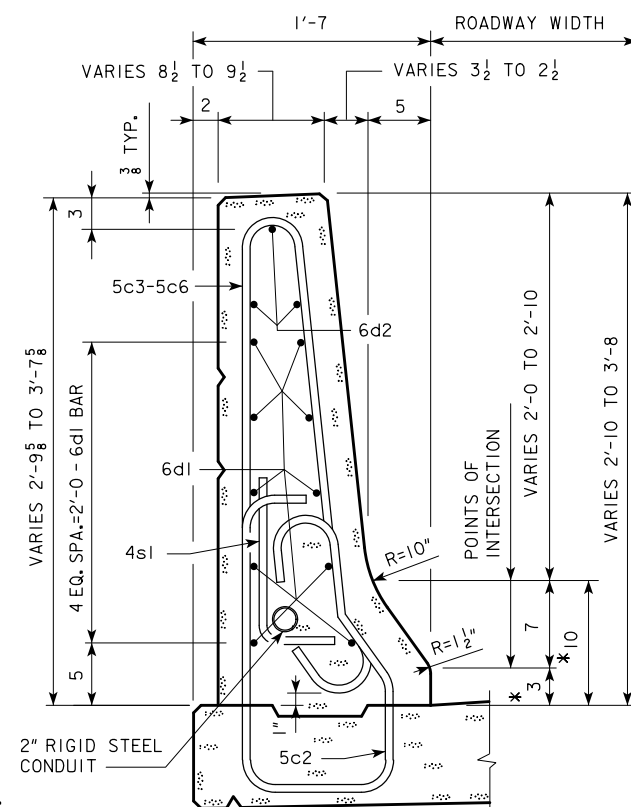
THE GALVANIZED STEEL CONDUIT SHALL BE SECURELY TIED AT EVERY 3'-0
INTERSECTION WITH THE 4S1 BARS TO AVOID CONTACT WITH THE STAINLESS
STEEL REINFORCING.

NOTE:
SEE BARRIER RAIL RUSTICATION
DETAILS ON DESIGN SHEET 30.



PART SECTION C-C

* DENOTES THE MAXIMUM
VALUE FOR THIS DIMENSION.
THIS DIMENSION MAY VARY DUE
TO CONSTRUCTION INACCURACIES.











PART SECTION F-F

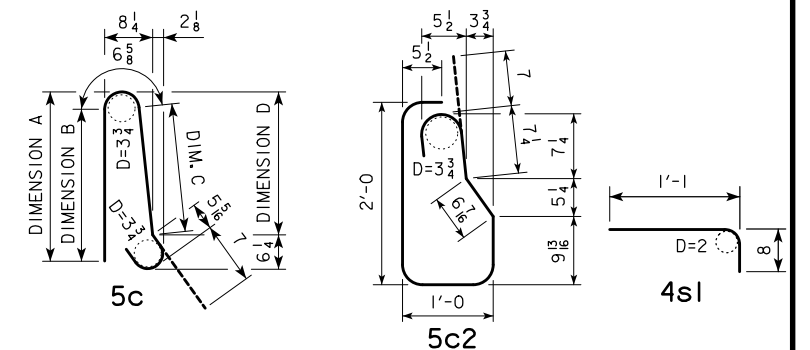
EPOXY COATED REINF. STEEL - TWO RAILS

SECTION	BAR	LOCATION	SHAPE	NO.	LENGTH	WEIGHT
STANDARD SECTIONS	4sI	RAIL, CONDUIT	┘	142	1'-9	166
EPOXY STEEL TOTAL (LBS.)						166

STAINLESS STEEL REINF. STEEL - TWO RAILS

SECTION	BAR	LOCATION	SHAPE	NO.	LENGTH	WEIGHT
STANDARD SECTIONS	5c1	RAIL, VERTICAL		418	7'-5	3,233
	5c2	RAIL, VERTICAL		434	6'-0	2,716
	5c3	RAIL, VERTICAL, SLOPED ENDS		4	7'-3	30
	5c4	RAIL, VERTICAL, SLOPED ENDS		4	6'-10	29
	5c5	RAIL, VERTICAL, SLOPED ENDS		4	6'-5	27
	5c6	RAIL, VERTICAL, SLOPED ENDS		4	6'-0	25
	6d1	RAIL, LONGITUDINAL		144	38'-4	8,291
	6d2	RAIL, LONGIT., TOP SLOPED ENDS		12	38'-5	692
STAINLESS STEEL TOTAL (LBS.)						15,043

BENT BAR DETAILS



5c BARS				
BAR	DIM. A	DIM. B	DIM. C	DIM. D
5c1	$3'-3\frac{3}{16}$	$3'-1\frac{5}{16}$	$2'-8\frac{1}{2}$	$2'-10\frac{1}{8}$
5c3	$3'-2\frac{9}{16}$	$3'-0$	$2'-7\frac{1}{4}$	$2'-9\frac{5}{16}$
5c4	$3'-0$	$2'-9\frac{9}{16}$	$2'-4\frac{3}{4}$	$2'-6\frac{7}{8}$
5c5	$2'-9\frac{9}{16}$	$2'-7\frac{1}{8}$	$2'-2\frac{1}{4}$	$2'-4\frac{3}{8}$
5c6	$2'-7\frac{1}{16}$	$2'-4\frac{9}{16}$	$1'-11\frac{7}{8}$	$2'-1\frac{7}{8}$

NOTE:
ALL DIMENSIONS ARE OUT TO OUT.
D = PIN DIAMETER.

CONCRETE PLACEMENT SUMMARY

SECTION		TOTAL
Δ	STANDARD SECTION 434'-0 @ 0.1281 CU. YD. PER FT.	55.6
	TOTAL (CU. YD.)	55.6

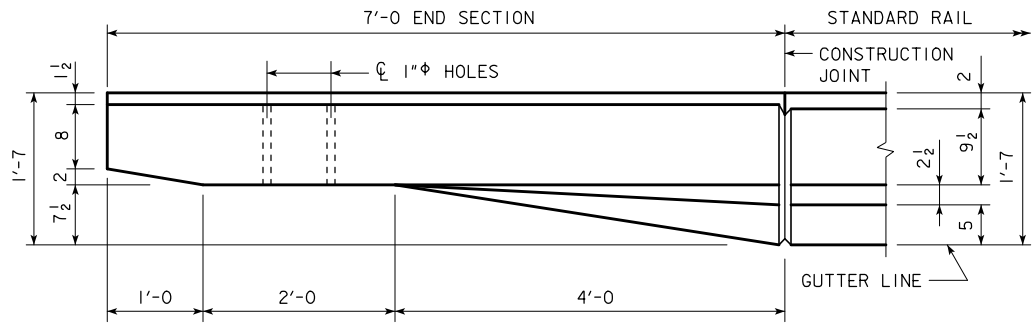
CONCRETE BARRIER RAIL QUANTITIES

ITEM	UNIT	QUANTITY
CONCRETE BARRIER RAILING, 3'-8	L.F.	462.0

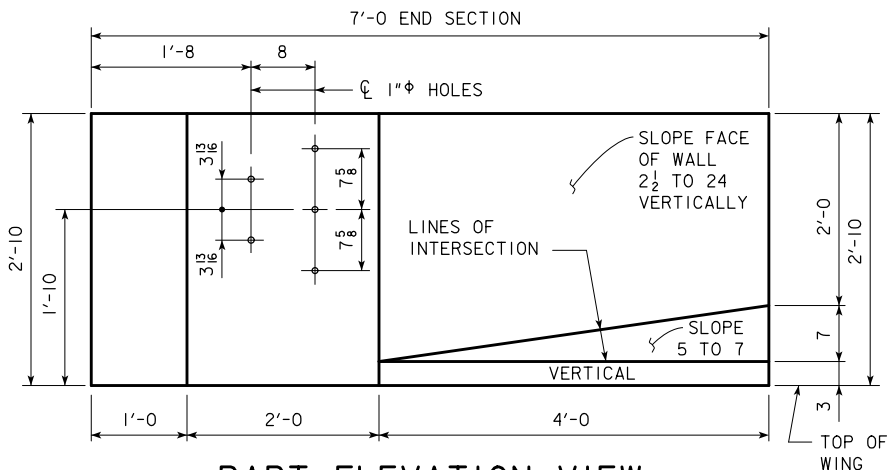
Δ DEDUCT 0.044 CU. YD. FOR ONE SLOPED END.

DESIGN FOR VARIABLE SKEW RADIUS = 1,820'
214'-0 X 28'-0 PRETENSIONED
PRESTRESSED CONCRETE BEAM BRIDGE
66'-0 & 71'-0 END SPANS 77'-0 INTERIOR SPAN
BARRIER RAIL DETAILS
STA. 32591+41.72 (RAMP B) APRIL, 2022
POLK COUNTY
IOWA DEPARTMENT OF TRANSPORTATION
DESIGN SHEET NO. 28 OF 34 FILE NO. 31046 DESIGN NO. 2118

ENGLISHDECKRAILBRIDGES.DGN 1017S - THIS SHEET ISSUED 04-14 - ADDED STAINLESS STEEL REINFORCING BAR LIST AND CHANGED 6c3, 6c4 & 5c5-10 BARS TO STAINLESS STEEL.

