

단국대학교 카피킬러캠퍼스 표절 검사 결과 확인서

확 인

성 명

서 명

아이디	72070621	표절률	15%
소속	건축공학과건축구조		
성명			

검사번호	00023710364	검사일자	2017.06.19 18:08
발급형태	<input type="checkbox"/> 기본보기 <input checked="" type="checkbox"/> 요약보기 <input type="checkbox"/> 상세보기	발급일자	2017.06.19 18:40
검사명	표절검사		
문서명	dissertation.pdf		
비고			

비교범위	[현재 첨부 문서] [카피킬러 DB]
검사설정	표절기준 [10 어절][5 문장], 인용/출처 표시문장 [제외], 법령/성경 포함문장 [제외], 목차/참고문헌 [제외]

검토 의견	
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표절률 분석 정보

표절률	전체문장	동일문장	의심문장	인용/출처	법령/성경
15%	1,050	41	197	77	0

비교 문서 정보

번호	표절률	출처정보	비고
1	12%	[카피킬러 DB] Copykiller - 문서명 : 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures - 저자 : 박은천 - 발행 : 서울 : 단국대학교 대학원, 2007.2	
2	10%	[카피킬러 DB] Copykiller - 문서명 : 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 - 저자 : 박은천 - 발행 : 서울 : 檀國大學校, 2006	
3	1%	[카피킬러 DB] Copykiller - 문서명 : Real-time Hybrid Test on a Semi-actively Controlled Building Structure Equipped with Full-scale MR Dampers - 저자 : Park, E[Park, Eunchurn];Min, KW[Min, Kyung-Won];Lee, SK[Lee, Sung-Kyung];Lee, SH[Lee, Sang-Hyun];Lee, HJ[Lee, Heon-Jae];Moon, SJ[Moon, Seok-Jun];Jung, HJ[Jung, Hyung-Jo] - 발행 : Dec-2010	
4	1%	[카피킬러 DB] Copykiller - 문서명 : Design of a Bi-directional Tuned Liquid Mass Damper for Controlling Dynamic Responses of Building Structures : 건축구조물의 동적응답제어를 위한 2방향 동조액체질량감쇠기 설계 - 저자 : 허재성 - 발행 : 서울 : 단국대학교 대학원, 2008	
5	1%	[카피킬러 DB] jiaa.org - 문서명 : Unified Semiactive Control System Based on MR Damper for Cable ...	

검사 문서

문장표절률: 70%

The actuator force is calculated by using the inverse transfer function of a target structural xv ABSTRACT responses to the actuator.

문장표절률: 56%

Filter and envelope function are used such that the error between wind and actuator induced responses is minimized by preventing the actuator from the exciting, unexpected modal responses and initial transient responses.

문장표절률: 92%

The analyses result from a 76-story benchmark building problem in which wind load obtained by wind tunnel test is given, indicate that the actuator installed at a particular floor can approximately embody the structural responses induced by the wind load applied to each floor of the structure.

문장표절률: 92%

The actuator designed by the proposed method can be effectively used for evaluating the wind response characteristics of a building structure in use and for obtaining an accurate analytical model of the building under wind load.

문장표절률: 94%

Secondly, a full scale forced vibration test simulating earthquake response is implemented by using a hybrid mass damper.

비교 문서

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자: 박은천

발행: 서울: 단국대학교 대학원, 2007.2

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Filter and envelope function are used such that the error between wind and actuator induced responses is minimized by preventing the shaker from exciting unexpected modal responses and initial transient responses - The effectiveness of the proposed method is verified through a numerical example of a 76 story-benchmark building excited by wind load of which deterministic time history is given.

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The analyses results from a 76-story benchmark building problem in which wind load obtained by wind tunnel test is given, indicate that the actuator installed at a specific floor can approximately embody the structural responses induced by the wind load applied to each floor of the structure. The actuator designed by the proposed method can be effectively used for evaluating the wind response characteristics of a practical building structure and for obtaining an accurate analytical model of the building under wind load.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

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저자: 박은천

발행: 서울:檀國大學校, 2006

Finally, in the chapter 6, a full scale forced vibration test simulating earthquake response is implemented by using a hybrid mass damper.

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저자: 박은천

발행: 서울: 단국대학교 대학원, 2007.2

문장표절률: 56%

The finite element model of the real-scaled building structure was analytically constructed, and the model was updated using the results experimentally measured by the forced vibration test.

Finally, in the chapter 6, a full scale forced vibration test simulating earthquake response is implemented by using a hybrid mass damper.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울: 檀國大學校, 2006

The finite element(FE) model of the structure was analytically constructed using ANSYS and the model was updated using the results experimentally measured by the forced vibration test.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

The finite element(FE) model of the structure was analytically constructed using ANSYS and the model was updated using the results experimentally measured by the forced vibration test.

문장표절률: 45%

Pseudo-earthquake excitation tests showed that the hybrid mass damper induced floor responses coincided with the earthquake induced ones which were numerically calculated based on the updated FE model.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울: 檀國大學校, 2006

Pseudo-earthquake excitation tests showed that HMD hybrid mass damper induced floor responses coincided with the earthquake induced ones which was numerically calculated based on the updated FE model.

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저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

Pseudo-earthquake excitation tests showed that HMD hybrid mass damper induced floor responses coincided with the earthquake induced ones which was numerically calculated based on the updated FE model.

문장표절률: 60%

First, the force required to drive the displacement of the story, at which the MR damper is located, xvi ABSTRACT is measured from the load cell attached to the UTM.

[Copykiller] Real-time Hybrid Test on a Semi-actively Controlled Building Structure Equipped with Full-scale MR Dampers

저자 : Park, E[Park, Euncheon]; Min, KW[Min, Kyung-Won]; Lee, SK[Lee, Sung-Kyung]; Lee, SH[Lee, Sang-Hyun]; Lee, HJ[Lee, Heon-Jae]; Moon, SJ[Moon, Seok-Jun]; Jung, HJ[Jung, Hyung-Jo]

발행 : Dec-2010

First, the force required to drive the displacement of the story, at which the MR damper is located, is measured from the load cell attached to the UTM.

문장표절률: 100%

The measured force is then returned to a control computer to calculate the response of the numerical substructure. Finally, the experimental substructure is excited by the UTM with the calculated response of the numerical substructure.

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저자 : Park, E[Park, Euncheon]; Min, KW[Min, Kyung-Won]; Lee, SK[Lee, Sung-Kyung]; Lee, SH[Lee, Sang-Hyun]; Lee, HJ[Lee, Heon-Jae]; Moon, SJ[Moon, Seok-Jun]; Jung, HJ[Jung, Hyung-Jo]

발행 : Dec-2010

The measured force is then returned to a control computer to calculate the response of the numerical substructure. Finally, the experimental substructure is excited by the UTM with the calculated response of the numerical substructure.

문장표절률: 77%

The hybrid testing method implemented in this study is validated because the hybrid testing results obtained by application of sinusoidal and earthquake excitations and the corresponding analytical results obtained using the Bouc-Wen model as the control force of the MR damper with respect to input currents are in good agreement.

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발행 : Dec-2010

The RT-HYTEM implemented in this study is validated because the real-time hybrid testing results obtained by application of sinusoidal and earthquake excitations and the corresponding analytical results obtained using the Bouc-Wen model as the control force of the MR damper with respect to input currents are in good agreement.

문장표절률: 81%

Also, the results from the hybrid testing method for the passive -on and -off control show that the structural responses did not decrease further by the excessive control force, but decreased due to the increase of the current applied to the MR damper.

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저자 : Park, E[Park, Euncheon]; Min, KW[Min, Kyung-Won]; Lee, SK[Lee, Sung-Kyung]; Lee, SH[Lee, Sang-Hyun]; Lee, HJ[Lee, Heon-Jae]; Moon, SJ[Moon, Seok-Jun]; Jung, HJ[Jung, Hyung-Jo]

발행 : Dec-2010

Also, the results from RT-HYTEM for the passive -on and -off control show that the structural responses did not decrease further by the excessive control force, but decreased due to the increase of the current applied to the MR damper.

문장표절률: 78%

[Copykiller] Real-time Hybrid Test on a Semi-actively Controlled Building Structure Equipped with Full-scale MR Dampers

Also, two semi-active control algorithms (modulated homogeneous friction and the clipped-optimal control algorithms) are applied to the MR damper to optimally control the structural responses.

문장표절률: 68%

To compare the hybrid testing method and numerical results, Bouc-Wen model parameters are identified for each input current.

문장표절률: 60%

The results of the comparison of experimental and numerical responses show that it is more practical to use the hybrid testing method in semi-active devices such as MR dampers.

문장표절률: 62%

The test results indicate that a control algorithm can be experimentally applied to the MR damper using the hybrid testing method.

문장표절률: 95%

The structural responses of the in-teractionsystem are calculated numerically in real-time using the analytical structural model with the excitations of measured control force, user-defined base earthquake loads, and its statespace realization incorporated in the in-tegrated controller of an actuator.

문장표절률: 33%

Also, in order to minimize the distortion of the acceleration of the shaking table or an actuator's displacement, the inverse transfer function of the shaking table or the actuator is identified and its statespace realization is implemented in the hybrid testing method controller.

문장표절률: 64%

Secondly, in chapter 3, simulation of dynamic responses of a buildingstructure u

structure Equipped with Full-scale MR Dampers

저자 : Park, E[Park, Euncheon];Min, KW[Min, Kyung-Won];Lee, SK[Lee, Sung-Kyung];Lee, SH[Lee, Sang-Hyun];Lee, HJ[Lee, Heon-Jae];Moon, SJ[Moon, Seok-Jun];Jung, HJ[Jung, Hyung-Jo]
발행 : Dec-2010

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발행 : Dec-2010

To compare the RT-HYTEM and numerical results, Bouc-Wen model parameters are identified for each input current. The results of the comparison of experimental and numerical responses show that it is more practical to use RT-HYTEM in semi-active devices such as MR dampers.

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발행 : Dec-2010

The test results show that a control algorithm can be experimentally applied to the MR damper using RT-HYTEM. This article provides a discussion on each algorithm with respect to the seismic performances.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천
발행 : 서울: 檀國大學校, 2006

The structural responses of the in-teractionsystem are calculated numerically in real time using the analytical structural model with the excitations of measured control force, user-defined base earthquake loads, and its statespace realization incorporated in the in-tegrated controller of the shaking table.

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ing a linear mass shaker is conducted. In order for the linear shaker to keep the structure in the target response trajectory, an inverse transfer function of a structural response to the shaker force is obtained using a statespace form governing equation of the structure and the discrete Fourier transform of structural response is performed.

