Solution Algorithms for Nonlinear Problems

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Outline

Consistent linearization

Newton and modified Newton methods

Line search

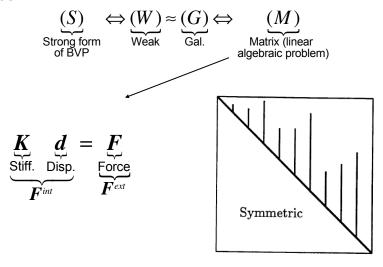
Quasi-Newton methods ("BFGS")

Arc-length methods

Convergence criteria

Brief Overview of Linear Finite Element Analysis

Statics



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Dynamics

$$\underbrace{(S)}_{\text{Strong form of IBVP}} \Leftrightarrow \underbrace{(W)}_{\text{t-cont.}} \approx \underbrace{(G)}_{\text{semi-}} \Leftrightarrow \underbrace{(M)}_{\text{o.d.e.'s}} \approx \underbrace{\text{stepping algorithm}}_{\text{oliscretization}}$$

+ update formulas

In each case, a linear algebraic problem

$$Ax = b$$

Nonlinear Finite Element Analysis

Nonlinear statics

 $(S) \Leftrightarrow (W) \approx (G) \Leftrightarrow \text{Nonlinear problem}$

$$\boldsymbol{F}^{int} = \boldsymbol{F}^{ext}_{known}$$

e.g. elastostatics

 $F^{int} = N(\underline{d}) = \text{Nonlin.}$ algebraic ftn. of d

 $N:\mathbb{R}^{n_{eq}} o \mathbb{R}^{n_{eq}}$ (vect. valued ftn. of vect. d)

$$\begin{split} \pmb{N} &= \{N_P\}, & \qquad 1 \leq P \leq n_{eq} \\ & \qquad \uparrow \\ & \qquad \text{eq. no.} \end{split}$$

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Nonlinear dynamics

$$(S) \Leftrightarrow (W) \approx (G) \Leftrightarrow \underset{\text{o.d.e.'s}}{\text{Nonlin.}} \approx \underset{\text{stepping algorithm}}{\text{time-stepping algorithm}}$$

$$M\ddot{d} + \underbrace{F^{int}}_{N(d)} = F^{ext}$$

$$N^*(\Delta a) = R$$

We need to solve:

nonlinear algebraic problems

Reduce to the solution of **linear** algebraic problems by way of incremental/iterative strategies

Residual Methods

Assume the external load is parameterized by "time" (or equivalently, a load parameter).

Try to achieve:

$$0 = \underbrace{\boldsymbol{F}^{ext}}_{\boldsymbol{F}^{ext}(t)} - \underbrace{\boldsymbol{F}^{int}}_{\text{e.g.}} N(\boldsymbol{d}(t))$$
 (residual)

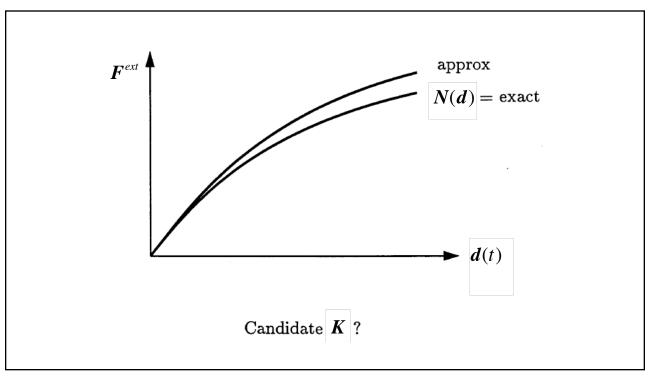
Example: Incremental load method

$$\overrightarrow{K} \Delta d = R^{n+1} \stackrel{\text{def}}{=} \underbrace{F_{n+1}^{ext}}_{F^{ext}(t_{n+1})} - \underbrace{F_{n}^{int}}_{N(d_n)}$$

$$d_{n+1} = d_n + \Delta d$$

$$n \leftarrow n + 1$$

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Consistent Tangents

Example 1. Nonlinear elastostatics

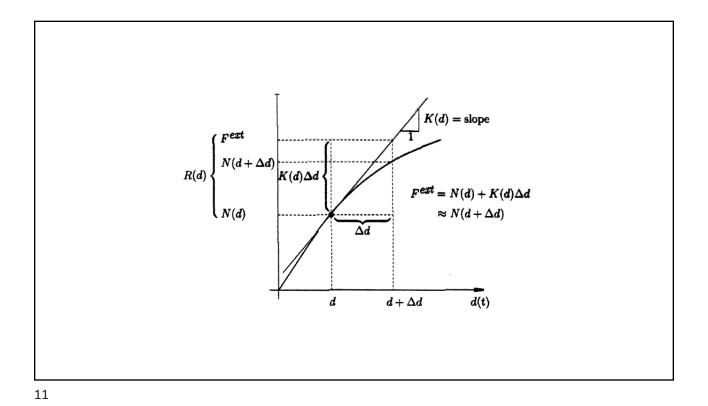
$$F^{int} = N(d)$$
 $K(d) \stackrel{\text{def}}{=} \frac{\partial N(d)}{\partial d}$ (tangent stiffness)
 $N = \{N_P\}$
 $K = \frac{\partial N}{\partial d} = \begin{bmatrix} \frac{\partial N_P}{\partial d_Q} \end{bmatrix}, \quad 1 \leq P, Q \leq n_{eq}$

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Example 1. Nonlinear elastostatics (cont.)

(hyper-) elasticity $\Leftrightarrow \exists \overset{\text{strain}}{U(\boldsymbol{d})}$ such that

$$egin{aligned} oldsymbol{N} &=& rac{\partial U}{\partial oldsymbol{d}}, & \text{i.e.} & N_P &=& rac{\partial U}{\partial d_P} \ &\Rightarrow & K_{PQ} &=& rac{\partial N_P}{\partial d_Q} &=& rac{\partial^2 U}{\partial d_Q \partial d_P} &=& K_{QP} \ oldsymbol{K} &=& oldsymbol{K}^{\mathsf{T}} & ext{symmetric} \end{aligned}$$



What's good about consistent tangents?