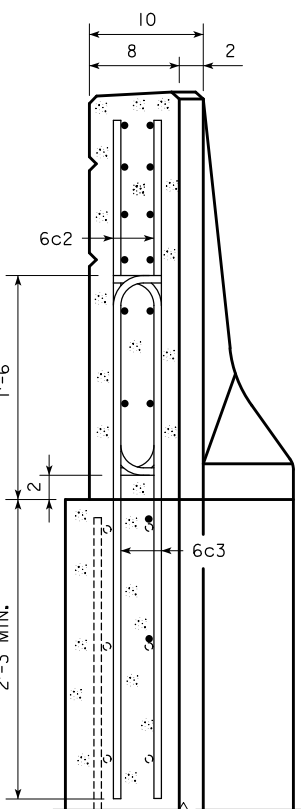


PART PLAN VIEW

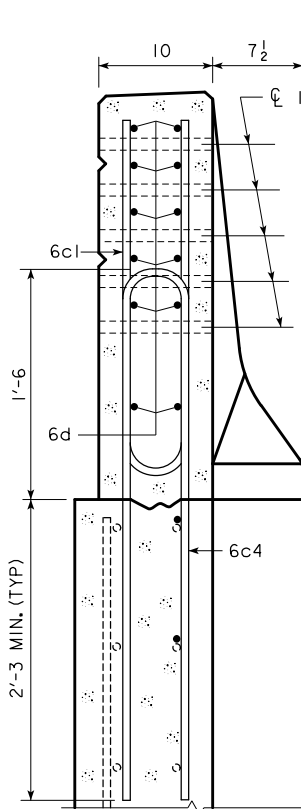


PART ELEVATION VIEW

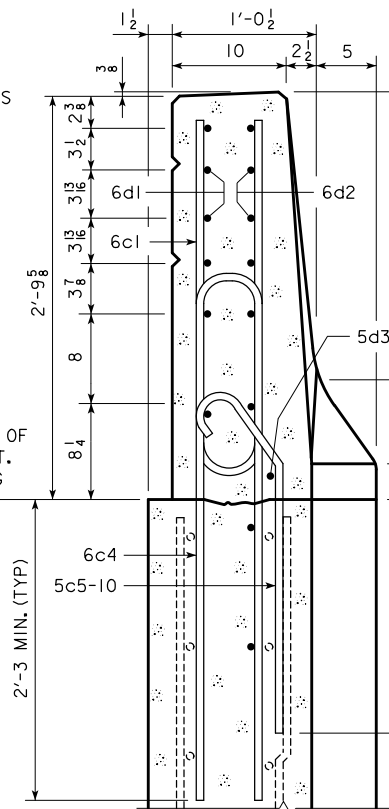
PROVIDE 5 HOLES FORMED WITH 1" ϕ PLASTIC CONDUIT. COST TO BE INCLUDED IN PRICE BID FOR CONCRETE BARRIER RAILING.



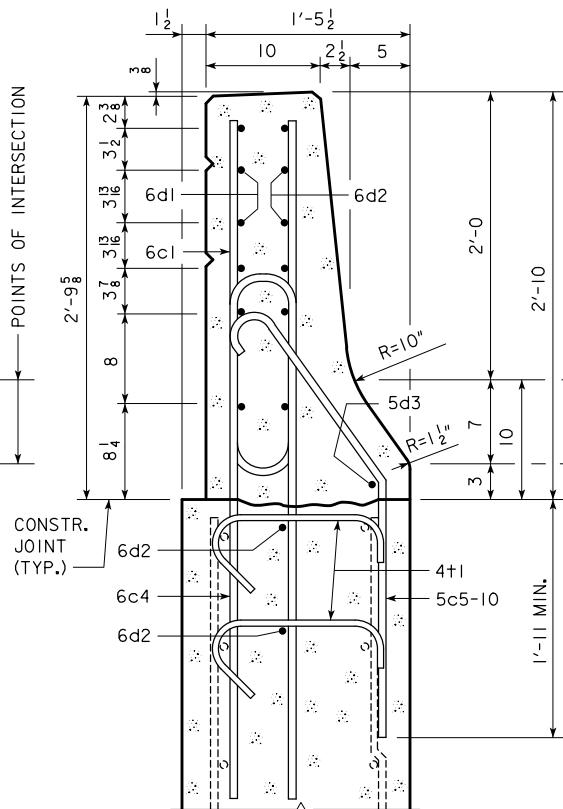
VIEW A-A



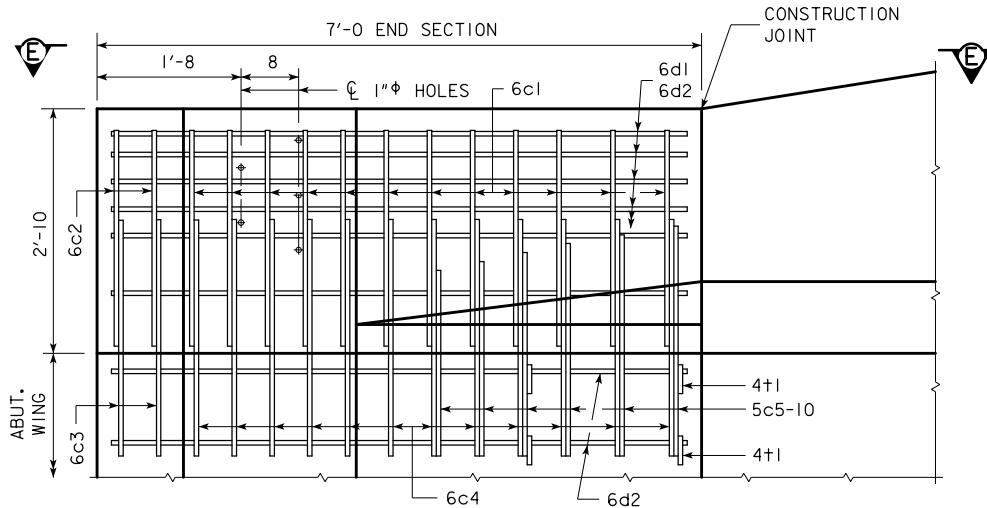
SECTION B-B



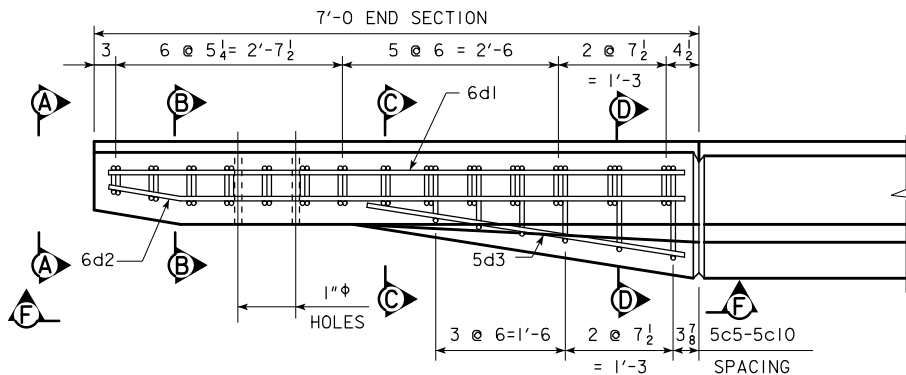
SECTION C-C



SECTION D-D



PART VIEW E-E



PART VIEW F-F

NOTE:
4+1 PLACEMENT - 2 BARS EACH LEVEL OF 6d2 IN WING FOOTING.

NOTE:
CONSTRUCTION JOINT BETWEEN TOP OF WING AND BARRIER RAIL IS ROUGHENED CONCRETE.

NOTE:
THE 10" RADIUS AND 1 1/2" RADIUS ARE TYPICAL AND SHALL BE USED WHEN CONSTRUCTING THE CORNERS FOR VIEW A-A, SECTION B-B, SECTION C-C AND SECTION D-D.

NOTE:
THE 6c4, 6c3, 5c5-10, 2 - 6d2 AND 4+1 BARS ARE TO BE PLACED WITH THE ABUTMENT WING. THE DETAILS FOR PLACEMENT ARE SHOWN ON THE WING ABUTMENT SHEET.

NOTE:
DASHED LINES BELOW THE TOP OF WING ARE THE ABUTMENT WING REINFORCING STEEL. SEE WING ABUTMENT SHEET FOR PLACEMENT.

NOTE:
SEE BARRIER RAIL RUSTICATION DETAILS ON DESIGN SHEET 30.

STAINLESS STEEL REINF. STEEL - ONE END SECT.