문장표절률: 61%

Filter and envelope function are used such that the error between wind and actuator induced responses is minimized by preventing the shaker from exciting unexpected modal response and initial transient response.

문장표절률: 100%

The effectiveness of the proposed method is verified through a numerical example of a 76 story- benchmark building excited by wind load of which deterministic time history is given.

문장표절률: 67%

This study presents the quantitative evaluation of the seismic performance of a building structure installed with a MR damper using hybrid testing method.

문장표절률: 83%

A building model is identified from the forced vibration testing results of a full-scale five-story building and is used as the numerical substructure, and an MR damper corresponding to an experimental substructure is physically tested using a universal testing machine (UTM).

문장표절률: 100%

First, the force required to drive the displacement of the story, at which the MR damper is located, is measured from the load cell attached to the UTM.

저자 : 박은천
발행 : 서울 :檀國大學校, 2006

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발행 : 서울 :檀國大學校, 2006

The effectiveness of the proposed method is verified through a numerical example of a 76 story- benchmark building excited by wind load of which deterministic time history is given.

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저자 : 박은천
발행 : 서울 : 단국대학교 대학원, 2007.2

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[Copykiller] Real-time Hybrid Test on a Semi-actively Controlled Building Structure Equipped with Full-scale MR Dampers

저자 : Park, E[Park, Eunchun]; Min, KW[Min, Kyung-Won]; Lee, SK[Lee, Sung-Kyung]; Lee, SH[Lee, Sang-Hyun]; Lee, HJ[Lee, Heon-Jae]; Moon, SJ[Moon, Seok-Jun]; Jung, HJ[Jung, Hyung-Jo]

발행 : Dec-2010
This article presents the quantitative evaluation of the seismic performance of a building structure installed with a magnetorheological MR damper using RT-H YTEM.

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발행 : Dec-2010
A building model is identified from the forced vibration testing results of a full-scale five-story building and is used as the numerical substructure, and an MR damper corresponding to an experimental substructure is physically tested using a universal testing machine (UTM).

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발행 : Dec-2010
First, the force required to drive the displacement of the story, at which the MR damper is located, is measured from the load cell attached to the UTM.

문장표절률: 100%

The measured force is then returned to a control computer to calculate the response of the numerical substructure. Finally, the experimental substructure is excited by the UTM with the calculated response of the numerical substructure.

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The hybrid testing method implemented in this study is validated because the hybrid testing results obtained by application of sinusoidal and earthquake excitations and the corresponding analytical results obtained using the Bouc-Wen model as the control force of the MR damper with input currents are in good agreement.

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문장표절률: 83%

Also, the results from hybrid testing method for the passive -on and -off control show that the structural responses did not decrease further by the excessive control force, but decreased due to the increase of the current applied to the MR damper.

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문장표절률: 100%

Also, two semi-active control algorithms (modulated homogeneous friction and the clipped-optimal control algorithms) are applied to the MR damper in order to optimally control the structural responses.

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However, their study is based on numerical verification, and also has several limitations for being applied to real-scaled structures.

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First, forced vibration tests are not appropriate for utilizing the vibration source with large amplitude in the experimental structure.

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Especially, the structure cannot be excited to its nonlinear arrangement, since safety may not be ensured during the test and the linear structural models used for the numerical simulation.

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문장표절률: 73%

Secondly, it is inappropriate for using the vibration source with full bandwidth. Thirdly, sensors may not be sufficiently deployed in field test due to expenses and their installations so that the structural members, in which critical behaviors are expected, are detected.

문장표절률: 91%

Finally, the process of obtaining the system matrix of a structure by using the system identification techniques should be additionally included in the entire research work, since the structure is based on an analytical model.

문장표절률: 100%

Accordingly, the validity of the proposed pseudo-earthquake excitation test needs to be experimentally investigated under these constraints.

문장표절률: 91%

Dyke et al. (1994) obtained controller canonical form statespace realization for a small scale three-story building by using both active mass driver and shaking table and measuring the absolute floor acceleration.

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Secondly, it is inappropriate for using the vibration source with wide bandwidth. Thirdly, sensors may not be sufficiently deployed in field test due to expenses and their installations so that the structural members, in which critical behaviors are expected, are detected.

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· System Identification and Force Vibration Test Dyke et al. (1994) obtained controller canonical form statespace realization for a small scale three story building by using both active mass driver and shaking table and measuring the absolute floor acceleration.

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문장표절률: 75%

The necessity of testing the upper substructure is found in diverse structures. For example, the wind-induced acceleration response of slender building structures is usually tremendous in the top story, and detailed experiment for this upper part of the building structure may be required, since excessive acceleration response may be harmful to human comfort or high precision equipment.

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For example, the wind-induced acceleration response of slender building structures is usually very large in the top story and detailed experiment for this upper part of the building structure may be required, since excessive acceleration response may be harmful to human comfort or high precision equipments.

문장표절률: 100%

Also, in some cases, a light appendage, for example, a penthouse, a small housing for mechanical equipment and an advertising billboard, may vibrate excessively rather than the primary structure.

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Also, in some cases, a light appendage, for example, a penthouse, a small housing for mechanical equipment and an advertising billboard, may vibrate excessively rather than the primary structure.

문장표절률: 44%

Besides, to suppress excessive vibration induced by winds and earthquakes, a tuned mass damper or tuned liquid damper is employed on the top story of the high-rise building, and 11 Chapter.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

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발행 : 서울 : 단국대학교 대학원, 2007.2

Besides, to suppress excessive vibration induced by winds and earthquakes, a tuned mass damper (TMD) or tuned liquid damper (TLD) is employed on the top story of the high-rise building and its performance should be verified in the laboratory before installation.

문장표절률: 62%

2. Hybrid Testing Method its performance should be verified in the laboratory before installation. Finally, the base isolation system to protect expensive machinery housed in a building structure needs experimental verification for a certain level of earthquake.

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Finally, the base isolation system to protect expensive machinery housed in a building structure needs experimental verification for a certain level of earthquake.

문장표절률: 36%

\$POUSPM 1\$ TVOCVTNUSFVSDJDUVBMSF NJOFUBFTSVGBSDFFN GPF OSDU FPG PDGP BOOUS BPD M UTJVHBOUPBSM FTVYQCFTSUJSNVDF UOVUSBFM XIPMF TUSVDUVSF FYQFSJNFOUBM TZTUFN (a) conventional method FTVYQCFTSUJSNVDFUOVUSBFM TVOCVTNUSFVSDJDUBVMS F GFTFJEHCOBBD M L DTPJHOOUSBPMM TIBLJOH UBCMF \$POUSPM 1\$ XIPMF TUSVDUVSF FYQFSJNFOUBM TZTUFN (b) proposed method Figure 2.1: Conceptual illustrations of the substructuring methods On these backgrounds, this subsection proposes a shaking table testing method using the upper part of a building structure as the experimental substructure based on the acceleration feedback from the experimental substructure.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

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Conceptual illustrations of the real-time substructuring methods On these backgrounds, this paper proposes a shaking table testing method using the upper part of a building structure as the experimental substructure based on the acceleration feedback from the experimental substructure.

문장표절률: 76%

At first, the interface force acting between the upper experimental and lower numerical substructures is calculated based on the acceleration feedback of the experimental substructure which is mounted on shaking table.

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At first, the interface force acting between the upper experimental and lower numerical substructures is calculated based on the acceleration feedback of the experimental 9 2.

문장표절률: 68%

Then the interface acceleration 12 Chapter. 2. Hybrid Testing Method required to replicate the dynamic behavior of the whole structure is calculated from the numerical substructures subjected to the interface force and assumed base motion for the entire structure.

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REAL-TIME SUBSTRUCTURING TECHNIQUE substructure which is mounted on shaking table. Then the interface acceleration required to replicate the dynamic behavior of the whole structure is calculated from the numerical substructures subjected to the interface force and assumed base motion for the whole structure.

문장표절률: 100%

Finally, shaking table excites the upper experimental substructure according to the command signal from the shaking table controller.

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The series of above processes are performed during a single time interval and repeated over entire duration of the experiment.

Schematic diagram of the proposed method 2.2 Formulation and Implementation Methodology 2.2.1 Experimental and Numerical Substructures This section addresses physical interpretation and formulation of splitting the whole structure into the experimental and numerical substructures.

A two-step process is carried out to formulate the equations of motion for these two substructures: separating the experimental substructure from the whole structure [10, 2].

3. The equation of motion of the whole shear-type building structure subjected to the seismic ground acceleration, $Y_q(t)$, is represented in Fig.

3 (a) and expressed as $[M]\{Y(t)\}HC^T(2A)$ where, $Y(t)$ is the $n \times W$ vector of the absolute floor displacements given by $\{7PF2, \dots, 1\}$, and $\{p(t)\}$ is the $n \times W$ vector of the external force given by 0, 0, M and $|A|$ are the structural mass, damping and stiffness matrices, respectively, and expressed as follows.

mn C" - Cn - 지 Q+Q 니 -巧·令 # ···· kn - kn n n · 參 2.2)m m -I ··· ^m-
r1^ wr7j ^in - c c + c - c ^if ^>n-1 ··· 馨 C2+Cl- W이 우, r+1^m 니L - &
k + k - k ·········· - k0 kj + ky The experimental and numerical substructures
are **separated from the whole structure by cutting at their interface, as shown in**
Fig.

2.3 (b). Mathematically, this operation means separating the equations included in Eq. (2.1) into two groups of which one corresponds to the upper part DOF and the other corresponds to the lower part DOF.

The dotted line in Eq. (2.2) divides those two groups and the resulting two equations of motions are written as $M_e \ddot{y}_e + C_e \dot{y}_e + K_e y_e = F_e(t)$ (2.3) and $M_n \ddot{y}_n + C_n \dot{y}_n + K_n y_n = F_n(t)$ (2.4) where, subscripts e and n denote the experimental and numerical sub structures, respectively.

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스템 설계

acts as the ground acceleration of the experimental substructure given by Eq.

저자 : 박은천
발행 : 서울: 檀國大學校, 2006

Concept of the proposed method It is obvious from Eq. (2.7) that the absolute acceleration of the top story of the numerical substructure acts as the ground acceleration of the experimental substructure given by Eq.

문장표절률: 63%

(2.3). This condition is physically realized by synchronizing the shaking table motion with the absolute acceleration of the top story of the numerical substructure.

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(2.3). This condition is physically realized by synchronizing the shaking table motion with the absolute acceleration of the top story of the numerical substructure.