According to Taylor's theorem, the consistent tangent is the **best local approximation** in the sense that

$$\frac{\left\| \boldsymbol{K}(\boldsymbol{d}) \Delta \boldsymbol{d} - \left(\boldsymbol{N}(\boldsymbol{d} + \Delta \boldsymbol{d}) - \boldsymbol{N}(\boldsymbol{d}) \right) \right\|}{\left\| \Delta \boldsymbol{d} \right\|} \to 0 \quad \text{as} \quad \left\| \Delta \boldsymbol{d} \right\| \to 0$$

K(d) is the unique matrix achieving this property.

Interpretation:

 $N(d)+K(d)\Delta d$ is a **good local** approximation of $N(d+\Delta d)$.

Example 2. Nonlinear heat conduction

$$F^{int} = \underbrace{\hat{K}(d)}_{\substack{\text{symm.} \\ \text{matrix}}} \underbrace{d}_{\substack{\text{temperature} \\ \text{vector}}}$$

consistent tangent:
$$\frac{\partial \boldsymbol{F}^{int}}{\partial \boldsymbol{d}} = \left[\frac{\partial F_P^{int}}{\partial d_Q} \right]$$

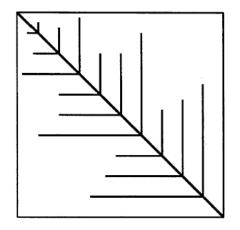
$$\begin{split} \frac{\partial (\hat{K}_{PR}^{\stackrel{\text{sum}}{\downarrow}} d_R^{\downarrow})}{\partial d_Q} &= \underbrace{\hat{K}_{PQ}}_{\text{symm.}} + \frac{\partial \hat{K}_{PR}}{\partial d_Q} d_R \\ \frac{\partial \hat{K}_{PR}}{\partial d_Q} &= \frac{\partial \hat{K}_{QR}}{\partial d_P} \quad \text{NO!} \quad \begin{array}{c} \text{Consistent} \\ \text{tangents are not} \\ \text{always symmetric} \\ \end{split}$$

$$\frac{\partial \hat{K}_{PR}}{\partial d_O} = \frac{\partial \hat{K}_{QR}}{\partial d_P} \quad \text{NO!}$$

always symmetric.

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Costs increase with the loss of symmetric matrices



Storage
$$\times 2$$

Factor $\times 2$

Newton - Raphson Method



I goseph Raphson of London Jehl do grant and agree to and with the Prefident, Council, and Fellows of the Royal Saciety of London for improving Natural knowledge, That so long as I shall continue Fellow of the said Society, I will pay to the Treasures.

Sealed and Delivered in the Presence of Dm. Hally

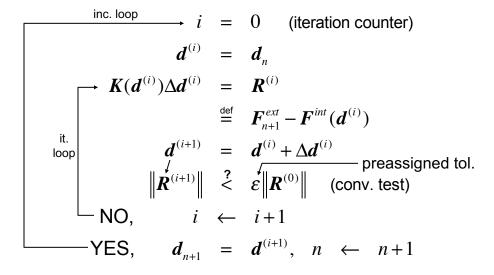
Isaac Newton

Joseph Raphson

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A method based on the consistent tangent

Newton-Raphson



where (strictly speaking)

$$K = \frac{\partial F^{int}}{\partial d} = \text{ consistent tangent}$$

In practice, frequently replace consistent tangent, e.g., modified Newton, etc.

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The order of convergence of an iterative scheme

Consider 1 d.o.f.

Let
$$e^{(i)} = d^{(i)} - \underbrace{d}_{\text{exact}}$$
 Write
$$\left| e^{(i+1)} \right| \leq c \left| e^{(i)} \right|^k$$

Write
$$\left|e^{(i+1)}\right| \leq c \left|e^{(i)}\right|^k$$

k = order of convergence

Newton-Raphson is such that k = 2.

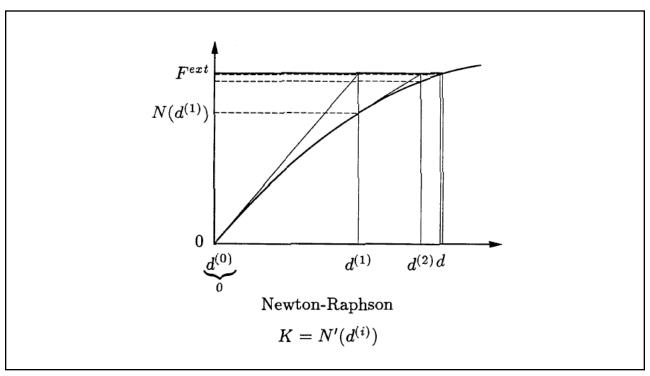
Interpretation:

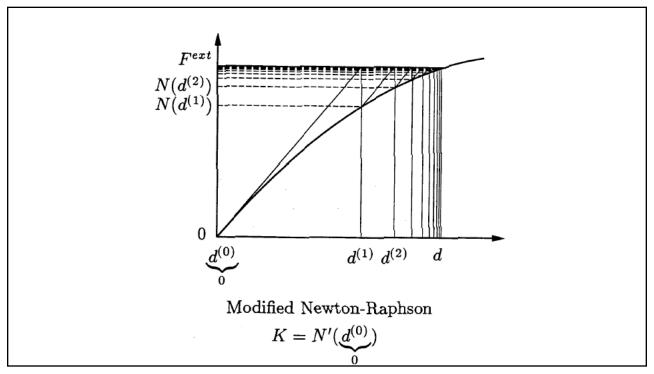
Assume
$$\left| e^{(0)} \right| = 0.9, \ c = 1, \ \text{then}$$