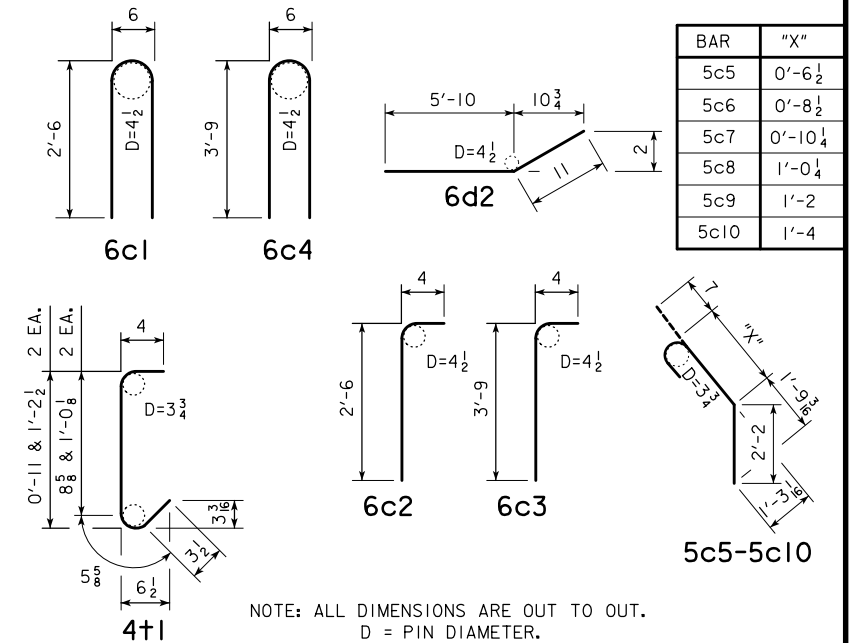
BAR	LOCATION	SHAPE	NO.	LENGTH	WEIGHT
6c1	RAIL, VERTICAL		12	5'-6	99
6c2	RAIL, VERTICAL		4	2'-10	17
6c3	RAIL, VERTICAL		4	4'-1	25
6c4	RAIL, VERTICAL		12	8'-0	144
5c5-10	RAIL, VERTICAL		6	VARIES	23
6d1	RAIL, HORIZONTAL		6	6'-8	60
6d2	RAIL, HORIZONTAL		8	6'-9	81
5d3	RAIL, HORIZONTAL		1	3'-9	4
4+1	RAIL, ABUTMENT WING TIE BARS		4	VARIES	5
STAINLESS STEEL TOTAL WEIGHT (LBS.)					458

NOTE: REINFORCING STEEL QUANTITIES ARE INCLUDED ON THE SUMMARY QUANTITIES SHEET.

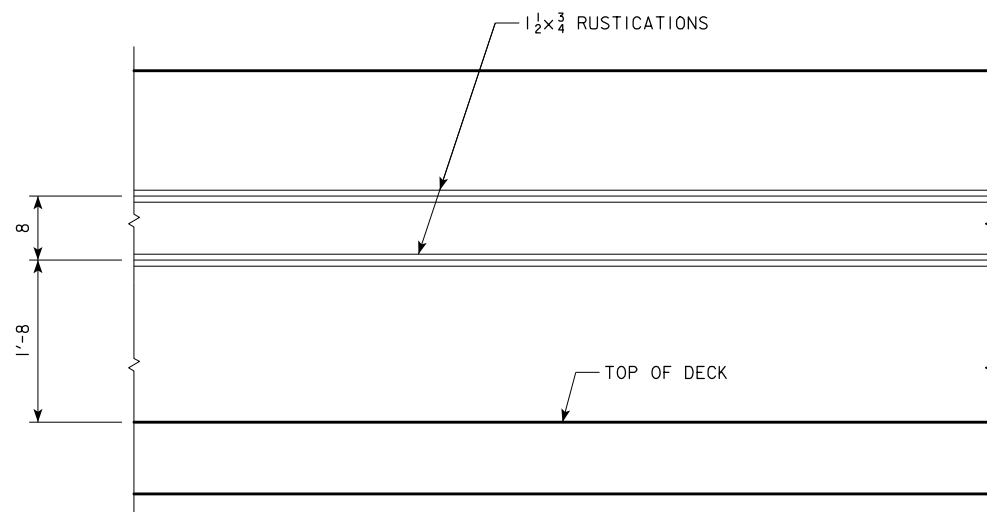
CONCRETE PLACEMENT SUMMARY

SECTION	TOTAL
BARRIER RAIL ONE END SECTION	0.65 CU. YD.

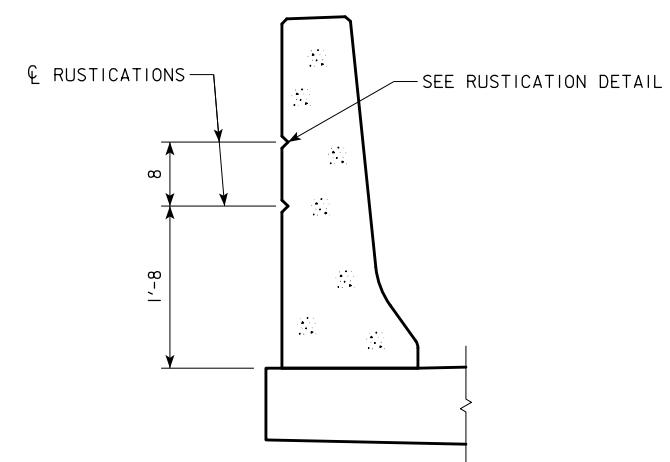
BENT BAR DETAILS



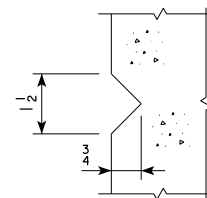
DESIGN FOR VARIABLE SKEW RADIUS = 1,820'
214'-0 X 28'-0 PRETENSIONED
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BARRIER RAIL END SECTION DETAILS
STA. 32591+41.72 (RAMP B) APRIL, 2022
POLK COUNTY
IOWA DEPARTMENT OF TRANSPORTATION
DESIGN SHEET NO. 29 OF 34 FILE NO. 31046 DESIGN NO. 2118