문장표절률: 14%

Besides, the first element of the external force vector in Eq. (2.8) means the interface force acting on the top story of the numerical substructure. 15 Chapter. 2. Hybrid Testing Method) Y m E(l) (t) E(l) Y m E(l) (t) E(l) E(l□1)(t) mE(l□1) YE(l□1)(t) mE(l□1) esuxbpsetriumcet unrtael) YE(1)(t) mE(1) Yn(t) mn cutting at YE(1)(t) mE(1) YN(m)(t) m(t) Yn□1(t) Y mn□1 m N(N(m) Y m N(m) (t) N(m) N(m□1)(t) Y m2(t) N(m□1) m2 YN(m□1)(t) numerical mN(m□1) YN(1)(t) YN(1)(t) Y1(t) m1 mN(1) Yg(t) Yg(t) mN(1) Yg(t) (a) Whole structure (b) Separation of whole (c) Experimental and numerical substructure Figure 2.3: Concept of the proposed methodology of the numerical substructure.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

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발행 : 서울: 檀國大學校, 2006

Besides, the first element of the external force vector in Eq. (2.8) means the interface force acting on the top story of the numerical substructure.

문장표절률: 100%

This interface force, which is denoted by $i(t)$ from now on, is produced by the damping and restoring forces of the first story in the upper experimental substructure as follows.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

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[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천
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This interface force, which is denoted by $i(t)$ from now on, is produced by the damping and restoring forces of the first story in the upper experimental substructure as follows.

문장표절률: 39%

$i(t) = cE(1) \dot{Y}_E(1) - Y_N(m) + kE(1) Y_E(1) - Y_N(m)$ (2.9) As a result, the numerical substructure, of which equation of motion is given by Eq. (2.4), is subjected to both the interface force at the top story and the ground motion at the base.

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(2.4), is subjected to both the interface force at the top story and the ground motion at the base. The boundary condition and external load of each substructure are illustrated in Fig.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

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(2.4), is subjected to both the interface force at the top story and the ground motion at the base. The boundary condition and external load of each substructure are illustrated in Fig.

문장표절률: 38%

The boundary condition and external load of each substructure are illustrated in Figure 2.3(c). Measurement of the interface force In this section, the interface force, $i(t)$, which should be fed-back to the numerical substructure for calculating the interface acceleration, is indirectly measured by the accelerometers attached on the experimental substructure, as shown in Figure 2.1.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

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2.3 c). 2.2.2 Measurement of the interface force In this study the interface force, $i(t)$, which should be fed-back to the numerical substructure for calculating the interface acceleration, is indirectly measured from the accelerometers attached on the experimental substructure, as shown in Fig.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

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문장표절률: 100%

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

Dynamic equilibrium in the experimental substructure is applied to derive the interface force using acceleration response measurement of the experimental substructure.

문장표절률: 80%

As shown in Figure 2.4, dynamic equilibrium in the experimental substructure is established between the interface force acting at its base and the resultant of its inertial forces, because internal forces between each story are canceled with each other.

문장표절률: 57%

This relation can be derived in another way. First, the dotted parts are shown in Eq. (2.5) are removed to the rightside of Eq. (2.3). Then the relation of Eq.

문장표절률: 44%

(2.9) is 16 Chapter. 2. Hybrid Testing Method applied to the rightside. Finally, the summation of simultaneous equations for each side gives following expression of the interface force, $i(t)$.

문장표절률: 72%

$\sum_{i=1}^m i(t) = -mE(i)Y(i)$ (2.10) Eq. (2.10) physically implies that the interface force, which is required as an input data of the numerical substructure, can be calculated using the measured acceleration responses of the experimental substructure.

문장표절률: 88%

$u(t)$ is the 2×1 vector of input variables given by $\{i(t), Y_g(t)\}$, in which the interface force, $i(t)$, is fed-back from the experimental substructure using Eq.

문장표절률: 48%

(2.10). $O(t)$ 17 Chapter. 2. Hybrid Testing Method is the $m \times 1$ vector of the output composed of the absolute accelerations of the numerical substructure.

시스템 설계

저자 : 박은천

발행 : 서울 :檀國大學校, 2006

2.2. Dynamic equilibrium in the experimental substructure is applied to derive the interface force using acceleration response measurement of the experimental substructure.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

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저자 : 박은천

발행 : 서울 :檀國大學校, 2006

This relation can be derived in another way. First, the dotted parts shown in Eq. (2.5) are removed to the rightside of Eq. (2.3). Then the relation of Eq.

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[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울 :檀國大學校, 2006

(10). $\{0\}$ is the mxl vector of the output composed of the absolute accelerations of the numerical substructure. The system matrices $[A]$, $[S]$, $[C]$ and $[D]$ have the dimension of $2m \times 2m$, $2m \times 2$, $m \times 2m$ and $m \times 2$, respectively, and are expressed as $[A] \begin{bmatrix} 0 & I \\ I & 0 \end{bmatrix}$ (2.12) $[B] \begin{bmatrix} 0 \\ 0 \end{bmatrix}$, $\{0\}$ $[A/v]$, $\{-1\}$ (2.13) $[C] = -[M]N_y \begin{bmatrix} K & I \\ I & -[M]^{-1}C_j \end{bmatrix}$ $[D] = [[M]N] - W_b \begin{bmatrix} 0 \\ 0 \end{bmatrix}$ (2.14) (2.15) where, $[0]$ and $[I]$ are $m \times m$ zero and unit matrices, respectively, and $\{0\}$ and $\{-1\}$ are an mxl vector of 0 and an mxl vector respectively, $\{b\}$ is an mxl vector given by $\{1, 0, \dots, 0\}^T$.

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$\{0\}$ is an mxl vector given by $\{1, 0, \dots, 0\}^T$ Eq. (2.11) with the inputs of the interface force and ground motion is incorporated in the 'Calculating' part shown in Fig.

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2.2, and produces the absolute acceleration responses of the numerical substructure online. Among those acceleration responses, the one corresponding to the top story is utilized as the reference signal to operate shaking table.

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문장표절률: 54%

The system matrices A , B , C and D have the dimension of $2m \times 2m$, $2m \times 2$, $m \times 2m$ and $m \times 2$, respectively, and are expressed as $[A] \begin{bmatrix} 0 & I \\ I & 0 \end{bmatrix}$ (2.12) $[B] \begin{bmatrix} 0 \\ 0 \end{bmatrix}$, $\{0\}$ $[M]N_y \begin{bmatrix} K & I \\ I & -[M]^{-1}C_j \end{bmatrix}$ $[D] = [[M]N] - W_b \begin{bmatrix} 0 \\ 0 \end{bmatrix}$ (2.14) (2.15) where, $[0]$ and $[I]$ are $m \times m$ zero and unit matrices, respectively, and $\{0\}$ and $\{-1\}$ are an mxl vector of 0 and an mxl vector of -1, respectively.

문장표절률: 44%

b is an $m \times 1$ vector given by $\{1, 0, \dots, 0\}^T$. Eq. (2.11) with the inputs of the interface force and ground motion is incorporated in the 'Calculating' part shown in Figure 2.1, and produces the absolute acceleration responses of the numerical substructure on-line.

문장표절률: 100%

Among those acceleration responses, the one corresponding to the top story is utilized as the reference signal to operate shaking table.

문장표절률: 86%

2.1.2 Numerical Substructure For the verification experiment of the proposed methodology, the objectstructure is assumed to be a five-story shear type buildings structure, of which two upper stories are assumed to be the experimental substructure as shown in Figure 2.15.

문장표절률: 100%

As a result, the numerical substructure is the three lowerstories of the whole structure. For the construction of the finite element model of the numerical substructure, the inter-story damping and stiffness coefficients of the shear-type experimental substructure are identified based on the measured acceleration responses, and the mass of the two floors are measured directly.

문장표절률: 29%

Table 2.1: Frequencies observed in the time records of the experimental substructure from the test without feedback and the natural frequencies of the assumed whole structure Mode Frequency components (Hz) observed (Hz) of the whole in the experimental structure with 5 DOFs substructure with 2 DOFs 1 1.3 1.3 2 2.5 4.2 3 4.2 7.1 4 7.1 9.4 5 8.6 10.8 The structural parameters for the first story of the experimental structure given in Eq.

문장표절률: 26%

(2.16) are applied to all stories of the numerical substructure and summarized as the following: 19 Chapter. 2. Hybrid Testing Method $i(t) = -mE(1)YE(1)(t) - mE(2)YE(2)(t)$ (2.17) $- + i(t)$ (2.18) $YN(3)(t) = -cN(3)YN(3) - YN(2)N(3)YN(3) - YN(2)Eq.$

문장표절률: 93%

(2.18) means that the interface force, which is produced by the shaking table, is calculated using the two measured absolute accelerations of the experimental substructure.

문장표절률: 39%

Then, the interface acceleration is calculated from the numerical substructure expressed by Eq. (2.11). Figure 2.6 represents the block diagram of the entire substructuring testing system including the numerical substructure marked by the sh

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It is observed that they agree well with each other, 20 2. REAL-TIME SUBSTRUCTURING TECHNIQUE 6 8 10 12 16 18 20 (Deg.) o 100 200 300 400 0 2 4 6 8 10 12 14 16 18 20 Frequency (Hz) Figure 2-9 Compensation result for the dynamic characteristic of shaking table 2.3.3 Numerical Substructure For the verification experiment of the proposed methodology, the objectstructure is assumed to be a five-story shear type buildingstructure, of which two upper stories are assumed to be the experimental substructure as shown in Fig.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

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2.8. As a result, the numerical substructure is the three lowerstories of the whole structure. For the construction of the finite element model of the numerical substructure, the inter-story damping and stiffness coefficients of the shear-type experimental substructure are identified based on the measured acceleration responses and the mass of the two floors are measured directly.

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발행 : 서울 :檀國大學校, 2006

2.8. As a result, the numerical substructure is the three lowerstories of the whole structure. For the construction of the finite element model of the numerical substructure, the inter-story damping and stiffness coefficients of the shear-type experimental substructure are identified based on the measured acceleration responses and the mass of the two floors are measured directly.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천
발행 : 서울 : 단국대학교 대학원, 2007.2

Table 2 1 Frequencies observed in the time records of experimental substructure from the test without feedback and natural frequencies of the assumed whole structure Mode Frequency components (Hz) observed in the experimental substructure with 2 DOFs Natural frequencies (Hz) of the whole structure with 5 DOFs 1 1.3 1.3 2 2.5 4.2 3 4.2 7.1 4 7.1 9.4 5 8.6 10.8 26 2.