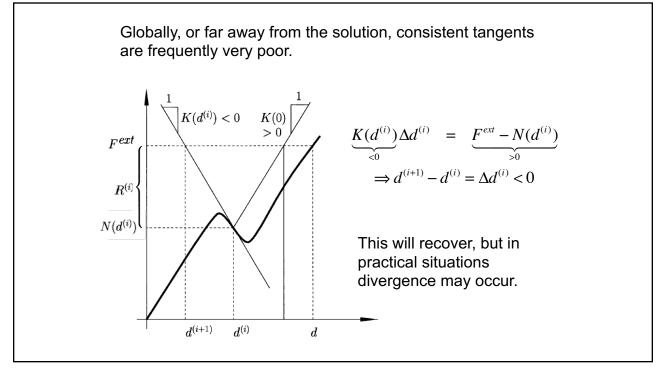
i	$\left e^{(i)} ight $ is \leq
1	.81
2	.66
2 3 4	.44
4	.19
5	. <u>0</u> 36
6	. <u>00</u> 13
7	. <u>0000</u> 016

quadratic convergence ⇒ the no. of 0's doubles (at least) in each iteration

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Developing an understanding of some aspects of nonlinear equation solving:

1. Soft/stiff tangent behavior

1 d.o.f. linear model problem: Kd = F, K > 0

Newton-Raphson-type algorithm

$$\tilde{K} \Delta d^{(i)} = F - Kd^{(i)} \quad (= R^{(i)})$$

$$\alpha < 1, \quad \text{soft}$$

$$\alpha > 1, \quad \text{stiff}$$

$$\alpha K(d^{(i+1)} - d^{(i)}) = F - Kd^{(i)}$$

$$-\alpha K(\underline{d} - \underline{d}) = -(\underline{F} - \underline{K}\underline{d})$$

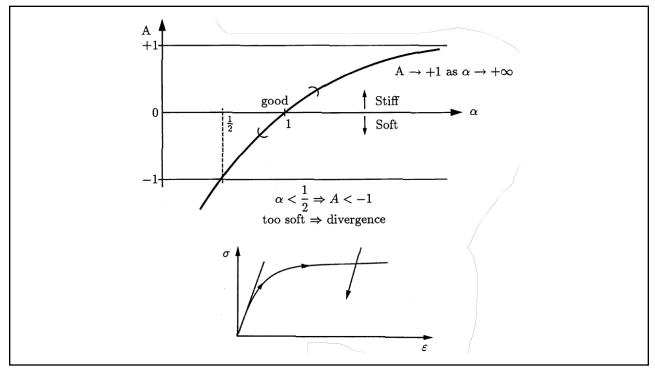
$$\alpha K(e^{(i+1)} - e^{(i)}) = -Ke^{(i)}$$

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where
$$e^{(i)} = d^{(i)} - d$$

$$e^{(i+1)} = Ae^{(i)}$$

$$A = \frac{\alpha - 1}{\alpha} = 1 - \frac{1}{\alpha}$$
convergence $\Leftrightarrow |A| < 1$
divergence $\Leftrightarrow |A| > 1$



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2. Linear systems reduce to 1 d.o.f. model problem

Consider

$$ilde{K}\Delta d^{(i)} = F - K d^{(i)}$$
 (*)
$$d^{(i+1)} = d^{(i)} + \Delta d^{(i)}$$
Assume $K = K^{\mathsf{T}} > 0$

$$ilde{K} = \tilde{K}^{\mathsf{T}} > 0$$

The associated eigenproblem:

$$(\mathbf{K} - \lambda \tilde{\mathbf{K}})\psi = 0$$

Solve for $\{\lambda,\psi\}_1^{n_{eq}}$; leads to uncoupled scalar versions

of (*):
$$\Delta d_{l}^{(i)} = F_{l} - \lambda_{l} d_{l}^{(i)}, \qquad l = 1,...,n_{eq}$$

no sum

Drop "l" subscript, proceed as before:

$$d^{(i+1)} - d^{(i)} = F - \lambda d^{(i)}$$

$$-(d - d) = -(F - \lambda d) \equiv 0$$

$$defines d$$

$$e^{(i+1)} = (1 - \lambda)e^{(i)}$$

Remarks

- 1. $\lambda = \frac{1}{\alpha}$ (cf. previous analysis) \Rightarrow each "mode" reduces to previous analysis.
- 2. A good \tilde{K} is such that λ 's are as close to unity as possible.
- 3. Soft modal components of $\tilde{\textbf{\textit{K}}}$ will cause the iteration to diverge.

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Line Search

I. Linear case

$$\begin{split} \tilde{\pmb{K}} & \underbrace{\Delta \pmb{d}^{(i)}}_{\substack{\text{search} \\ \text{dir. (vect.)}}} = \pmb{F} - \pmb{K} \pmb{d}^{(i)} = \pmb{R}^{(i)} \\ \pmb{d}^{(i+1)} &= \pmb{d}^{(i)} + \underbrace{\pmb{s}^{(i)}}_{\substack{\text{search} \\ \text{param. (scalar)}}} \Delta \pmb{d}^{(i)} \end{split}$$

What is the proper $s^{(i)}$?