PART OUTSIDE ELEVATION OF
STANDARD BARRIER RAIL



PART SECTION THROUGH
STANDARD BARRIER RAIL



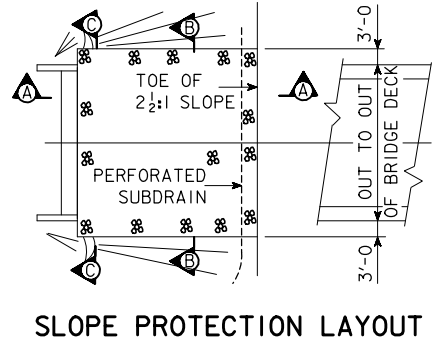
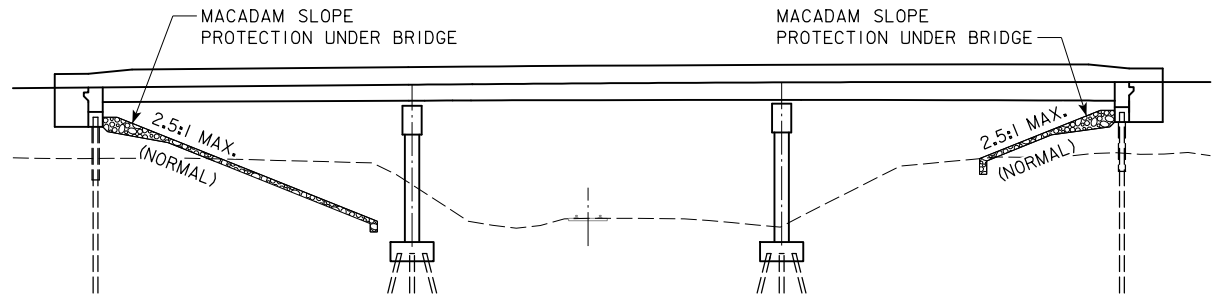
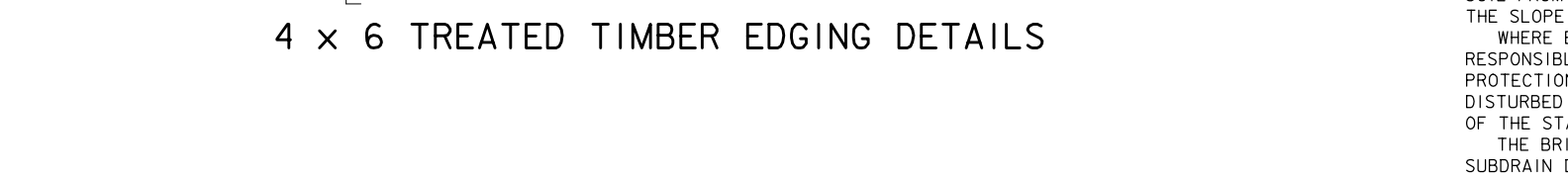
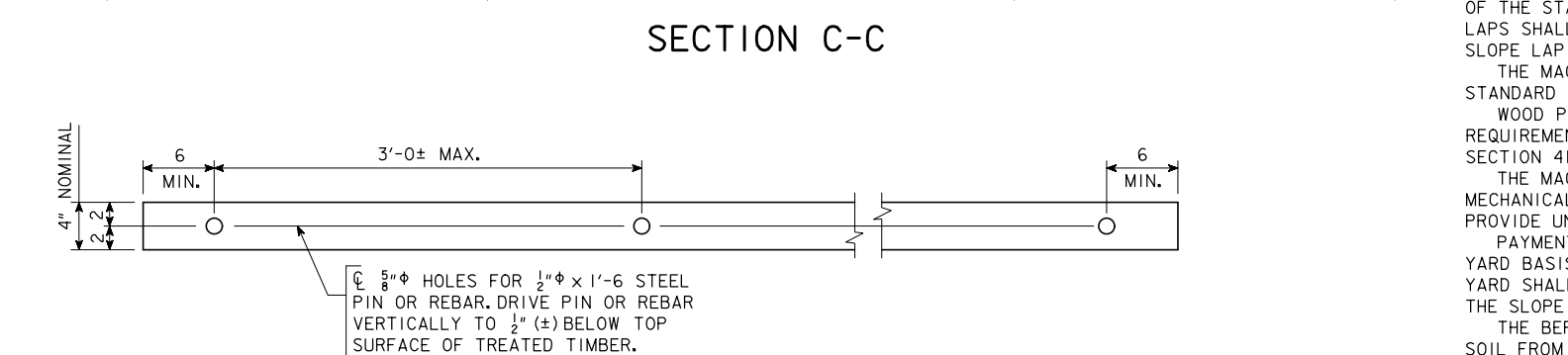
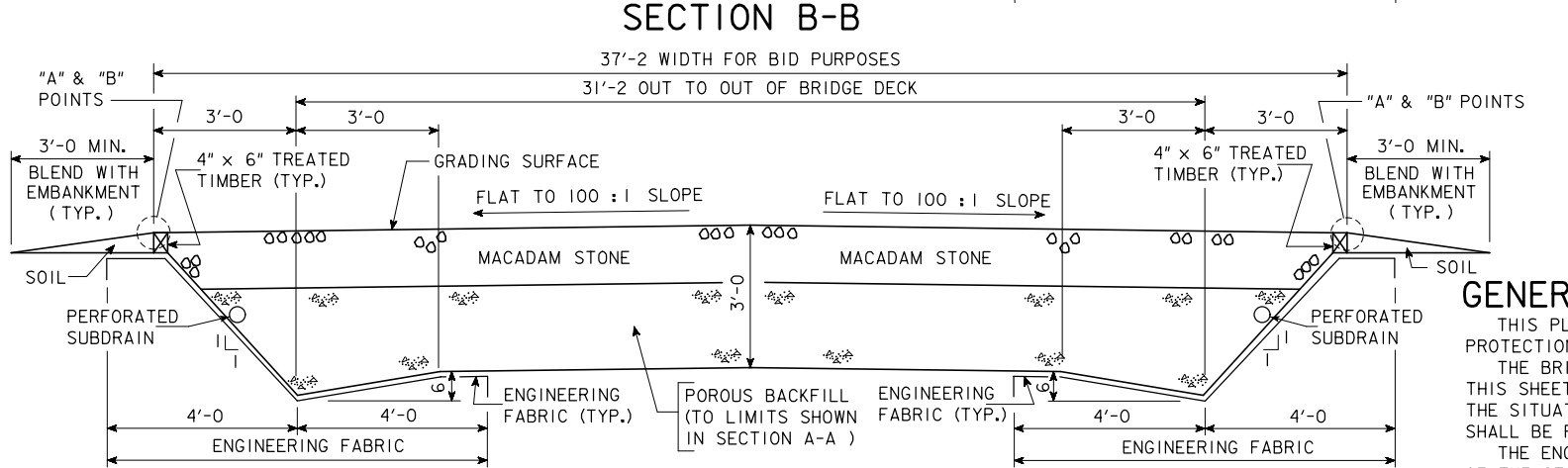
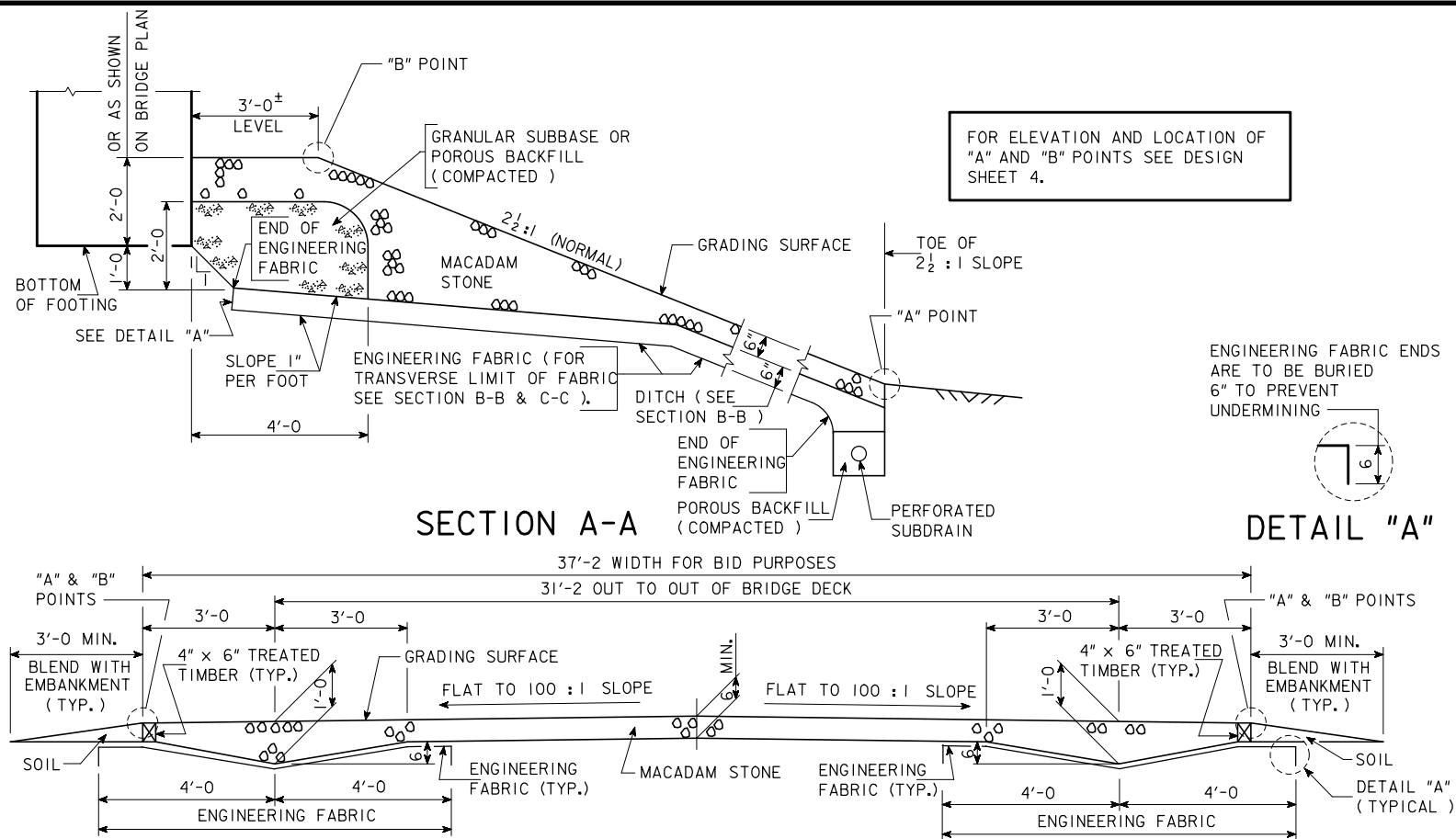
RUSTICATION DETAIL

NOTES:
SEE GENERAL NOTES SHEET FOR RUSTICATION NOTES.
RUSTICATIONS EXTEND THE FULL LENGTH OF BRIDGE
BARRIER, INCLUDING BARRIER END SECTION.

DESIGN FOR VARIABLE SKEW RADIUS = 1,820'
214'-0 X 28'-0 PRETENSIONED
PRESTRESSED CONCRETE BEAM BRIDGE
66'-0 & 71'-0 END SPANS 77'-0 INTERIOR SPAN
BARRIER RAIL RUSTICATION DETAILS
STA. 32591+41.72 (RAMP B) APRIL, 2022
POLK COUNTY
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REVISED 10-12 - LOCATED THE "A" AND "B" POINTS IN SECTION A-A AND CURB & ALTERNATE CURB DETAILS.
ENGLISH FOR SLOPE PROTECTION BRIDGES.DGN 1006D - THIS SHEET ISSUED 9-16-92



GENERAL NOTES:

THIS PLAN SHEET SHOWS DETAILS FOR PLACING A "MACADAM STONE SLOPE PROTECTION" UNDER OVERHEAD STRUCTURES.