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발행 : 서울 : 단국대학교 대학원, 2007.2

(2.18) are applied to all stories of the numerical substructure and summarized as the following: $mN(1) = m_1 = 2.04, mN(2) = m_2 = 2.04, mN(3) = m_3 = 2.04$ (kg) $V(1) = kV = 45.3, kN(2) = k_2 = 45.3, V(3) = h = 45-3$ (N/m) $cN(1) = c_1 = 0.023, cN(2) = c_2 = 0.023, cN(3) = c_3 = 0.023$ (N · s/m) With these parameters for the numerical substructure the interface force interface acceleration can be written as follows.

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$I'(0) = \sim fnE(J)Y, m(0 - mE(2)YEi2)(t) \cdots \text{차} V(3)(0 - \text{Ci}V(3)(\sim V(3) - \sim V(2)) - \sim N(3)\sim V(3) + \{t\} Eq. (2.20)$ means that the interface force, which is produced by the shaking table, is calculated using the two measured absolute accelerations of the experimental substructure.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

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발행 : 서울 : 단국대학교 대학원, 2007.2

aded area.

문장표절률: 77%

In Figure 2.6, the ground acceleration, \ddot{Y}_g , is not a measured signal but an input data pre-scribed by a user. Finally, the shaking table motion is controlled using the inverse transfer function of Eq.

문장표절률: 62%

(2.36) to minimize the errors between the interface acceleration computed as the third story absolute acceleration of the numerical substructure, $\ddot{Y}_N(3)$, and the actual shaking table acceleration.

문장표절률: 40%

Comparison between the structural responses obtained by the hybrid testing method and the conventional shaking table test of a single story steel frame with TLD is made in order to verify the accuracy of the hybrid testing method and the uncontrolled and TLD-controlled structural responses of a three-story structure are obtained by the hybrid testing method in both time and frequency domains.

문장표절률: 55%

2.2.1 Hybrid Testing Method with TLD Figure 2.7 depicts the conceptual illustrations of the hybrid testing method for an n -degrees-of-freedom structural model which is excited by base acceleration and has a tuned liquid damper at its top story.

문장표절률: 90%

First, the whole control system is separated into the lower part structure, and the upper part TLD and the interaction force between the structure and TLD are considered.

문장표절률: 88%

The TLD with the interacting force at its bottom is physically tested, and the response of the structure with interacting force at the top floor and the base acceleration is numerically calculated by using the computer controlling the motion of the shaking table.

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Then, the interface acceleration is calculated from the numerical substructure expressed by Eq. (2.11) Fig. 2.11 represents the block diagram of entire real-time substructuring experimental system including the numerical substructure marked by the shaded area.

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저자: 박은천

발행: 서울: 단국대학교 대학원, 2007.2

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발행: 서울: 단국대학교 대학원, 2007.2

(2.37) to minimize the errors between the interface acceleration computed as the third story absolute acceleration of the numerical substructure, $\ddot{Y}_N(3)$, and the actual shaking table acceleration.

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저자: 박은천

발행: 서울: 단국대학교 대학원, 2007.2

Comparison between the structural responses obtained by the RHSTTM and the conventional shaking table test of a single story steel frame with TLD/TLCD is made in order to verify the accuracy of the RHSTTM and the uncontrolled and TLD/TLCD-controlled structural responses of a three story structure are obtained by the RHSTTM in both time and frequency domains.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자: 박은천

발행: 서울: 단국대학교 대학원, 2007.2

3.1 depicts the conceptual illustrations of the RHSTTM for an n -degrees-of-freedom structural model which is excited by base acceleration and has a tuned liquid damper at its top story.

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저자: 박은천

발행: 서울: 단국대학교 대학원, 2007.2

First, the whole control system is separated into the lower part structure, and the upper part TLD and the interaction force between the structure and TLD is considered. The TLD with the interacting force at its bottom is physically tested and the response of the structure with interacting force at the top floor and the base acceleration is numerically calculated by using the computer controlling motion of the shaking table.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자: 박은천

발행: 서울:檀國大學校, 2006

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저자: 박은천

발행: 서울: 단국대학교 대학원, 2007.2

First the whole control system is separated into the lower part structure, and the upper part TLD and the interaction force between the structure and TLD is considered. The TLD with the interacting force at its bottom is physically tested and the response of the structure with interacting force at the top floor and the base acceleration is numerically calculated by using the computer controlling motion of the shaking table.

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저자: 박은천

발행 : 서울 : 檀國大學校, 2006

The TLD with the interacting force at its bottom is physically tested and the response of the structure with interacting force at the top floor and the base acceleration is numerically calculated by using the computer controlling motion of the shaking table.

문장표절률: 88%

Measurement of interacting force is easily accomplished by installing a shear-type load-cell at the bottom of the TLD, as shown in Figure 2.7.

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저자 : 박은천

발행 : 서울 : 檀國大學校, 2006

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저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

Measurement of interacting force is easily accomplished by installing a shear-type load-cell at the bottom of the TLD, as shown in Fig.

문장표절률: 100%

TLD-generated shear force is fed back to the control computer. With this feedback interacting force, the structural response of the story, where a TLD is installed, is calculated using the numerical part.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스템 설계

저자 : 박은천

발행 : 서울 : 檀國大學校, 2006

3.1. TLD-generated shear force is fed back to the control computer. With this feedback interacting force, the structural response of the story, where a TLD is installed, is calculated using the numerical part. The shaking table excites the upper TLD according to this calculated response.

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저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

3.1. TLD-generated shear force is fed back to the control computer. With this feedback interacting force, the structural response of the story, where a TLD is installed, is calculated using the numerical part.

문장표절률: 45%

The shaking table excites the upper TLD according to this calculated response. This process is carried out on real-time. The numerical part with n-DOFs, which is subjected to the excitations of the measured control force, $i_e(t)$, and the input acceleration, $\ddot{z}_0(t)$, at its top and bottom, respectively, as enclosed in dotted line in Figure 2.7, is calculated by $M\ddot{Y}_i(t) + C\dot{Y}_i(t) + KY_i(t) = p(t)$ (2.19) Chapter.

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저자 : 박은천

발행 : 서울 : 檀國大學校, 2006

Concept of the real-time hybrid testing method (TLCD) Of these procedures the numerical part with n-DOFs, which is subjected to the excitations of the experimentally measured control force, $i_e(t)$, and the input acceleration, $\ddot{z}_0(t)$, at its top and bottom, respectively, as enclosed in dotted line in Fig.

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저자 : 박은천

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The shaking table excites the upper TLD according to this calculated response. This process is carried out on real-time. Figure 3-1.

문장표절률: 15%

2. Hybrid Testing Method assessing TLD TLD Experimental Part TLD $i(t)$ $Y(t)$ n m $Y(t)$ n m n $Y(t)$ Cutting at $n-1$ m TLD interface $Y(t)$ $n-1$ $n-1$ m $n-1$ sen sing Shaking Table $i(t)$ Numerical Part $Y(t)$ $Y(t)$ 1 1 m 1 1 $\&z\&(t)$ $\&z\&(t)$ 0 0 0 shear type load cell $\&z\&(t)$ $\&z\&(t)$ 0 0 Figure 2.7: Conceptual view of the hybrid shaking table test where, $Y_i(t)$ is the absolute displacement at the i th ($i = 1 \rightarrow n$) story, and the location vector of external forces with the length of n , $p(t)$ equals to $\{-i_e(t), 0, \dots, 0, c\ddot{z}_0(t) + k\ddot{z}_0(t)\}$, in which subscript "e" denotes the "experimentally" measured interacting force.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

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4.1 is calculated by $[M]\{\ddot{r}\} + [C]\{\dot{r}\} + [K]\{r\} = \{p(r)\}$ (4.1) where, $\{r\}$ is the absolute displacement at the i th ($i = 1 \sim n$) story, and the location vector of external forces with the length of n , $p(t)$ equals to $\{i_e(t), 0, c\ddot{z}_0(t) + k\ddot{z}_0(t)\} \cdot \Delta t$ so, the structural mass, damping, and stiffness matrices are represented by $[M] = m_{nn}$, $[C] = c_{nn}$, $[K] = K_{nn}$, $\sim k_{nn}$, $k_{nA} = V_1 \dots$.

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저자 : 박은천

발행 : 서울 : 檀國大學校, 2006

4.1, is calculated by $[A]\{\ddot{r}\} + [C]\{\dot{r}\} + [K]\{r\} = \{f(t)\}$ (4.1) where, $\{r\}$ is the absolute displacement at the i th ($i = 1 \sim n$) story, and the location vector of external forces with the length of n , $p(t)$ equals to $\{-i_e(t), 0, c\ddot{z}_0(t) + k\ddot{z}_0(t)\}$.

문장표절률: 76%

In this section, the vibration control effect of a TLCD for a building structure ex

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cited by earthquake load is experimentally evaluated through the hybrid testing method.

저자 : 박은천
발행 : 서울: 檀國大學校, 2006

In this paper, the vibration control effect of a TLCD for a building structure excited by earthquake load is experimentally evaluated through the real-time hybrid shaking table testing method (RHSTTM).

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천
발행 : 서울: 단국대학교 대학원, 2007.2

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문장표절률: 77%

The hybrid testing method does not require a physical building structural model in performing the experiment of a TLCD-structure interactions system, and it only uses a TLCD.

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저자 : 박은천
발행 : 서울: 단국대학교 대학원, 2007.2

The RHSTTM does not require a physical building structural model in performing the experiment of a TLCD-structure interactions system and it only uses a TLCD.

문장표절률: 100%

The structural responses of the interactions system are calculated numerically in real time using the analytical structural model with the excitations of measured control force, user-defined base earthquake loads, and its statespace realization incorporated in the integrated controller of the shaking table.

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문장표절률: 100%

Also, in order to minimize the distortion of the acceleration of the shaking table, the inverse transfer function of the shaking table is identified, and its statespace realization is implemented in the shaking table controller.

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발행 : 서울: 檀國大學校, 2006

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발행 : 서울: 단국대학교 대학원, 2007.2

Also, in order to minimize the distortion of the acceleration of the shaking table, the inverse transfer function of the shaking table is identified and its statespace realization is implemented in the shaking table controller.

문장표절률: 37%

The shaking table behaves as the absolute acceleration of the TLCD mounted floor by calculating the feed back signal of the shear force signal measured by the load cell positioned between the TLCD and plate of the shaking table.

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저자 : 박은천
발행 : 서울: 檀國大學校, 2006

The shaking table reproduces the absolute acceleration of the TLCD installed floor by modulating the feed back gain of the shear force signal measured by the load cell positioned between the TLCD and the shaking table.