Two derivations

1. Select $\boldsymbol{s}^{(i)}$ such that the potential energy is minimized

$$p(s^{(i)}) = P(d^{(i)} + s^{(i)} \Delta d^{(i)})$$
where
$$P(d) = \frac{1}{2} d^{\mathsf{T}} K d - d^{\mathsf{T}} F$$

$$\frac{dp}{ds} = 0 \Rightarrow \tilde{K} \Delta d^{(i)}$$

$$s^{(i)} = \frac{\Delta d^{(i)} \cdot \tilde{R}^{(i)}}{\Delta d^{(i)} \cdot K \Delta d^{(i)}}$$

> 0, if $\Delta d \neq 0$

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$$Kd = F$$

$$P(d) = \frac{1}{2}d^{T}Kd - d^{T}F$$

$$P'(d) = 0 \Rightarrow Kd = F$$

$$K^{-1}F$$

$$2K^{-1}F$$

1 d.o.f.:

$$s^{(i)} = \frac{\Delta d^{(i)^T} \hat{K} \Delta d^{(i)}}{\Delta d^{(i)^T} K \Delta d^{(i)}}$$
$$= \alpha \frac{K}{K} = \alpha$$

Two derivations (cont.)

2. Select $s^{(i)}$ such that $\mathbf{R}^{(i+1)} = \mathbf{F} - \mathbf{K}\mathbf{d}^{(i+1)}$ has zero component in the direction of $\Delta \mathbf{d}^{(i)}$, viz.

$$0 = \Delta \boldsymbol{d}^{(i)} \cdot \boldsymbol{R}^{(i+1)} \Longrightarrow s^{(i)}$$
 above

This criterion is very general and may be applied when no potential exists.

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II. Nonlinear cases

$$\tilde{K}\Delta d^{(i)} = F^{ext} - F^{int}(d^{(i)})$$
$$d^{(i+1)} = d^{(i)} + s^{(i)}\Delta d^{(i)}$$

Derivation 2 may be used:

(*)
$$\underbrace{0 = G(s^{(i)})}_{\text{Nonlin. scalar}} = \Delta \boldsymbol{d}^{(i)} \cdot \left(\boldsymbol{F}^{ext} - \boldsymbol{F}^{int} (\boldsymbol{d}^{(i)} + s^{(i)} \Delta \boldsymbol{d}^{(i)}) \right)$$
eq. for $s^{(i)}$

Strang-Matthies suggest accepting $s^{(i)}$ if

$$\left| G(s^{(i)}) \right| \le \frac{1}{2} \left| G(0) \right|$$

For details see H. Matthies and G. Strang, "The solution of Nonlinear F.E. Equations," *IJNME*, Vol. 14, 1613-1626 (1979).

Remarks

1. If \exists a potential $U(\boldsymbol{d})$ such that

$$\mathbf{F}^{int} = \mathbf{N} = \frac{\partial U}{\partial \mathbf{d}},$$

then derivation 1 also results in (*). This is the case for nonlinear elastostatics where

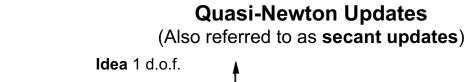
$$P(\boldsymbol{d}) = U(\boldsymbol{d}) - \boldsymbol{d}^{\mathsf{T}} \boldsymbol{F}$$

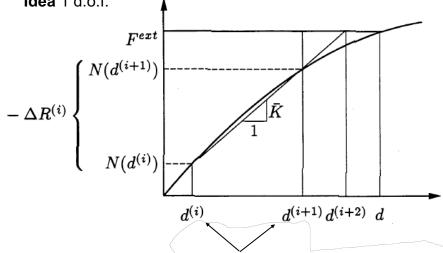
2. For situations in which the consistent tangent is not symmetric, positive definite, a minimization of the residual is frequently used to define *s* :

$$\min \ \mathbf{R}(\mathbf{d} + s\Delta \mathbf{d}) \cdot \mathbf{R}(\mathbf{d} + s\Delta \mathbf{d})$$

Caution: Structural bending elements, multiphysics, etc.

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Assume these have already been calculated.

$$\begin{split} \Delta R^{(i)} &= R^{(i+1)} - R^{(i)} \\ &= \left(X^{ext} - N(d^{(i+1)}) - \left(X^{ext} - N(d^{(i)}) \right) \right) \\ &= - \left(N(d^{(i+1)}) - N(d^{(i)}) \right) \end{split}$$

The secant \overline{K} would suffice as a good approximate l.h.s. matrix

$$\bar{K} = \frac{-\Delta R^{(i)}}{(d^{(i+1)} - d^{(i)})} \tag{*}$$

$$\overline{K}(d^{(i+2)} - d^{(i+1)}) = R^{(i+1)} \equiv F^{ext} - N(d^{(i+1)})$$
defines $d^{(i+2)}$

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For $n_{eq}=1, \ (*)$ uniquely defines \overline{K} . The generalization of (*) for $n_{eq}>1$ is

$$\overline{K}(\underline{d^{(i+1)}} - \underline{d^{(i)}}) = -\underline{\Delta R^{(i)}}_{\text{known}}$$

This is called the **quasi-Newton equation**. It does not uniquely define \overline{K} for $n_{eq}>1$, but places a restriction on its definition.

Quasi-Newton updates provide definitions for $\overline{\pmb{K}}$ such that the quasi-Newton equation is satisfied.

Quasi-Newton updates are often used with line searches.

The quasi-Newton equation becomes

$$\mathbf{d}^{(i+1)} = \mathbf{d}^{(i)} + s^{(i)} \wedge \mathbf{d}^{(i)}$$

$$\bar{\mathbf{K}}s^{(i)}\Delta\mathbf{d}^{(i)} = -\Delta\mathbf{R}^{(i)}$$

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Perhaps the most widely used in F. E. analysis is BFGS, which we shall describe.