THE BRIDGE BERM FORESLOPE SHALL BE COMPACTED AND SHAPED AS SHOWN ON THIS SHEET, SHAPING WILL INCLUDE EXCAVATION, FROM THE GRADING SURFACE SHOWN, THE SITUATION PLAN, AND AS DIRECTED BY THE ENGINEER. THE BERM FORESLOPE SHALL BE FIRM WHEN THE ENGINEERING FABRIC AND MACADAM STONE ARE PLACED.

THE ENGINEERING FABRIC SHALL BE IN ACCORDANCE WITH ARTICLE 4196.01, B, 3, OF THE STANDARD SPECIFICATIONS. IF THE ENGINEERING FABRIC IS LAPPED, THE LAPS SHALL BE A MINIMUM OF ONE FOOT IN LENGTH, SHINGLE FASHION WITH UP SLOPE LAP PIECE ON TOP AND STAPLED FOR CONTINUITY.

THE MACADAM STONE SHALL BE IN ACCORDANCE WITH SECTION 4122, OF THE STANDARD SPECIFICATIONS, COARSE MATERIAL (NO CHOKO STONE IS ALLOWED).

WOOD PRESERVATIVE TREATMENT FOR THE TIMBER EDGING SHALL MEET THE REQUIREMENTS FOR GUARDRAIL POSTS, SAWED FOUR SIDES, IN ACCORDANCE WITH SECTION 4161, OF THE STANDARD SPECIFICATIONS.

THE MACADAM STONE SHALL BE DEPOSITED, SPREAD, CONSOLIDATED AND SHAPED BY MECHANICAL OR HAND METHODS THAT WILL PROVIDE UNIFORM DEPTH AND DENSITY AND PROVIDE UNIFORM SURFACE APPEARANCE.

PAYMENT FOR "MACADAM STONE SLOPE PROTECTION" WILL BE MADE ON A SQUARE YARD BASIS FOR SLOPE PROTECTION CONSTRUCTED. THE UNIT PRICE BID PER SQUARE YARD SHALL INCLUDE ALL COSTS FOR MATERIAL AND LABOR REQUIRED TO CONSTRUCT THE SLOPE PROTECTION SHOWN ON THESE PLANS.

THE BERM FORESLOPE SHAPING AND COMPACTING AND THE DISPOSAL OF EXCESS SOIL FROM SHAPING OR TRENCHING SHALL BE CONSIDERED INCIDENTAL TO PLACING THE SLOPE PROTECTION.

WHERE EROSION CONTROL WORK HAS BEEN COMPLETED THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY PLANT MATERIALS DESTROYED ADJACENT TO THE SLOPE PROTECTION AREA. THE CONTRACTOR SHALL REPLANT, RESEED AND REMULCH ALL DISTURBED AREAS, DESIGNATED BY THE ENGINEER, IN ACCORDANCE WITH SECTION 2601, OF THE STANDARD SPECIFICATIONS, AT THE CONTRACTOR'S EXPENSE.

THE BRIDGE CONTRACTOR IS TO INSTALL SUBDRAINS AS DETAILED ON THE SUBDRAIN DETAILS SHEET.

ESTIMATED QUANTITIES

DESCRIPTION	LOCATION	QUANTITY
MACADAM STONE SLOPE PROTECTION	SOUTH ABUT.	140 SQ. YDS.
MACADAM STONE SLOPE PROTECTION	NORTH ABUT.	250 SQ. YDS.
TOTAL		390 SQ. YDS.

ITEMS TO BE INCLUDED IN "MACADAM STONE SLOPE PROTECTION":

- EXCAVATING, SHAPING AND COMPACTING
- ENGINEERING FABRIC
- MACADAM STONE
- 4" x 6" TREATED TIMBER EDGING
- 1/2" STEEL PINS (OR REBARS)
- POROUS BACKFILL OR GRANULAR SUBBASE BACKFILL AT FRONT FACE ABUTMENT FOOTING

DESIGN FOR VARIABLE SKEW RADIUS = 1,820'

**214'-0 X 28'-0 PRETENSIONED
PRESTRESSED CONCRETE BEAM BRIDGE**

66'-0 & 71'-0 END SPANS 77'-0 INTERIOR SPAN

MACADAM STONE SLOPE PROTECTION

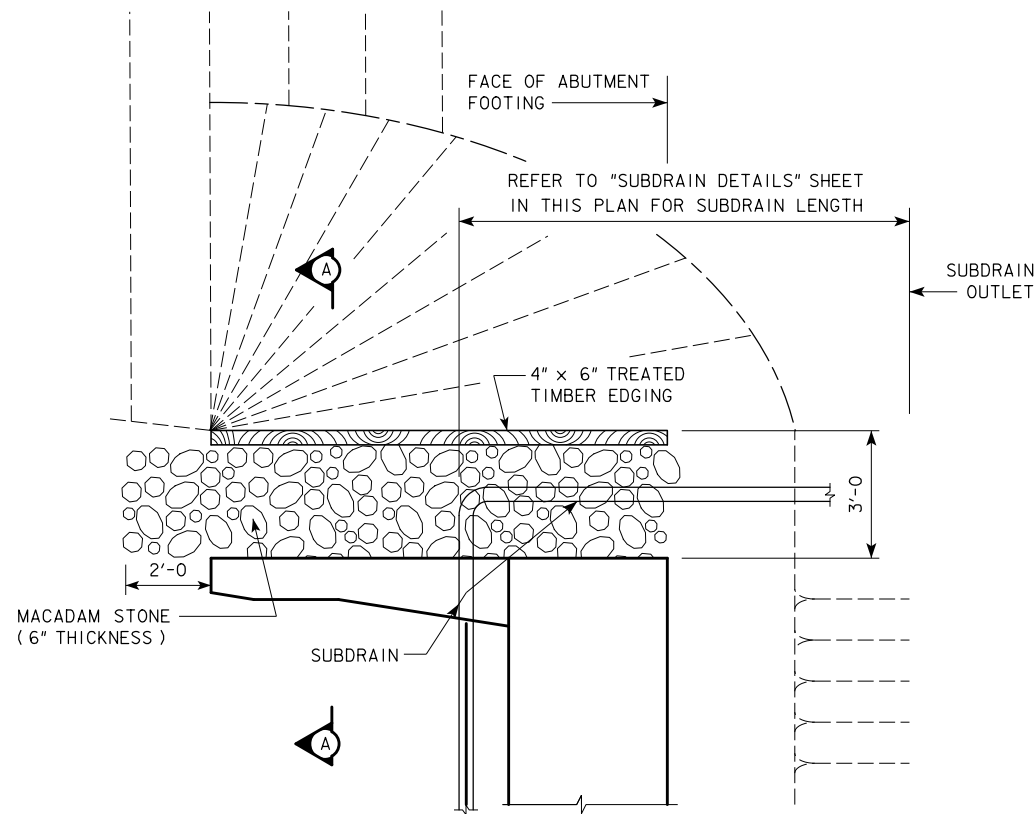
STA. 32591+41.72 (RAMP B) APRIL, 2022

POLK COUNTY

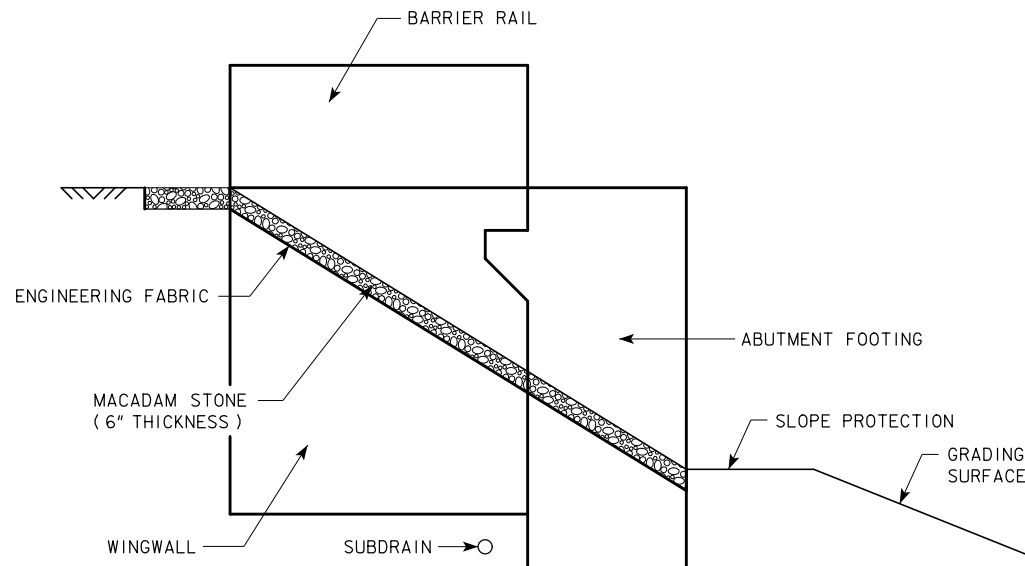
IOWA DEPARTMENT OF TRANSPORTATION

DESIGN SHEET NO. 31 OF 34 FILE NO. 31046 DESIGN NO. 2118

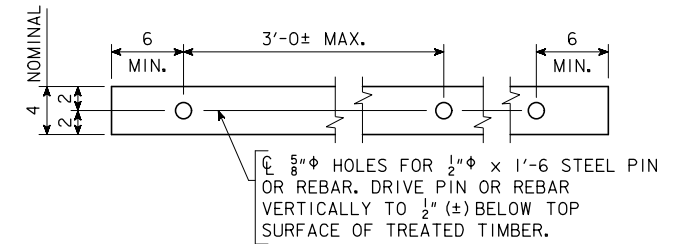
REVISED 06-14 - ADDED 2 FEET OF LENGTH OF MACADAM STONE IN FRONT OF THE BRIDGE WING.
ENGLISHFORSLOPEPROTECTIONBRIDGES.DGN 1005 - THIS SHEET ISSUED 06-02.