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발행 : 서울: 단국대학교 대학원, 2007.2

문장표절률: 31%

Comparison results between the structural responses obtained by the hybrid testing method and the conventional shaking table test of a single story steel frame with TLCD are made to verify the accuracy of the hybrid testing method in both time and frequency domains.

The shaking table reproduces the absolute acceleration of the TLCD installed floor by modulating the feed back gain of the shear force signal measured by the load cell positioned between the TLCD and the shaking table.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울 :檀國大學校, 2006

Comparison between the structural responses obtained by the RHSTTM and the conventional shaking table test of a single story steel frame with TLCD is made in order to verify the accuracy of the RHSTTM in both time and frequency domains.

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저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

Comparison between the structural responses obtained by the RHSTTM and the conventional shaking table test of a single story steel frame with TLCD is made in order to verify the accuracy of the RHSTTM in both time and frequency domains.

문장표절률: 100%

The whole structural control system, which a TLCD was installed onto the structural model with n-degrees-of-freedom at its top story, is separated, and as the result of that, the force interacts at their interface.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울 :檀國大學校, 2006

4.2 Real-Time Hybrid Shaking Table Testing Method with TLCD Fig. 1 shows the conceptual view of the experiment. The whole structural control system, which a TLCD was installed onto the structural model with n-degrees-of-freedom at its top story, is separated, and as the result of that, the force interacts at their interface.

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저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

4.2 Real-Time Hybrid Shaking Table Testing Method with TLCD Fig. 1 shows the conceptual view of the experiment. The whole structural control system, which a TLCD was installed onto the structural model with n-degrees-of-freedom at its top story, is separated, and as the result of that, the force interacts at their interface.

문장표절률: 98%

The upper part of TLCD with the interacting force at its bottom is physically tested and the lower part of the structural model with the interacting force and the input motion at its top story and base, respectively, is numerically calculated within the computer to control the motion of shaking table.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

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The upper part of TLCD with the interacting force at its bottom is physically tested and the lower part of structural model with the interacting force and the input motion at its top story and base, respectively, is numerically calculated within the computer to control the motion of shaking table.

문장표절률: 100%

For the experimental implementation, interacting or control force generated by a TLCD, which is observed from a load-cell, is fed back to the control computer.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

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With the fed-back interacting force, the structural response of the story, where a TLCD is incorporated, is calculated from the numerical part.

문장표절률: 14%

The shaking table excites the upper TLCD with this calculated response. These processes are carried out on real-time. 5VOFE -JRVJE \$PMVNO %BNQFS i & BTTFTTJOH1YBQSUFJSNFOUBM TFOTJOH 4IBLJOH 5BCMF t) Yn(t) Yn(t) mn mn i(t) Yn□1(t) n□1 5-\$V%UU JJOOUHF BSGUB DF Yn□1(t) mn□1 1/BVSNU FSJDBM TIFBS UZQF MPBE DFMM Y1(t) Y1(t) m1 m1 z0(t) z0(t) z0(t) z0(t) Figure 2.8: Concept of the hybrid testing method (TLCD) Of theses procedures, the numerical part with n-DOFs, which is subjected to the excitations of the experimentally measured control force, ie(t), and the input acceleration, z0(t), at its top and bottom, respectively, as enclosed in dotted line in Figure 2.8, is calculated by $MY_i(t) + CY_i(t) + KY_i(t) = p(t)$ (2.26) where, $Y_i(t)$ is the absolute displacement at the i th ($i = 1 \rightarrow n$) story, and the location vector of external forces with the length of n , $p(t)$ equals to $\{-ie(t), 0, \dots, 0, c1z0(t) + k1z0(t)\}$, in which subscript "e" denotes the "experimentally" measured interacting force.

문장표절률: 76%

The shaking table shown in Figure 2.11 moves in accordance with the control signal, which is generated by the control computer and sent through D/A conversion board.

문장표절률: 59%

In almost every case, the target acceleration signal and actual acceleration produced by the shaking table are different in their amplitudes and phases due to the shaking table dynamics.

문장표절률: 94%

Therefore, in order to compensate the distortion of the actual shaking table acceleration against the shaking table dynamics existing between the reference signal and the actual measured acceleration of the shaking table, the inverse transfer function of the actual acceleration of shaking table with respect to the command signal generated by the control computer is constructed and implemented in the shaking table control computer as shown in Figure 2.15.

문장표절률: 57%

First, the inverse transfer function, of which amplitude and phase are represented in Figure 2.12 by the dashed line, is obtained experimentally.

문장표절률: 100%

Then, the experimental inverse transfer function is approximated by a rational function for its implementation in the control computer.

한글 표절률

저자 : 박은천
발행 : 서울:檀國大學校, 2006

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[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천
발행 : 서울:단국대학교 대학원, 2007.2

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저자 : 박은천
발행 : 서울:단국대학교 대학원, 2007.2

Concept of the real-time hybrid testing method (TLCD) Of theses procedures, the numerical part with n-DOFs, which is subjected to the excitations of the experimentally measured control force, ij), and the input acceleration, z0(t), at its top and bottom, respectively, as enclosed in dotted line in Fig.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천
발행 : 서울:檀國大學校, 2006

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발행 : 서울:단국대학교 대학원, 2007.2

2.6. The shaking table shown in Fig. 2.6 moves in accordance with the control signal, which is generated by the control computer and sent through D/A conversion board.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

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발행 : 서울:단국대학교 대학원, 2007.2

2.8. First, the inverse transfer function, of which amplitude and phase are represented in Fig. 2.7 by the dashed line, is obtained experimentally.

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저자 : 박은천
발행 : 서울:단국대학교 대학원, 2007.2

Then, the experimental inverse transfer function is approximated by a rational function for its implementation in the control computer.

문장표절률: 60%

First, the shaking table test without feedback loop of accelerations measured from the experimental substructure into the shaking table controller in Figure 2.6 is performed to confirm the validity of the proposed method experimentally.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스템 설계

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발행 : 서울:단국대학교 대학원, 2007.2

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문장표절률: 100%

In this case, the third story acceleration of responses pre-calculated from the assumed whole building with structural parameters such as Eqs.

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문장표절률: 59%

(2.16) and (2.17) is used as a reference signal in Figure 2.15. Figure 2.25 compares the time histories and Fourier transform of the acceleration responses, which are measured from the experimental substructure and shaking table, with those calculated from the numerical analysis of the whole assumed structure.

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(2.18) and (2.19) is used as a reference signal in Fig. 2.8. Fig. 2.12 compares the time histories and Fourier transform of the acceleration responses, which are measured from the experimental substructure and shaking table, with those calculated from the numerical analysis of the assumed whole structure.

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문장표절률: 62%

In other words, the responses corresponding to the 3rd, 4th and 5th story accelerations are compared. Also, Figures 2.26~2.28 showing the variation of frequency components according to time lapse illustrate the spectrogram and contour plots of experimental and numerical accelerations of the 3rd, 4th and 5th story, respectively.

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[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스템 설계

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문장표절률: 55%

As can be confirmed from Figure 2.25, the experimental accelerations obtained from the shaking table test without their feedback agree well with those obtained from the analysis of the whole assumed structure in both time and frequency domains.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스템 설계

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발행 : 서울:檀國大學校, 2006

As can be confirmed from Fig. 2.12, the experimental accelerations obtained from the shaking table test without their feedback agree well with those obtained from the analysis of the assumed whole structure in both time and frequency domains.

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저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

As can be confirmed from Fig. 2.12, the experimental accelerations obtained from the shaking table test without their feedback agree well with those obtained from the analysis of the assumed whole structure in both time and frequency domains.

문장표절률: 46%

However, it can be observed that the small discrepancies are shown in the time histories of 4th and 5th story accelerations as shown from Figure 2.25, while the 3rd story acceleration is identical to the numerical one over the entire time history as known from Figures 2.25~2.26.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

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However, it can be observed that the small discrepancies are shown in the time histories of 4th and 5th story accelerations as shown from Fig.

문장표절률: 63%

These differences are caused by the inherent modes of experimental substructure; the frequency components of 2.5 and 8.6 Hz corresponding to the first and second modes of the experimental substructure, respectively, are observed in the measured acceleration of 4th story as known from Figure 2.27, and also the component of 2.5 Hz is expressed in the 5th story experimental acceleration as like Figure 2.28.

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문장표절률: 54%

It is considered that this tendency is especially conspicuous in the case of utilizing a lightly-damped testing model as an experimental substructure.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

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2.14, and also the component of 2.5 Hz is expressed in the 5th story experimental acceleration as like Fig. 2.15. It is considered that this tendency is especially conspicuous in case of utilizing a lightly-damped testing model as an experimental substructure.

문장표절률: 83%

Table 1 shows the frequency components observed in the time records of an experimental substructure from the test without its acceleration feedback and the natural frequencies calculated from the assumed whole structure with five stories.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

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발행 : 서울 :檀國大學校, 2006

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문장표절률: 100%

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

From Table 2.1, it can be noted that the first natural frequency of the experimental substructure is shifted from 2.5 Hz to 1.3 Hz by the dynamics of the three-story numerical substructure added to its base.

문장표절률: 59%

Then, the substructuring testing expressed as Figure 2.6 was carried out based on the acceleration feedback of the experimental substructure.

문장표절률: 30%

2. Hybrid Testing Method culated from the numerical analysis of the whole assumed structure with 5 DOFs. Also, Figures 2.30~2.32 express the frequency components according to time lapse, which is observed in the measured responses from the test with feedback and the calculated ones of the 3rd, fourth and fifth story, respectively.

문장표절률: 59%

As known from Figure 2.29, the inclination in entire time history responses measured from the proposed testing method agrees well with that calculated from the whole assumed structure.

문장표절률: 44%

These are why the first mode responses of substructured system coincide well with those of the whole supposed system over the entire time range, as shown in Figures 2.30~2.32.

문장표절률: 53%

However, as can be confirmed from Figures 2.30~2.32, instead of the second and third mode responses of the substructured system, those of the experimental substructure are observed from the testing results in the vicinity of 2.5 and 8.6 Hz.

문장표절률: 74%

It is considered that in the process of the acceleration feedback of the experimental substructure, its fundamental modes affect to the numerical substructure and then the numerical error occurs in calculating the numerical substructure.

인용포함 문장

문장표절률: 38%

41 Chapter. 2. Hybrid Testing Method & "YOQBFMZSTJNJTFUO 5JNF TFD (a) Time domain N T UI PPS & "YOQBFMZSTJNJTFUO 'SFRVF ODZ)[(b) Frequency domain Figure 2.25: Comparisons of results measured from the experiment without feedback and those calculated from numerical analysis.