BFGS (inverse) update

Broyden-Fletcher-Goldfarb-Shanno

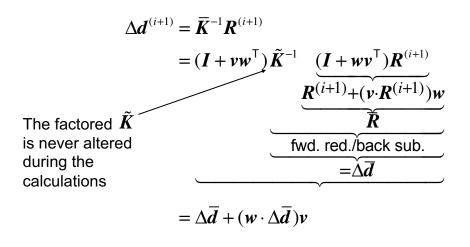
$$\overline{\boldsymbol{K}}^{-1} = (\boldsymbol{I} + \boldsymbol{v}\boldsymbol{w}^{\mathsf{T}})\tilde{\boldsymbol{K}}^{-1}(\boldsymbol{I} + \boldsymbol{w}\boldsymbol{v}^{\mathsf{T}})$$

v,w are vectors

Properties

- 1. $ilde{m{K}}^{-1}$ symmetric \Rightarrow $m{ar{K}}^{-1}$ symmetric
- 2. $\tilde{\pmb{K}}^{-1} > 0$ (and $\pmb{v} \cdot \pmb{w} \neq -1$), then $\bar{\pmb{K}}^{-1} > 0$
- 3. "Rank-2 update"

Calculations (from right to left)



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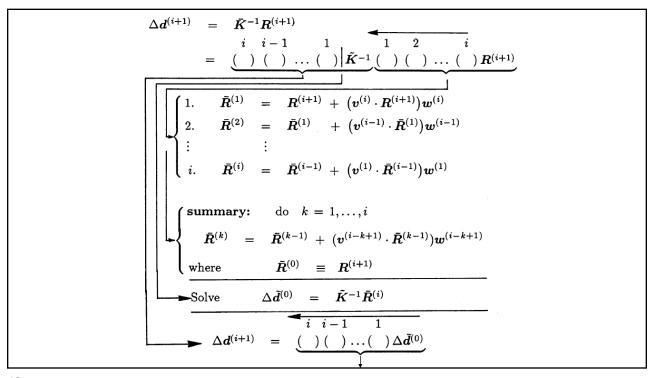
Series of updates (i updates)

 $ilde{m{K}}^{-1}$ represents the last formed and factored stiffness.

$$\bar{K}^{-1} = (I + v^{(i)}w^{(i)\mathsf{T}})(I + v^{(i-1)}w^{(i-1)\mathsf{T}})...$$

$$(I + v^{(1)}w^{(1)\mathsf{T}})\tilde{K}^{-1}(I + w^{(1)}v^{(1)\mathsf{T}}) \times$$

$$(I + w^{(2)}v^{(2)\mathsf{T}})...(I + w^{(i)}v^{(i)\mathsf{T}})$$



$$\begin{cases}
1. & \Delta \bar{d}^{(1)} = \Delta \bar{d}^{(0)} + (w^{(1)} \cdot \Delta \bar{d}^{(0)})v^{(1)} \\
2. & \Delta \bar{d}^{(2)} = \Delta \bar{d}^{(1)} + (w^{(2)} \cdot \Delta \bar{d}^{(1)})v^{(2)} \\
\vdots & \vdots \\
i. & \Delta \bar{d}^{(i)} = \Delta \bar{d}^{(i-1)} + (w^{(i)} \cdot \Delta \bar{d}^{(i-1)})v^{(i)}
\end{cases}$$

$$\begin{cases}
\text{summary: do } k = 1, \dots, i \\
\Delta \bar{d}^{(k)} = \Delta \bar{d}^{(k-1)} + (w^{(k)} \cdot \Delta \bar{d}^{(k-1)})v^{(k)} \\
\text{define } \Delta d^{(i+1)} = \Delta \bar{d}^{(i)}
\end{cases}$$

$$\begin{aligned} \textbf{Storage} &= \text{ no. of BFGS updates } \times 2 \times n_{eq} \\ &= \{ \pmb{v}^{(i)}, \pmb{w}^{(i)} \}_{i=1}^{\text{no. updates}} \end{aligned}$$

Definitions of BFGS vectors:

$$\begin{array}{rcl} \boldsymbol{v}^{(k)} & = & \frac{\Delta \boldsymbol{d}^{(k)}}{(\Delta \boldsymbol{d}^{(k)} \cdot \Delta \boldsymbol{R}^{(k)})} \\ \boldsymbol{w}^{(k)} & = & -\Delta \boldsymbol{R}^{(k)} + \alpha^{(k)} \boldsymbol{R}^{(k)} \\ \text{where} & \alpha^{(k)} & = & \sqrt{\frac{-s^{(k)} \Delta \boldsymbol{R}^{(k)} \cdot \Delta \boldsymbol{d}^{(k)}}{\boldsymbol{R}^{(k)} \cdot \Delta \boldsymbol{d}^{(k)}}} \end{array}$$

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Alternative definitions of BFGS vectors commonly used in practice.

Recall:
$$G^{(k)}(s^{(k)}) = \Delta d^{(k)} \cdot \left(R(d^{(k)} + s^{(k)} \Delta d^{(k)}) \right)$$

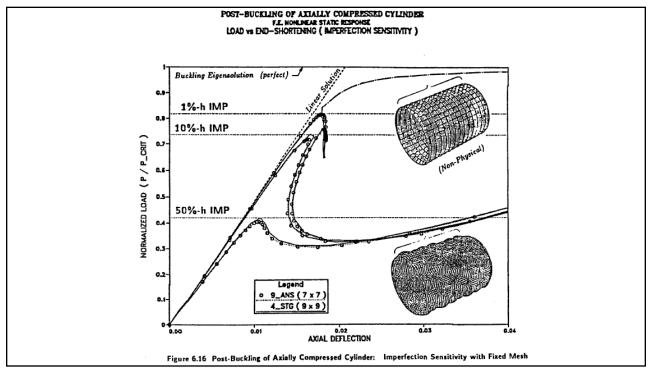
$$G^{(k)}(0) = \Delta d^{(k)} \cdot \left(R(d^{(k)}) \right)$$

$$G^{(k)}(s^{(k)}) - G^{(k)}(0) = \Delta d^{(k)} \cdot \Delta R^{(k)}$$

$$v^{(k)} = \frac{\Delta d^{(k)}}{(G^{(k)}(s^{(k)}) - G^{(k)}(0))}$$

$$w^{(k)} = -\Delta R^{(k)} + \alpha^{(k)} R^{(k)}$$
where $\alpha^{(k)} = \sqrt{\frac{-s^{(k)} \left(G^{(k)}(s^{(k)}) - G^{(k)}(0) \right)}{G^{(k)}(0)}}$

(See Strang and Matthies for further details.)