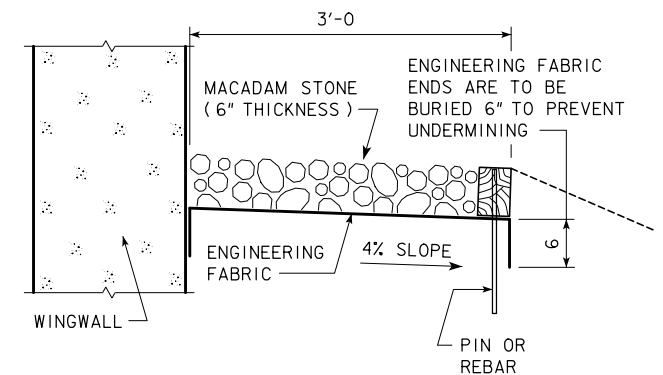
TOP VIEW OF WING ARMORING



PROFILE VIEW OF WING ARMORING



4" x 6" TREATED TIMBER EDGING DETAILS



SECTION A-A

GENERAL NOTES:

MACADAM STONE SHALL BE PLACED ALONG THE SIDE OF THE WING AND ABUTMENT FOOTING AS SHOWN IN SECTION A-A. THIS IS TYPICAL AT EACH CORNER OF THE BRIDGE UNLESS OTHERWISE NOTED IN THE PLANS. THE MACADAM STONE AT THESE LOCATIONS SHALL BE UNDERLAYED WITH ENGINEERING FABRIC IN ACCORDANCE WITH ARTICLE 4196.01, B, 3, OF THE STANDARD SPECIFICATIONS.

THE MACADAM STONE SHALL BE IN ACCORDANCE WITH SECTION 4122, OF THE STANDARD SPECIFICATIONS, COARSE MATERIAL (NO CHOKE STONE IS ALLOWED).

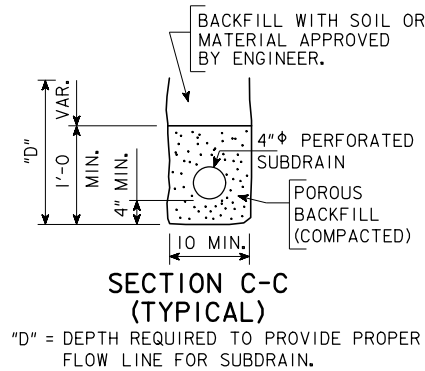
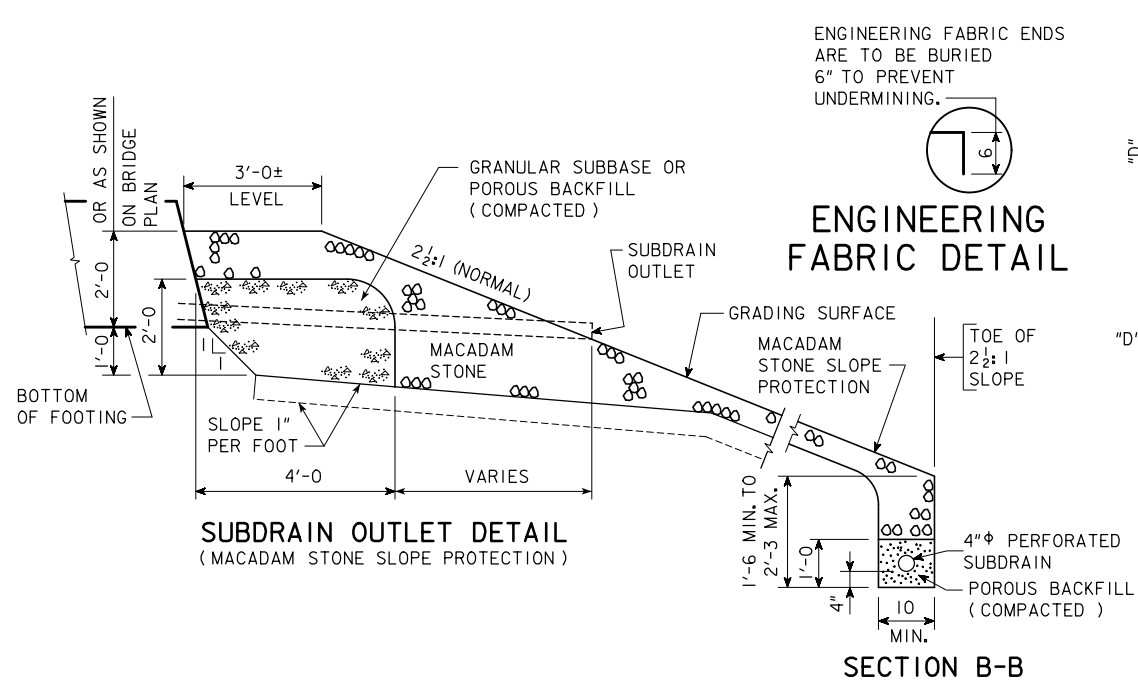
WOOD PRESERVATIVE TREATMENT FOR THE TIMBER EDGING SHALL MEET THE REQUIREMENTS FOR GUARDRAIL POSTS, SAWED FOUR SIDES, IN ACCORDANCE WITH SECTION 4161, OF THE STANDARD SPECIFICATIONS.

THE MACADAM STONE SHALL BE DEPOSITED, SPREAD, CONSOLIDATED AND SHAPED BY MECHANICAL OR HAND METHODS THAT WILL PROVIDE UNIFORM 6" DEPTH AND DENSITY AND PROVIDE UNIFORM SURFACE APPEARANCE.

PAYMENT FOR THE BRIDGE WING ARMORING WILL BE BID PER SQUARE YARD. COST WILL INCLUDE ENGINEERING FABRIC, MACADAM STONE, TREATED TIMBER EDGING, EXCAVATION, SHAPING, AND COMPACTION TO DIMENSIONS SHOWN IN THESE PLANS. BID ITEM SHALL BE "BRIDGE WING ARMORING - MACADAM STONE."

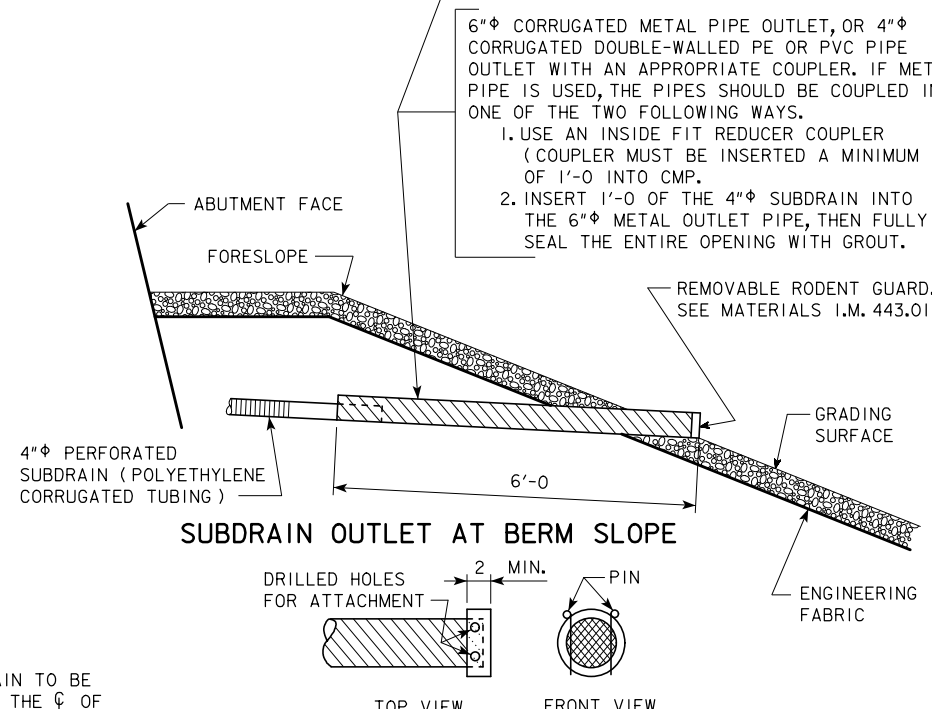
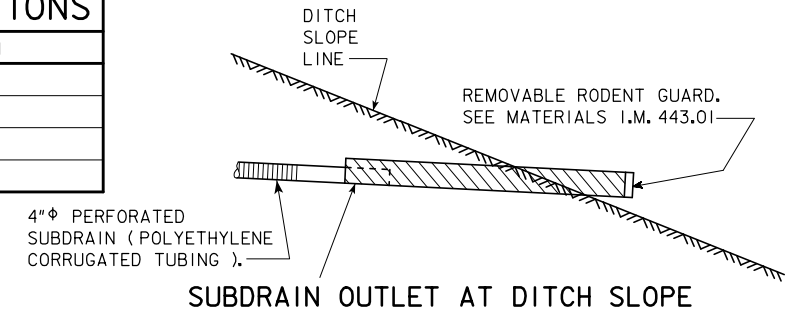
DESIGN FOR VARIABLE SKEW		RADIUS = 1,820'	
214'-0 X 28'-0 PRETENSIONED			
PRESTRESSED CONCRETE BEAM BRIDGE			
66'-0 & 71'-0 END SPANS		77'-0 INTERIOR SPAN	
BRIDGE WING ARMORING			
STA. 32591+41.72 (RAMP B)		APRIL, 2022	
POLK COUNTY			
IOWA DEPARTMENT OF TRANSPORTATION			
DESIGN SHEET NO. 32 OF 34		FILE NO. 31046	
		DESIGN NO. 2118	