스텝 설계

저자 : 박은천

발행 : 서울:檀國大學校, 2006

From Table 2.1, it can be noted that the first natural frequency of the experimental substructure is shifted from 2.5 Hz to 1.3 Hz by the dynamics of the three-story numerical substructure added to its base.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스텝 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

From Table 2.1, it can be noted that the first natural frequency of the experimental substructure is shifted from 2.5 Hz to 1.3 Hz by the dynamics of the three-story numerical substructure added to its base.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스텝 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

Then, the real-time substructuring shaking tabletest expressed as Fig. 2.11 was carried out based on the acceleration feedback of the experimental substructure.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스텝 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

Fig. 16 compares the responses measured from the test with the acceleration feedback of the experimental substructure with those calculated from the numerical analysis of the assumed whole structure with 5 DOFs. Also, Figs. 2.17~2.19 express the frequency components according to time lapse, which are observed in the measured responses from the test with feedback and the calculated ones of the 3rd, 4th and 5th story, respectively.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스텝 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

As known from Fig. 2.16, the inclination in entire time history responses measured from the proposed testing method agrees well with that

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스텝 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

REAL-TIME SUBSTRUCTURING TECHNIQUE calculated from the assumed whole structure. This is why the first mode responses of substructured system coincide well with those of the assumed whole system over the entire time range, as shown in Figs.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스텝 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

2.17~2.19. However, as can be confirmed from Figs. 2.17~2.19, instead of the 2nd and 3rd mode responses of substructured system, those of the experimental substructure are observed from the testing results in the vicinity of 2.5 and 8.6 Hz.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스텝 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

It is considered that in the process of the acceleration feedback of the experimental substructure, its fundamental modes affect to the numerical substructure and then numerical error occurs in calculating the numerical substructure.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스텝 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

Comparisons of results measured from the experiment without FEEDBACK AND THOSE CALCULATED FROM NUMERICAL ANALYSIS27 Figure 2-13.

문장표절률: 89%

As a result, the mass ratio of the TLD to the structure is about 1.3%. To confirm whether the numerically calculated frequency of the TLD is modulated to the structural one, the transfer function is shown in Figure 2.34, from the shaking table acceleration to the shear force by the TLD, was obtained by using the white noise excitation.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울 :檀國大學校, 2006

As a result, the mass ratio of the TLD to the structure is about 1.3 To confirm whether the numerically calculated frequency of the TLD is modulated to the structural one, the transfer function shown in Fig.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

3.5, from the shaking table acceleration to the shear force by the TLD, was obtained by using the white noise excitation. It is observed in Fig.

문장표절률: 75%

It is observed in Figure 2.34 that the TLD has the sloshing frequency of 1.25Hz which is very closeto the structural natural frequency of 1.23Hz.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울 :檀國大學校, 2006

3.5 that the TLD has the sloshing frequency of 1.25Hz which is very closeto the structural natural frequency of 1.23Hz. 43 4. RHSTTM for the Performance Evaluation of a Timed Liquid Column Damper 병자 · · 4 m r (a) Conventional shaking tabletest (b) Real-time hybrid shaking tabletest Figure 3-4.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

3.5 that the TLD has the sloshing frequency of 1.25Hz which is very closeto the structural natural frequency of 1.23Hz. 43 4. RHSTTM for the Performance Evaluation of a Tuned Liquid Column Damper (a) Conventional shaking tabletest (b) Real-time hybrid shaking tabletest Figure 3-4.

문장표절률: 52%

At first, the conventional shaking tabletest is shown in Figure 2.33(a) is performed to investigate the seismic response control performance of the TLD.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울 :檀國大學校, 2006

3.4 (a) is performed to investigate the seismic response control performance of the TLD. Previously mentioned four earthquake records are scaled to have the peak acceleration of 100 gal and used to excite the TLD-structuresystem.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

3.4 (a) is performed to investigate the seismic response control performance of the TLD- Previously mentioned four earthquake records are scaled to have the peak acceleration of 100 gal and used to excite the TLD-structuresystem.

문장표절률: 64%

Previously mentioned four earthquake records are scaled to have the peak acceleration of 100 gals and used to excite the TLD-structuresystem.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울 :檀國大學校, 2006

3.4 (a) is performed to investigate the seismic response control performance of the TLD. Previously mentioned four earthquake records are scaled to have the peak acceleration of 100 gal and used to excite the TLD-structuresystem.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

3.4 (a) is performed to investigate the seismic response control performance of the TLD- Previously mentioned four earthquake records are scaled to have the peak acceleration of 100 gal and used to excite the TLD-structuresystem.

문장표절률: 68%

Figures 2.35 and 2.36 show the measured structural acceleration responses in the time and frequency domains, respectively.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울 :檀國大學校, 2006

Figs. 3.6 and 3.7 show the measured structural acceleration responses in the time and frequency domains, respectively. It is observed from Fig.

문장표절률: 62%

2. Hybrid Testing Method 2.6.3 Hybrid Testing Method of Three Story Structure with a TLD The control performance of a TLD installed in a three-story structure is investigated by using the hybrid testing method.

문장표절률: 54%

The structure is assumed to be a three story shear-type model, which has identical story properties as follows; $m_i = 128.8\text{kg}$, $c_i = 13.52\text{N}\cdot\text{s/m}$, $k_i = 33908\text{N/m}$ for $i = 1, 2, 3$.

문장표절률: 76%

The structure has natural frequencies of 1.15Hz, 3.22Hz and 4.65Hz. The TLD discussed in the previous section is used, and its water level is modulated to 4.6 cm for the TLD to have sloshing frequency of 1.15Hz.

문장표절률: 100%

As a result, the mass ratio of the TLD to the structure is about 2%. The four earthquake waves used for the excitation of the single story steel frame were scaled to have peak acceleration of 40gal.

문장표절률: 100%

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

Figs. 3.6 and 3/7 show the measured structural acceleration responses in the time and frequency domains, respectively. It is observed from Fig.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울 :檀國大學校, 2006

RHSTTM for the Performance Evaluation of a Timed Liquid Column Damper 3.4.2 A Three Story Structure with a TLD The control performance of a TLD installed in a three story structure is investigated by using the RHSTTM.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

RHSTTM for the Performance Evaluation of a Tuned Liquid Column Damper 3.4.2 A Three Story Structure with a TLD The control performance of a TLD installed in a three story structure is investigated by using the RHSTTM.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울 :檀國大學校, 2006

The structure is assumed to be a three story shear-type model, which has identical story properties as follows; $W_i = 128.8\text{kg}$, $c_i = 13.52\text{N}\cdot\text{s/m}$, $k_i = 33908\text{N/m}$ for $i = 1, 2, 3$.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

The structure is assumed to be a three story shear-type model, which has identical story properties as follows; $m_i = 128.8\text{kg}$, $c_i = 13.52\text{N}\cdot\text{s/m}$, $k_i = 33908\text{N/m}$ for $i = 1, 2, 3$.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울 :檀國大學校, 2006

The structure has natural frequencies of 1.15Hz, 3.22Hz and 4.65Hz. The TLD discussed in the previous section is used and its water level is modulated to 4.6 cm in order for the TLD to have sloshing frequency of 1.15Hz.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

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[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울 :檀國大學校, 2006

As a result, the mass ratio of the TLD to the structure is about 2%. The four earthquake waves used for the excitation of the single story steel frame were scaled to have peak acceleration of 40gal.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

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[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

The uncontrolled structural responses were obtained by removing the feedback loop of the TLD-generated interacting force, which causes the numerical structural model to be excited only by the base earthquake motion.

문장표절률: 72%

Figures 2.39 and 2.40 compare the uncontrolled and controlled accelerations of the third story in time and frequency domains, respectively, which is realized by the shaking table through the hybrid testing method.

문장표절률: 100%

It is observed that the structural accelerations are significantly reduced by the TLD, especially in the region of the fundamental frequency.

문장표절률: 69%

2. Hybrid Testing Method identified structural parameters are reflected in the numerical part expressed by the shaded region in the integrated controllers shown in Figure 2.19.

문장표절률: 78%

The continuous filters in the figure are converted into discrete ones with a time step of 0.01 sec in the actual implementation of the experiment.

문장표절률: 50%

Figure 2.42 compare the controlled accelerations experimentally measured by implementing the conventional and the hybrid testing method in both time and frequency domain, respectively.

문장표절률: 77%

The validity of the hybrid testing method performed in this paper is verified from the fact that the experimental results from two methods well coincide with each

한글 표절

저자 : 박은천
발행 : 서울: 檀國大學校, 2006

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[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천
발행 : 서울: 단국대학교 대학원, 2007.2

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[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천
발행 : 서울: 檀國大學校, 2006

Figs. 3.10 and 3.11 compare the uncontrolled and controlled accelerations of the third story in time and frequency domains, respectively, which is realized by the shaking table through the RHSTTM.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

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저자 : 박은천
발행 : 서울: 檀國大學校, 2006

It is observed that the structural accelerations are significantly reduced by the TLD, especially in the region of the fundamental frequency.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

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저자 : 박은천
발행 : 서울: 단국대학교 대학원, 2007.2

3.4 (b). For its implementation for the controlled case, the identified structural parameters are reflected in the numerical part expressed by the shaded region in the integrated controllers shown in Fig.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천
발행 : 서울: 단국대학교 대학원, 2007.2

RHSTTM for the Performance Evaluation of a Tuned Liquid Column Damper in Fig. 4.3. The continuous filters in the figure are converted into discrete ones with a time step of 0.01 sec in actual implementation of the experiment.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천
발행 : 서울: 단국대학교 대학원, 2007.2

Fig. 4.5 compare the controlled accelerations experimentally measured by implementing the conventional and the real-time hybrid shaking table tests in both time and frequency domain, respectively.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

other on the whole.

문장표절률: 20%

BDD H UJNF T FD (a) El Centro Earthquake(time domain) BDD H UJNF T F D (c) Kobe Earthquake(time domain) H TFD ' SFR VFO D Z) [(b) El Centro Earthquake(frequency do- main) H TFD ' SFR VFO D Z) [(d) Kobe Earthquake(frequency domain) Figure 2.42: Comparisons between the results from the conventional testing method(dotted line) and those from the hybrid testing method(solid line) for the controlled response 60 Chapter.

문장표절률: 83%

Figures 2.53 and 2.54 show the displacement and acceleration response in the frequency domain, respectively. It is observed that the displacement response of the shaking table was reduced by 78% for the case in the TMD control direction.

문장표절률: 40%

The displacement control performance index of a TMD is 0.22, as shown in Figure 2.53. Also, the acceleration response of the shaking table was reduced by 78 %, as shown in Figure 2.54.