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Arc-length strategies (written in collaboration with R.M. Ferencz)

So far in this lecture external loading has been assumed to be a **given** function of time. Now we wish to trace load-displacement processes in which there is no **a priori** knowledge of the load-time function.

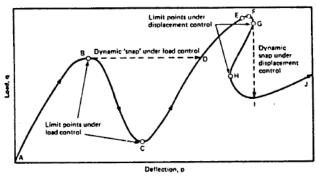


Fig. 1. "Snap buckling". (Crisfield 1981)

Better methods are clearly needed. In structures, the basic ideas are due to Riks, Wempner, Ramm, Bergan and Crisfield. Some useful references follow. In the mathematical literature, these techniques are referred to as **continuation methods**, in structures they are called **arc-length methods**.

References

- M.A. Crisfield, "A Fast Incremental/Iterative Solution Procedure that Handles 'Snap-through'," Computers and Structures, Vol. 13, 55-62, (1981).
- M.A. Crisfield, "An Arc-length Method Including Line Searches and Accelerations," *IJNME*, Vol. 19, 1269-1289, (1983).

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References (cont.)

- K.H. Schweizerhof and P. Wriggers, "Consistent Linearization for Path Following Methods in Nonlinear F.E. Analysis," CMAME, Vol. 59, 261-279, (1986)
- C.A. Felippa, "Solution of Nonlinear Static Equations," Handbook Series on Computer Methods in Mechanics, Volume on Large Deflection and Stability of Structures.

The essential idea in arc-length strategies is to control the arc-length of the solution curve in force-displacement space. This will accommodate automatic unloading as is needed to trace the solution curves in the above fig. A scalar arc-length function (i.e. constraint equation) is introduced along with an additional unknown, the loading parameter, to automatically control the amplitude of applied loading. The continuous version of the problem may be stated as follows:

Find d(t) and $\lambda(t)$ such that

(*) $\begin{cases} \boldsymbol{F}^{int}(\boldsymbol{d}(t)) = \lambda(t)\boldsymbol{F}^{ext} \\ f(\dot{\boldsymbol{d}},\dot{\lambda}) = \dot{a} = \text{ given} \end{cases}$ f = arc-length function $(\dot{}) = \frac{\partial()}{\partial t}$

where

$$(\dot{}) = \frac{\partial()}{\partial t}$$

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 $f\,$ should satisfy the definition of a norm on the $n_{eq}+1\,$ dimensional space consisting of pairs d, λ .

The discrete form of (*) is seen more frequently in the literature:

$$egin{align*} & oldsymbol{F}^{int}(\ oldsymbol{d}_{n+1}\) = \ \lambda_{n+1} \ oldsymbol{F}^{ext} \ & \text{unknown} \ & f(\delta oldsymbol{d}, \delta \lambda) \equiv f(oldsymbol{d}_{n+1} - oldsymbol{d}_n, \lambda_{n+1} - \lambda_n) \ & = \delta a = \ ext{given} \end{aligned}$$

As a candidate f consider:

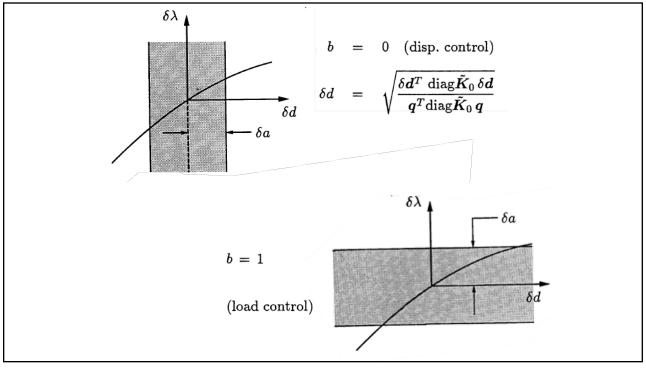
$$f(\delta d, \delta \lambda) = \sqrt{(c\delta d^{\mathsf{T}} \mathrm{diag} \tilde{K}_0 \delta d + b\delta \lambda^2)}$$

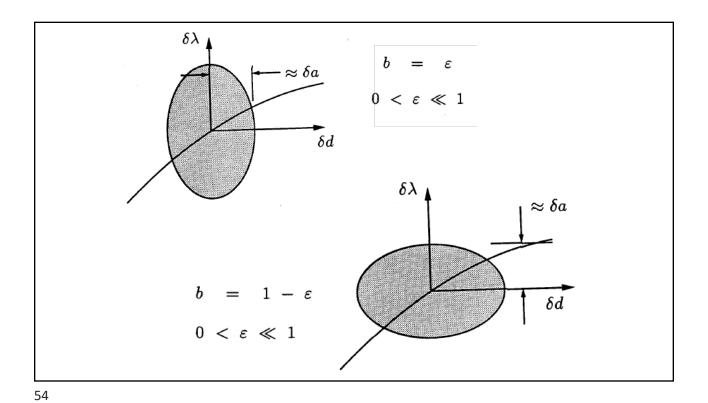
where

$$c = \frac{(1-b)}{(q^{\mathsf{T}} \mathrm{diag} \tilde{K}_0 q)}$$
$$q = \tilde{K}_0^{-1} F^{ext}$$
$$b \in [0,1]$$

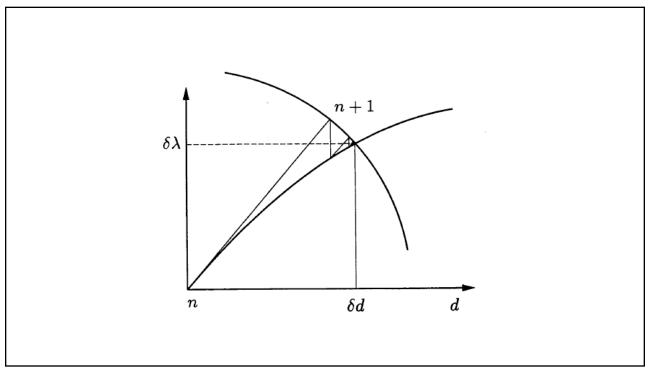
and $\tilde{\pmb{K}}_0$ is the first formed/factorized l.h.s. matrix.