BENCH MARK NO. 26: NORTHING: 604283.735, EASTING: 1624489.540, ELEV. 886.700, FENO. MONUMENT, W/CAP STAMPED "026", 6" DEEP, IN THE NORTH ROW/SHOULDER OF NE 54TH AVE., 9' NORTH OF EDGE AC SLAB; 128' FROM P.I. OF NE 54TH AVE. WITH OF RR CROSSING.



SUBDRAIN OUTLET ELEVATIONS

LOCATION	ELEVATION
NORTH ABUTMENT	952.12
TOE OF NORTH BERM	931.50
SOUTH ABUTMENT	953.16
TOE OF SOUTH BERM	942.50



**REMOVABLE RODENT GUARD DETAILS
OUTLET DETAILS**

DESIGN FOR VARIABLE SKEW RADIUS = 1,820'

**214'-0 X 28'-0 PRETENSIONED
PRESTRESSED CONCRETE BEAM BRIDGE**

66'-0 & 71'-0 END SPANS 77'-0 INTERIOR SPAN

SUBDRAIN DETAILS

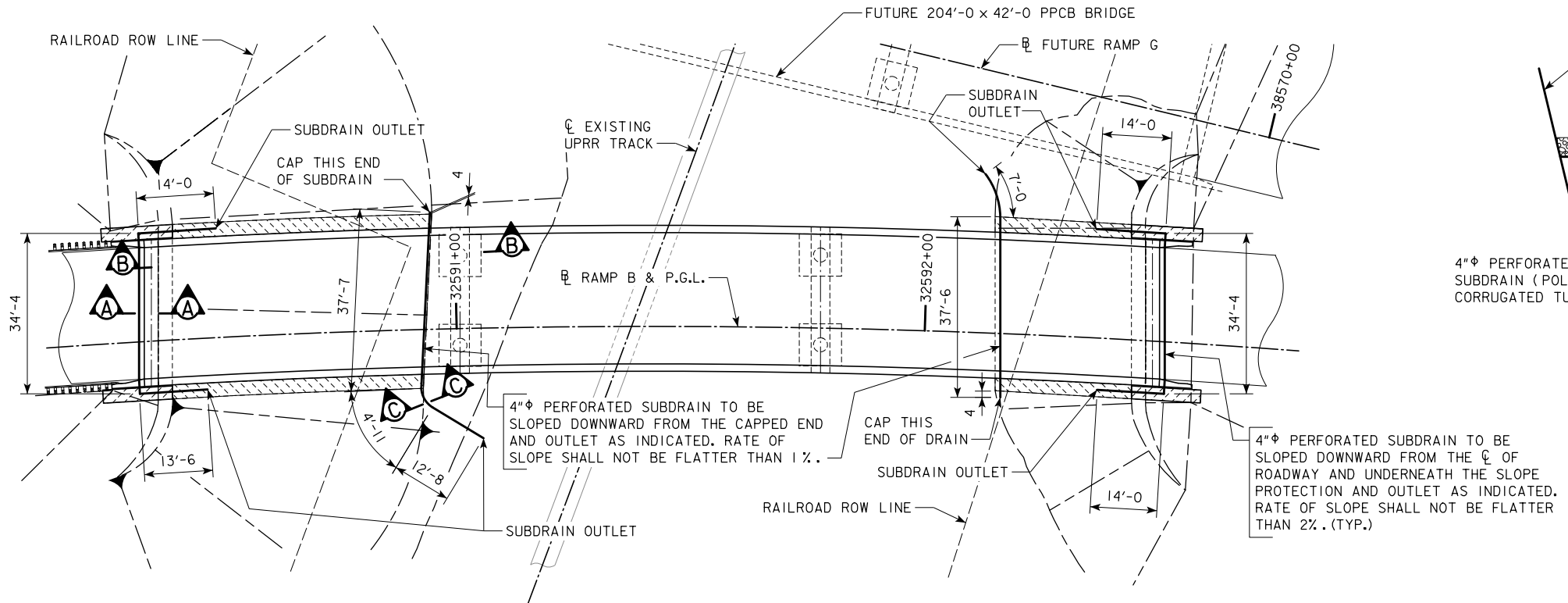
STA. 32591+41.72 (RAMP B)

POLK COUNTY

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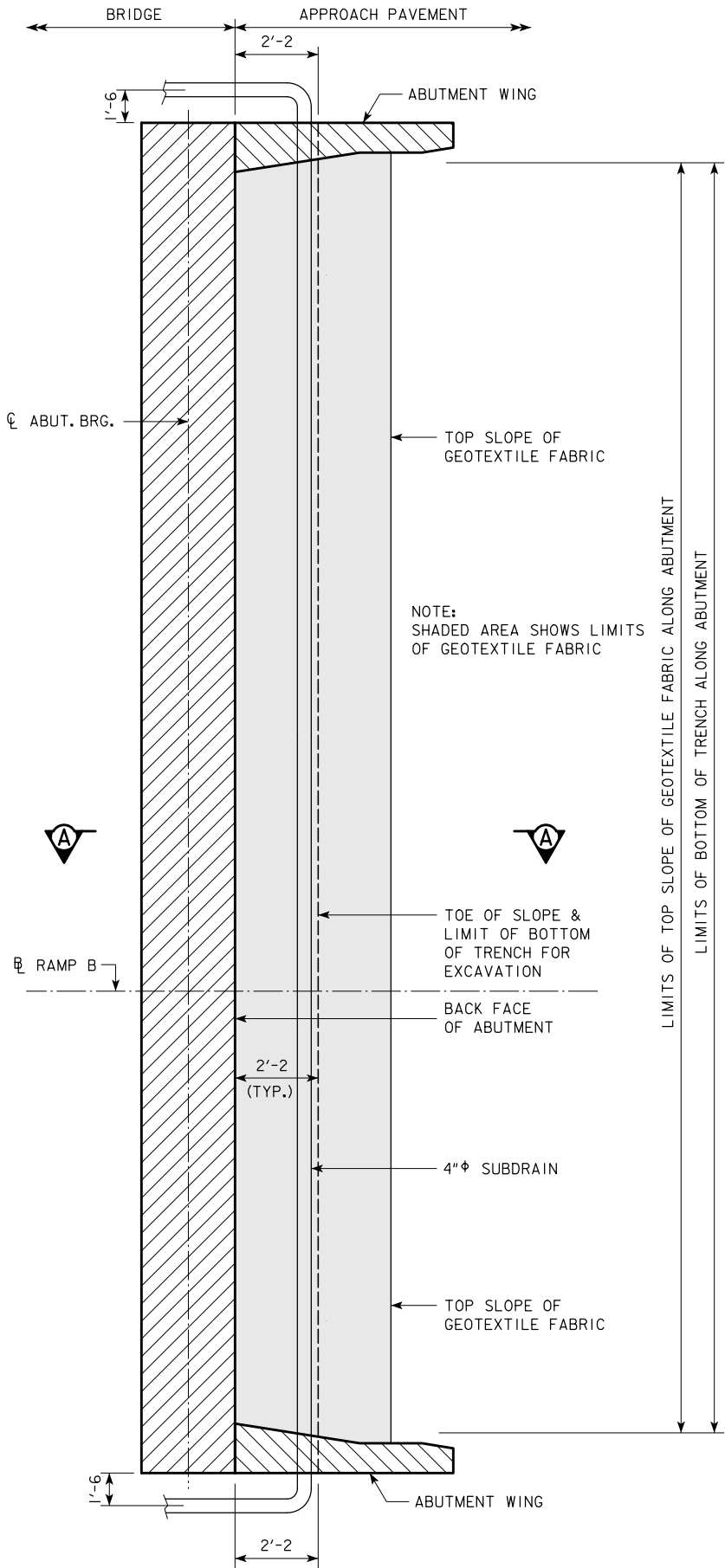
APRIL, 2022



NOTE:
SECTION A-A IS SHOWN ON ABUTMENT
BACKFILL DETAILS SHEET.