문장표절률: 59%

Figure 2.55 shows the displacement in the time domain. The response of the shaking table is considerably reduced by 78% at a resonance frequency of 0.82 Hz.

문장표절률: 45%

Then, the test was performed in the TLCD control direction. Figures 2.56 and 2.57 show the displacement and acceleration response of the shaking table in the frequency domain, respectively.

문장표절률: 100%

It is observed that the displacement response was reduced by 71% for the case in the TLCD control direction, and the acceleration response reduced by 70%.

문장표절률: 21%

Also, the displacement and acceleration response of the shaking table are considerably reduced at 64 Chapter. 2. Hybrid Testing Method VDPDOPUSOPUMS PM 'SFRVF O D Z) [Figure 2.48: Acceleration in the frequency domain (TMD direction) V DP OODPUSOPUMSPM 5JN F T Figure 2.49: Displacement in the time domain (TMD direction, 0.82 Hz) a resonance frequency of 0.73 Hz, respectively, as shown in Figure 2.58.

문장표절률: 58%

The lower part of the whole structure or whole buildingstructure is modeled numerically. In order to verify the validity and accuracy of the proposed technique, a shaking tabletest was conducted.

저자: 박은천
발행: 서울: 단국대학교 대학원, 2007.2

The validity of the real-time hybrid shaking tabletest performed in this paper is verified from the fact that the experimental results from two methods well coincide with each other on the whole.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자: 박은천
발행: 서울: 단국대학교 대학원, 2007.2

Comparisons between the results from the conventional testing method(dotted line) and those from the RHSTTM(solid line) for the controlled response 61 4.

[Copykiller] Design of a Bi-directional Tuned Liquid Mass Damper for Controlling Dynamic Responses of Building Structures : 건축구조물의 동적응답제어를 위한 2방향 동조액체질량감쇠기 설계

저자: 허재성
발행: 서울: 단국대학교 대학원, 2008

Fig.4.17 shows the displacement and acceleration response in the frequency domain, respectively. It is observed that the displacement response of the shaking table was reduced by 78 for the case in the TMD control direction.

[Copykiller] Design of a Bi-directional Tuned Liquid Mass Damper for Controlling Dynamic Responses of Building Structures : 건축구조물의 동적응답제어를 위한 2방향 동조액체질량감쇠기 설계

저자: 허재성
발행: 서울: 단국대학교 대학원, 2008

So, The displacement control performance index of a TMD is 0.22, as shown in Fig.4.17(a). Also, the acceleration response of the shaking table was reduced by 78 %.

[Copykiller] Design of a Bi-directional Tuned Liquid Mass Damper for Controlling Dynamic Responses of Building Structures : 건축구조물의 동적응답제어를 위한 2방향 동조액체질량감쇠기 설계

저자: 허재성
발행: 서울: 단국대학교 대학원, 2008

Displacement and acceleration in the time domain by the real-time hybrid test (TMD direction, 0.83Hz) Fig.4.18 shows the displacement and acceleration response in the time domain, respectively, the response of the shaking table is considerably reduced by 78% at a resonance frequency of 0.83 Hz.

[Copykiller] Design of a Bi-directional Tuned Liquid Mass Damper for Controlling Dynamic Responses of Building Structures : 건축구조물의 동적응답제어를 위한 2방향 동조액체질량감쇠기 설계

저자: 허재성
발행: 서울: 단국대학교 대학원, 2008

Fig.4.19 shows the displacement and acceleration response of the shaking table in the frequency domain, respectively. It is observed that the displacement response was reduced by 71 % for the case in the TLCD control direction, and the acceleration response reduced by 70 %.

[Copykiller] Design of a Bi-directional Tuned Liquid Mass Damper for Controlling Dynamic Responses of Building Structures : 건축구조물의 동적응답제어를 위한 2방향 동조액체질량감쇠기 설계

저자: 허재성
발행: 서울: 단국대학교 대학원, 2008

Fig.4.19 shows the displacement and acceleration response of the shaking table in the frequency domain respectively. It is observed that the displacement response was reduced by 71 for the case in the TLCD control direction, and the acceleration response reduced by 70 %.

[Copykiller] Design of a Bi-directional Tuned Liquid Mass Damper for Controlling Dynamic Responses of Building Structures : 건축구조물의 동적응답제어를 위한 2방향 동조액체질량감쇠기 설계

저자: 허재성
발행: 서울: 단국대학교 대학원, 2008

Time (s) Time (s) Figure 4-20. Displacement and acceleration in the time domain by the real-time hybrid test (TLCD direction, 0.73 Hz) Also, the displacement and acceleration response of the shaking table are considerably reduced at a resonance frequency of 0.73 Hz, respectively.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자: 박은천
발행: 서울: 단국대학교 대학원, 2006

In order to verify the validity and accuracy of the proposed technique, a shaking tabletest was conducted. The result of the study can be summarized as follows

문장표절률: 75%

The result of the study can be summarized as follows. 1. To reduce the distortion of the interface acceleration, the inverse transfer function of the shaking table was identified, and its statespace realization was implemented in the shaking table controller.

문장표절률: 100%

2. In this paper, the linear transfer function approach for controlling the motion of a shaking table was considered to experimentally verify the proposed method for a linear experimental part.

문장표절률: 58%

However, this approach would be inappropriate in a coupled non-linear system leading to experimental instability. Therefore, in such case, the controller using the inverse transfer function of shaking table, shown in Figure 2.6, would be modified to compensate an experimental instability.

문장표절률: 95%

3. The interface force between the experimental and numerical structures was obtained using only acceleration measurement and mass information so that high-capacity loads cell and installation jigs are not required in the structuring technique.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

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[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울 :檀國大學校, 2006

(1 To reduce the distortion of the interface acceleration, the inverse transfer function of the shaking table was identified and its statespace realization was implemented in the shaking table controller.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

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저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

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[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울 :檀國大學校, 2006

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저자 : 박은천

발행 : 서울 :檀國大學校, 2006

However, this approach would be inappropriate for a coupled non-linear system leading to experimental instability. Therefore, in such case the controller using the inverse transfer function of shaking table, shown in Fig.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

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저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

11, would be modified to compensate an experimental instability. (3 The interface force between the experimental and numerical structures was obtained using only acceleration measurement and mass information so that high-capacity loads cell and installation jigs are not required in the experiment.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울 :檀國大學校, 2006

11, would be modified to compensate an experimental instability. (3 The interface force between the experimental and numerical structures was obtained using only acceleration measurement and mass information so that high-capacity loads

문장표절률: 73%

4. The proposed method basing the interface force measurement on acceleration measurements from an experimental substructure is partially available only when the mass distribution is discrete – for example, this technique would be applicable to the TMD as an experimental part.

ds cell and installation jigs are not required in the experiment.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

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저자 : 박은천

발행 : 서울 :檀國大學校, 2006

(4 The proposed method basing the interface force measurement on acceleration measurements from an experimental substructure is partially available only when the mass distribution is discrete – for example this would be applicable to the TMD as an experimental part.

문장표절률: 100%

Also, the interface force measurement using force transducers is required to perform the proposed method when wind forces are applied to the experimental substructure.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

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Also, the interface force measurement using force transducers is required to perform the proposed method when wind forces are applied to the experimental substructure.

문장표절률: 94%

6. An unexpected vibration of the experimental substructure can be induced by the feedback of responses including its inherent natural modes and then by the error occurred in calculating the numerical substructure.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울 :檀國大學校, 2006

(6) Unexpected vibration of the experimental substructure can be induced by the feedback of responses including its inherent natural modes and then by the error occurred in calculating the numerical substructure.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

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발행 : 서울 : 단국대학교 대학원, 2007.2

(6) Unexpected vibration of the experimental substructure can be induced by the feedback of responses including its inherent natural modes and then by the error occurred in calculating the numerical substructure.

문장표절률: 56%

72 Chapter. 2. Hybrid Testing Method 7. It is considered that to minimize the effect of natural modes of an experimental substructure on the substructured system, the structural model as heavily-damped as possible would be used as an experimental part.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울 :檀國大學校, 2006

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발행 : 서울 : 단국대학교 대학원, 2007.2

(2) It is considered that to minimize the effect of natural modes of an experimental substructure on the substructured system, the structural model as heavily-damped as possible would be used as an experimental part.

문장표절률: 73%

8. The proposed technique can be extended to the substructuring technique with the middle part of a whole structure in combination with the conventional substructuring technique employing lower part as the experimental substructure.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

REAL-TIME SUBSTRUCTURING TECHNIQUE possible would be used as an experimental part. (8 The proposed technique can be extended to the real-time s

substructuring technique with the middle part of a whole structure in combination with the conventional substructuring technique employing lower part as the experimental substructure.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울: 檀國大學校, 2006

REAL-TIME SUB STRUCTURING TECHNIQUE possible would be used as an experimental part. (8 The proposed technique can be extended to the real-time substructuring technique with the middle part of a whole structure in combination with the conventional substructuring technique employing lower part as the experimental substructure.

문장표절률: 82%

9. The TLD installed on the top floor of the structure is physically tested, and simultaneously numerical calculation is carried out for the assumed analytical structural model.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울: 檀國大學校, 2006

The TLD installed at the top floor of the structure is physically tested, and simultaneously numerical calculation is carried out for the assumed analytical structural model.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울: 단국대학교 대학원, 2007.2

The TLCD installed at the top floor of the structure is physically tested, and simultaneously numerical calculation is carried out for the assumed analytical structural model.

문장표절률: 26%

10. Comparison between the structural responses obtained by the hybrid testing method and the conventional shaking table test of a single story steel frame with TLD and TLCD indicates that the performance of the TLD and TLCD can be accurately evaluated using the hybrid testing method without the physical structural model.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울: 檀國大學校, 2006

Comparison between the structural responses obtained by the RHSTTM and the conventional shaking table test of a single story steel frame with TLD/TLCD indicates that the performance of the TLD/TLCD can be accurately evaluated using the RHSTTM without the physical structural model.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울: 단국대학교 대학원, 2007.2

Comparison between the structural responses obtained by the RHSTTM and the conventional shaking table test of a single story steel frame with TLD/TLCD indicates that the performance of the TLD/TLCD can be accurately evaluated using the RHSTTM without the physical structural model.