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 $b = \frac{1}{2}$ Experience seems to favor displacement control (b = 0).



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Solution of (**)

Consistently linearize (**) w.r.t. $d_{\scriptscriptstyle n+1}$ and $\lambda_{\scriptscriptstyle n+1}$. That is

$$\begin{aligned} \boldsymbol{d}_{n+1} &\leftarrow \boldsymbol{d}_{n+1}^{(i)} + \Delta \boldsymbol{d}^{(i)} \\ \boldsymbol{\lambda}_{n+1} &\leftarrow \boldsymbol{\lambda}_{n+1}^{(i)} + \Delta \boldsymbol{\lambda}^{(i)} \end{aligned}$$

in (**) and linearize w.r.t. $\Delta \pmb{d}^{(i)}$ and $\Delta \lambda^{(i)}$. Using Taylor's formula:

$$\mathbf{F}^{int}(\mathbf{d}_{n+1}^{(i)}) + \mathbf{K}(\mathbf{d}_{n+1}^{(i)}) \Delta \mathbf{d}^{(i)} \cong \mathbf{F}^{int}(\mathbf{d}_{n+1}^{(i)} + \Delta \mathbf{d}^{(i)})
= (\lambda_{n+1}^{(i)} + \Delta \lambda^{(i)}) \mathbf{F}^{ext}$$

$$\begin{split} f_{n+1}^{(i)} + & \left(\frac{\partial f}{\partial \delta \boldsymbol{d}} \right)_{n+1}^{(i)} \cdot \Delta \boldsymbol{d}^{(i)} + \left(\frac{\partial f}{\partial \delta \lambda} \right)_{n+1}^{(i)} \cdot \Delta \lambda^{(i)} \\ & \cong f(\boldsymbol{d}_{n+1}^{(i)} + \Delta \boldsymbol{d}^{(i)} - \boldsymbol{d}_n, \lambda_{n+1}^{(i)} + \Delta \lambda^{(i)} - \lambda_n) \\ & = \delta a \end{split}$$

$$\text{where} \qquad f_{n+1}^{(i)} & = f(\delta \boldsymbol{d}^{(i)}, \delta \lambda^{(i)}) \\ & \left(\frac{\partial f}{\partial \delta \boldsymbol{d}} \right)_{n+1}^{(i)} & = \frac{\partial f}{\partial \delta \boldsymbol{d}} (\delta \boldsymbol{d}^{(i)}, \delta \lambda^{(i)}) \\ & \left(\frac{\partial f}{\partial \delta \lambda} \right)_{n+1}^{(i)} & = \frac{\partial f}{\partial \delta \lambda} (\delta \boldsymbol{d}^{(i)}, \delta \lambda^{(i)}) \\ & \delta \boldsymbol{d}^{(i)} & = \boldsymbol{d}_{n+1}^{(i)} - \boldsymbol{d}_n \\ & \delta \lambda^{(i)} & = \lambda_{n+1}^{(i)} - \lambda_n \end{split}$$

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$n_{eq} + 1$ equation matrix problem :

$$\begin{bmatrix} \mathbf{K} & -\mathbf{F}^{ext} \\ \frac{\partial f}{\partial \delta \mathbf{d}} & \frac{\partial f}{\partial \delta \lambda} \end{bmatrix} \begin{cases} \Delta \mathbf{d}^{(i)} \\ \Delta \lambda^{(i)} \end{cases} = \begin{cases} \mathbf{R}_{n+1}^{(i)} \mathbf{F}^{ext} - \mathbf{F}^{int} (\mathbf{d}_{n+1}^{(i)}) \\ \delta a - f_{n+1}^{(i)} \end{cases}$$

Remarks

- 1. K can be replaced by any convenient l.h.s. matrix, say \overline{K} , accounting, e.g., for quasi-Newton updates.
- 2. Line searches may also be accounted for by updating

$$\boldsymbol{d}_{n+1}^{(i+1)} = \boldsymbol{d}_{n+1}^{(i)} + s^{(i)} \Delta \boldsymbol{d}^{(i)}$$

where $S^{(i)}$ is defined by

$$0 = G(s^{(i)})$$

$$\equiv \Delta \boldsymbol{d}^{(i)} \cdot \left(\lambda_{n+1}^{(i)} \boldsymbol{F}^{ext} - \boldsymbol{F}^{int} (\boldsymbol{d}_{n+1}^{(i)} + s^{(i)} \Delta \boldsymbol{d}^{(i)}) \right)$$

- 3. It is not expensive to calculate $\frac{\partial f}{\partial \delta d}$ and $\frac{\partial f}{\partial \delta \lambda}$, so normally the latest updated values are used.
- 4. Explicit formulae:

$$\left(\frac{\partial f}{\partial \delta \boldsymbol{d}}\right)_{n+1}^{(i)} = \frac{c}{f_{n+1}^{(i)}} \operatorname{diag} \tilde{\boldsymbol{K}}_0 \delta \boldsymbol{d}^{(i)}$$
$$\left(\frac{\partial f}{\partial \delta \lambda}\right)_{n+1}^{(i)} = \frac{b}{f_{n+1}^{(i)}} \delta \lambda^{(i)}$$

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Solution (Note: $\frac{\partial f}{\partial \delta \lambda}$ may be zero!)