REVISED 07-11 - THE BERM SLOPE IS IDENTIFIED AS THE GRADING SURFACE. ENGLISH FOR SLOPE PROTECTION BRIDGES.DGN 1007A - THIS SHEET ISSUED 06-02.

REVISED 09-14 - THE TECHNICAL DATA INFORMATION TABLE WAS REMOVED AND IS LOCATED IN THE STANDARD SPECIFICATIONS. CHANGED SURFACE FLOODING TIME TO 5 MINUTE INCREMENTS.
REVISED 09-2016 - CHANGED THE BRIDGE APPROACH PAVEMENT STANDARD TO "BR" (WAS "RK").
ENGLISHFORSLOPEPROTECTIONBRIDGES.DGN - 1007D - THIS SHEET ISSUED 08-07.



ABUTMENT PLAN WITHOUT WING EXTENSIONS

ABUTMENT BACKFILL PROCESS:

THE BASE OF THE EXCAVATION SUBGRADE BEHIND THE ABUTMENT IS TO BE GRADED WITH A 4% SLOPE AWAY FROM THE ABUTMENT FOOTING AND A 2% CROSS SLOPE IN THE DIRECTION OF THE SUBDRAIN OUTLET. THIS EXCAVATION SHAPING IS TO BE DONE PRIOR TO BEGINNING INSTALLATION OF THE GEOTEXTILE AND BACKFILL MATERIAL.

AFTER THE SUBGRADE HAS BEEN SHAPED, THE GEOTEXTILE FABRIC SHALL BE INSTALLED IN ACCORDANCE WITH THE DETAILS SHOWN. THE FABRIC IS INTENDED TO BE INSTALLED IN THE BASE OF THE EXCAVATION AND EXTENDED VERTICALLY UP THE ABUTMENT BACKWALL, ABUTMENT WING WALLS, AND EXCAVATION FACE TO A HEIGHT THAT WILL BE APPROXIMATELY 1 TO 2 FOOT HIGHER THAN THE HEIGHT OF THE POROUS BACKFILL PLACEMENT AS SHOWN IN THE "BACKFILL DETAILS" ON THIS SHEET. THE STRIPS OF THE FABRIC PLACED SHALL OVERLAP APPROXIMATELY 1 FOOT AND SHALL BE PINNED IN PLACE. THE FABRIC SHALL BE ATTACHED TO THE ABUTMENT BY USING LATH FOLDED IN THE FABRIC AND SECURED TO THE CONCRETE WITH SHALLOW CONCRETE NAILS. THE FABRIC PLACED AGAINST THE EXCAVATION FACE SHALL BE PINNED.

WHEN THE FABRIC IS IN PLACE, THE SUBDRAIN SHALL BE INSTALLED DIRECTLY ON THE FABRIC AT THE TOE OF THE REAR EXCAVATION SLOPE. A SLOT WILL NEED TO BE CUT IN THE FABRIC AT THE POINT WHERE THE SUBDRAIN EXITS THE FABRIC NEAR THE END OF THE ABUTMENT WING WALL.

POROUS BACKFILL IS THEN PLACED AND LEVELED, NO COMPACTION IS REQUIRED.

THE REMAINING WORK INVOLVES BACKFILLING WITH FLOODABLE BACKFILL, SURFACE FLOODING, AND VIBRATORY COMPACTION. THE FLOODABLE BACKFILL MATERIAL SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. THE FLOODABLE BACKFILL SHALL BE PLACED IN INDIVIDUAL LIFTS, SURFACE FLOODED, AND COMPACTED WITH VIBRATORY COMPACTION TO ENSURE FULL CONSOLIDATION. LIMIT THE LOOSE LIFTS TO NO MORE THAN 2 FEET OF THICKNESS.

START SURFACE FLOODING FOR EACH FLOODABLE BACKFILL LIFT AT THE HIGH POINT OF THE SUBDRAIN AND PROGRESS TO THE LOW POINT WHERE THE SUBDRAIN EXITS THE FABRIC. TO ENSURE UNIFORM SURFACE FLOODING, WATER RUNNING FULL IN A 2-INCH DIAMETER HOSE SHOULD BE SPRAYED IN SUCCESSIVE 6-FOOT TO 8-FOOT INCREMENTS FOR 5 MINUTES WITHIN EACH INCREMENT.

FLOODABLE BACKFILL LIFT PLACEMENT, FLOODING, AND COMPACTION SHALL PROGRESS UNTIL THE REQUIRED FULL THICKNESS OF THE ABUTMENT BACKFILL HAS BEEN COMPLETED.

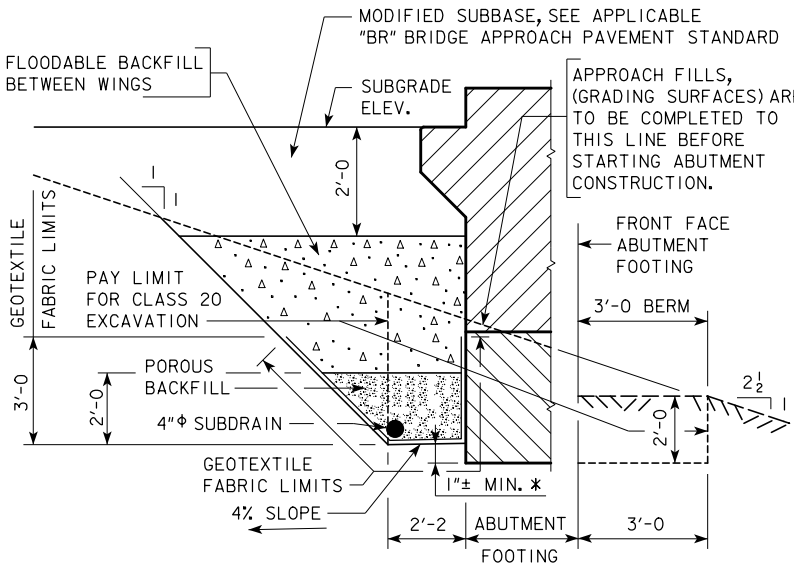
WATER REQUIRED FOR FLOODING, SUBDRAINS, POROUS BACKFILL, FLOODABLE BACKFILL, AND GEOTEXTILE FABRIC FURNISHED AT THE BRIDGE ABUTMENTS WILL NOT BE MEASURED SEPARATELY FOR PAYMENT.

THE COST OF WATER REQUIRED FOR FLOODING, SUBDRAINS, POROUS BACKFILL, FLOODABLE BACKFILL, AND GEOTEXTILE FABRIC FURNISHED AT THE BRIDGE ABUTMENTS SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID FOR STRUCTURAL CONCRETE.

NOTES:

SUBDRAIN SHALL SLOPE DOWNWARD 2% FROM CL APPROACH ROADWAY WHEN OUTLETTING BOTH SIDES OF THE ABUTMENT.

THE GEOTEXTILE FABRIC SHALL BE IN ACCORDANCE WITH ARTICLE 4196.01, B, 6 OF THE STANDARD SPECIFICATIONS. IF THE ENGINEERING FABRIC IS LAPPED THE LAPS SHALL BE A MINIMUM OF ONE FOOT IN LENGTH, SHINGLE FASHION WITH UP SLOPE LAP PIECE ON TOP AND STAPLED FOR CONTINUITY.



SECTION A-A
BACKFILL DETAILS

NOTE: GEOTEXTILE FABRIC WILL BE ATTACHED TO FACE OF ABUTMENT FOOTING AND WINGS.

* DIMENSION VARIES DUE TO 2% SUBDRAIN SLOPE.

NOTE:
SEE SUBDRAIN DETAILS SHEET FOR DETAILS NOT SHOWN ON THIS SHEET WHICH ARE PERTINENT TO THIS STRUCTURE.

DESIGN FOR VARIABLE SKEW RADIUS = 1,820'

214'-0 X 28'-0 PRETENSIONED
PRESTRESSED CONCRETE BEAM BRIDGE

66'-0 & 71'-0 END SPANS 77'-0 INTERIOR SPAN

ABUTMENT BACKFILL DETAILS

STA. 32591+41.72 (CL RAMP B) APRIL, 2022

POLK COUNTY

IOWA DEPARTMENT OF TRANSPORTATION

DESIGN SHEET NO. 34 OF 34 FILE NO. 31046 DESIGN NO. 2118