- 1. Solve first eq. for $\Delta d^{(i)}$ as a function of $\Delta \lambda^{(i)}$ (unknown).
- 2. Substitute this result into second eq. and solve for $\Delta \lambda^{(i)}$.

Results

$$\Delta \lambda^{(i)} = \frac{\left(\delta a - f_{n+1}^{(i)}\right) - \left(\frac{\partial f}{\partial \delta d}\right)_{n+1}^{(i)} \cdot \Delta \overline{d}}{\left(\left(\frac{\partial f}{\partial \delta d}\right)_{n+1}^{(i)} \cdot \overline{q} + \left(\frac{\partial f}{\partial \delta \lambda}\right)_{n+1}^{(i)}\right)}$$

$$\Delta d_{n+1}^{(i)} = \Delta \lambda^{(i)} \overline{q} + \Delta \overline{d}$$

$$\Delta \overline{d} = \overline{K}^{-1} R_{n+1}^{(i)}$$

$$\overline{q} = \overline{K}^{-1} F^{ext}$$

Initilization

To get the arc-length method started in each step requires **specification** of $\delta\lambda^{(0)}$ and $\delta d^{(0)}$.

Let

$$\tilde{\boldsymbol{q}} = \tilde{\boldsymbol{K}}^{-1}(\boldsymbol{d}_n)\boldsymbol{F}^{ext}$$

$$\delta \boldsymbol{d}^{(0)} = \delta \lambda^{(0)} \tilde{\boldsymbol{q}}$$

Thus all that is required is to define $\delta\lambda^{(0)}$. The arc-length constraint determines the **magnitude** of $\delta\lambda^{(0)}$.

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$$\delta a = f(\delta \mathbf{d}^{(0)}, \delta \lambda^{(0)})$$
$$= f(\delta \lambda^{(0)} \tilde{\mathbf{q}}, \delta \lambda^{(0)})$$
$$= |\delta \lambda^{(0)}| f(\tilde{\mathbf{q}}, 1)$$

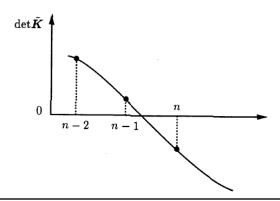
$$\Rightarrow \left| \delta \lambda^{(0)} \right| = \frac{\delta a}{f(\tilde{\mathbf{q}}, 1)}$$

To determine the **sign** of $\delta\lambda^{(0)} = \delta\lambda_{n+1}^{(0)}$, we proceed as follows:

If the following two conditions are true we set

$$\operatorname{sign} \delta \lambda^{(0)} = -\operatorname{sign} \delta \lambda_n^{(0)}:$$

- (i) $\det \tilde{\textbf{\textit{K}}}(\textbf{\textit{d}}_{\scriptscriptstyle n}) = -\det \tilde{\textbf{\textit{K}}}(\textbf{\textit{d}}_{\scriptscriptstyle n-1})$
- (ii) The determinant has passed through **zero**; for example:



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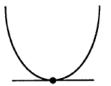
If either of the above is not true we set

$$\operatorname{sign} \delta \lambda^{(0)} = \operatorname{sign} \delta \lambda_n^{(0)}$$

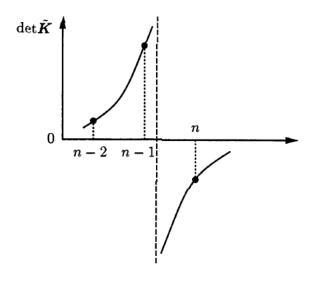
Remarks

To understand the rationale behind the strategy, consider the scalar case. The condition that the determinant passed through zero corresponds to a **maximum** or a **minimum** in the load-displacement diagram:



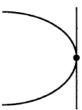


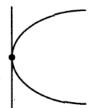
Note that the determinant can change signs by passing through **infinity**; for example:



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This corresponds to **turning points** in the load-deflection curve:





Remarks

- 1. An adaptive strategy for increasing/decreasing the size of δa is essential for good performance of the algorithm.
- 2. Arc-length strategies have not yet progressed to the point where they are 100% reliable.

Convergence

def. $\|\mathbf{R}\|^2 = \mathbf{R}^{\mathsf{T}} (\operatorname{diag} \tilde{\mathbf{K}}_0)^{-1} \mathbf{R}$

(i) Insist $\left\| oldsymbol{R}_{n+1}^{(i)}
ight\| \leq arepsilon_R \left\| oldsymbol{R}^{(0)}
ight\|$

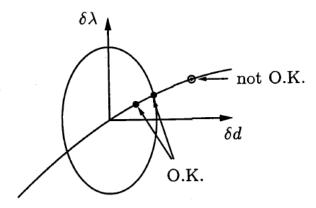
where \mathcal{E}_{R} is a user-specified tolerance.

(ii) In addition, we must ensure arc-length constraint is maintained:

$$f_{n+1}^{(i)} - \delta a \le \varepsilon_f \delta a$$

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Interpretation: $d_{n+1}^{(i)}, \lambda_{n+1}^{(i)}$ is inside the ellipse.



Some codes also employ a displacement criterion:

Let
$$\|\boldsymbol{d}\|^2 = \boldsymbol{d}^\mathsf{T} \mathrm{diag} \tilde{\boldsymbol{K}}_0 \boldsymbol{d}$$

Insist,

$$\|\Delta \boldsymbol{d}^{(i)}\| \le \varepsilon_d \|\delta \boldsymbol{d}^{(i)}\| = \varepsilon_d \|\boldsymbol{d}_{n+1}^{(i)} - \boldsymbol{d}_n\|$$

As long as $b \neq 1$ (i.e., not load control), the displacement criterion is redundant with the arc-length constraint. In other situation (e.g., Newton-like methods), the displacement criterion should generally be used.

The next slide shows an application of the arc-length method to a shell buckling problem. From G. Stanley, Ph.D. Thesis, Stanford, 1985.

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