문장표절률: 88%

11. The uncontrolled and TLD-controlled structural responses of a three-story structure are obtained by the hybrid testing method in both time and frequency domains, showing that TLD can effectively mitigate the seismic responses of building structures and the hybrid testing method can reproduce the dynamic behavior of TLD-structure interaction systems for both the uncontrolled and controlled case.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울: 단국대학교 대학원, 2007.2

Finally the uncontrolled and TLD-controlled structural responses of a three story structure are obtained by the RHSTTM in both time and frequency domains, showing that TLD can effectively mitigate the seismic responses of building structures and the RHSTTM can reproduce the dynamic behavior of TLD-structure interaction systems for both the uncontrolled and controlled case.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울: 檀國大學校, 2006

Finally the uncontrolled and TLD-controlled structural responses of a three story structure are obtained by the RHSTTM in both time and frequency domains, showing that TLD can effectively mitigate the seismic responses of building structures and the RHSTTM can reproduce the dynamic behavior of TLD-structure interaction systems for both the uncontrolled and controlled case.

문장표절률: 91%

Figure 3.2(a) shows the shape of the band-stop filter in discrete Fourier transform dealing with the finite duration discrete signal as an infinite one multiplied by a rectangular window, the original signal in time domain is distorted especially in initial and final time intervals.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울: 단국대학교 대학원, 2007.2

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문장표절률: 59%

This distortion can be reduced by using an envelope functionsuch that the given deterministic wind load has ascending and descending time intervals.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

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This distortion can be reduced by using an envelop functionsuch that the given dterministic wind load has ascending and descending time intervals.

문장표절률: 98%

Although the envelope function changes the deterministic wind load, the effect of this another distortion would be trivial in evaluating the characteristics of wind load induced response because the grave concern is generally in the intermediate time of the total loading duration when the peak response is expected to occur.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

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문장표절률: 55%

Figure 3.2(b) shows the shape of the envelope function used in this study. 3.1.2 Numerical Example 76 story wind-induced benchmark buildings The wind-induced responsesimulating actuator is applied to a 76-story 306 meters office tower benchmark building which is slender with a height to width ratio of 306.1/42 = 7.3.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천
발행 : 서울 : 檀國大學校, 2006

DESIGN OF AN ACTUATOR FOR SIMULATING WIND RESPONSE The wind-induced responsesimulating actuator is applied to a 76-story 306 meters office tower benchmark building which is slender with a height to width ratio of 306.1/42 = 7.3.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천
발행 : 서울 : 단국대학교 대학원, 2007.2

DESIGN OF AN ACTUATOR FOR SIMULATING WIND RESPONSE The wind-induced responsesimulating actuator is applied to a 76-story 306 meters office tower benchmark building which is slender with a height to width ratio of 306.1/42 = 7.3.

문장표절률: 25%

Because the deterministic across-wind load 77 Chapter. 3. Design of Excitation System for Simulating Dynamic Loads &OWFMPQ GVODUJPO'SF RVFODZ) [t1 5JNF TFDPOE t2 (a) The shape of the band-stop filter (b) The shape of the envelop function Figure 3.2: Exciter gain shape of the band-stop filter and the envelop function.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천
발행 : 서울 : 단국대학교 대학원, 2007.2

0 1 c UJ t Time (second) i (b) The shape of the envelop function Figure 5-2 Exciter gain shape of the band-stop filter and the envelop function 66 5.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천
발행 : 서울 : 檀國大學校, 2006

Scheme of simulation of wind induced responses using LMS and ATMD64 Figure 5-2 Exciter gain shape of the band-stop filter and the envelop function.

문장표절률: 57%

data is given through wind tunnel tests for this benchmark building, the force of the actuator realizes target across-wind induced structural response can be calculated using Eq.

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저자 : 박은천
발행 : 서울 : 단국대학교 대학원, 2007.2

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저자 : 박은천
발행 : 서울 : 檀國大學校, 2006

Because the deterministic across-wind load data is given through wind tunnel tests for this benchmark building, the force of the actuator force realizing target a cross-wind induced structural response can be calculated using Eq.(4).

문장표절률: 92%

(3.4). In order to reduce numerical computation time, a 23 degree of freedom (DOF) state reduced-ordersystem model proposed by Yang et al.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천
발행 : 서울 : 檀國大學校, 2006

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문장표절률: 97%

(2004b). is used in this study. The wind load vector is modeled physically by lumping wind forces on adjacent floors at the locations that correspond to the 23 DOF model.

스텝 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천
발행 : 서울 : 단국대학교 대학원, 2007.2

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[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스텝 설계

저자 : 박은천
발행 : 서울 :檀國大學校, 2006

is used in this study. The wind load vector is modeled physically by lumping wind forces on adjacent floors at the locations that correspond to the 23 DOF model.

문장표절률: 73%

Figures 3.3(a), 3.3(b) shows the plan view and elevation view of the 76th benchmark building and Figures 3.3(c), 3.3(d) show the mode shape of first three mode of the structure and time history of wind-load.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스텝 설계

저자 : 박은천
발행 : 서울 :檀國大學校, 2006

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저자 : 박은천
발행 : 서울 : 단국대학교 대학원, 2007.2

3 a), (b) shows the plan view and elevation view of the 76th benchmark building and Fig.3 c), (d) show the mode shape of first three mode of the structure and time history of wind-load.

문장표절률: 92%

3.1.3 Error evaluation criteria In order to verify the effectiveness of proposed method through the comparison between the wind and actuator induced structural responses, two error criteria are considered in time and frequency domains, respectively.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스텝 설계

저자 : 박은천
발행 : 서울 :檀國大學校, 2006

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저자 : 박은천
발행 : 서울 : 단국대학교 대학원, 2007.2

5A.2 Error evaluation criteria In order to verify the effectiveness of proposed method through the comparison between the wind and actuator induced structural responses, two error criteria are considered in time and frequency domains, respectively.

문장표절률: 39%

Figure 3.4: Transfer function of 75th story responses to LMS. Figure 3.5 shows the frequency response function and the time history of the actuator force obtained without using a filter when the target response is acceleration or displacement of the 75th floor.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스텝 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

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발행 : 서울 : 단국대학교 대학원, 2007.2

Transfer function of 75th story responses to LMS Fig. 5.5 shows the frequency response function and the time history of the actuator force obtained without using filter when the target response is acceleration or displacement of 75th floor.

문장표절률: 51%

It is known from Figure 3.5 that much larger force are required for the shaker to achieve the target displacement than acceleration response, and furthermore there exist high-frequency components in Figure 3.4(b), which result in high-speed switching of control force as shown in Figure 3.5(b).

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스텝 설계

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발행 : 서울 :檀國大學校, 2006

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al-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

It is known from Fig. 5.5 that much larger force are required for the shaker to achieve the target displacement than acceleration response, and furthermore there exist high frequency components in Fig.

문장표절률: 85%

In practice, hydraulic actuators popular in civil engineering structures is not suitable for this undesirable chattering problem which causes spillover instability in higher modes, and acceleration response concerned with service ability criteria is more critical for such high-rise building excited by wind load as this benchmark building. 15' T 1 15' T 1 80 Chapter.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스템 설계

저자 : 박은천

발행 : 서울 :檀國大學校, 2006

5.4(d), which result in high speed-switching of control force as shown in Fig. 5.4(e). In practice, hydraulic actuators popular in civil engineering structures is not suitable for this undesirable chattering problem which causes spillover instability in higher modes, and acceleration response concerned with service ability criteria is more critical for such high-rise building excited by wind load as this benchmark building than displacement.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

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문장표절률: 67%

3. Design of Excitation System for Simulating Dynamic Loads than displacement. Figure 3.6 shows the comparison between the frequency responses of wind and LMS induced 75th-floor acceleration and displacement when the target response is 75th displacement.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스템 설계

저자 : 박은천

발행 : 서울 :檀國大學校, 2006

Fig. 5.6 shows the comparison between the frequency responses of wind and LMS induced 75th floor acceleration and displacement when the target response is 75th displacement.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

Fig. 5.6 shows the comparison between the frequency responses of wind and LMS induced 75th floor acceleration and displacement when the target response is 75th displacement.

문장표절률: 73%

It is obviously shown that the wind and LMD induced displacement coincide well with each other while acceleration responses are very different.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스템 설계

저자 : 박은천

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저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

It is obviously shown that wind and LMD induced displacement coincide well with each other while acceleration responses are very different.

문장표절률: 71%

Based upon the observation in Figure 3.5 and 3.6, only acceleration response is considered as target response for calculating LMD force reproducing wind induced displacement as well as acceleration in this study.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스템 설계

저자 : 박은천

발행 : 서울 :檀國大學校, 2006

Based upon the observation in Fig. 5.5 and Fig. 5.6, only acceleration response is considered as target response for calculating LMD force reproducing wind induced displacement as well as acceleration in this study.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

Based upon the observation in Fig. 5.5 and Fig. 5.6, only acceleration response is considered as target response for calculating LMD force reproducing wind induced displacement as well as acceleration in this study.

문장표절률: 43%

Table 3.1: Cutoff frequency for filter design Target response ω_1 (rad/sec) ω_2 (rad/sec) 75th floor acceleration 1.01 1.13 50th floor acceleration 8.17 8.80 30th floor acceleration 17.59 20.73 ure 3.4(a)) while the processing effect is trivial wh en the target response is 30th or 50th floor acceleration of which transfer functi on has zero away from the first modal one.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스템 설계

저자 : 박은천

발행 : 서울: 檀國大學校, 2006

5.4(a)) while the processing effect is trivial when the target response is 30th or 5 0th floor acceleration of which transfer function has zero away from the first m odal one.

문장표절률: 97%

Also, it is known from observing the error dis- tribution that the targeted respon ses are almost identical to wind-induced ones with a small magnitude of error w hile the other non-targeted responses are slightly different withincreasing error.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스템 설계

저자 : 박은천

발행 : 서울: 檀國大學校, 2006

Also, it is known from observing the error dis tribution that the targeted respons es are almost identical to wind-induced ones with small magnitude of error wh ile the other non-targeted responses are slightly different withincreasing error.

문장표절률: 100%

Targeting 30th or 50th-floor ac- celeration provides greater discrepancy betwe en the wind and LMS induced 75-floor accelerations which are critical in evalua ting service ability.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스템 설계

저자 : 박은천

발행 : 서울: 檀國大學校, 2006

Targeting 30th or 50th floor ac- celeration provides greater discrepancy between the wind and LMS induced 75-floor accelerations which are critical in evaluatin g service ability.

문장표절률: 54%

The com- parison between the results in Figure 3.7(a) and 3.7(b) indicates that t he distribution tendency of et and ef is quite different and the magnitude of et is much larger than that of ef .

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스템 설계

저자 : 박은천

발행 : 서울: 檀國大學校, 2006

The com parison between the results in Fig. 5.7(a) and (b) indicates that the 72 5. DESIGN OF AN ACTUATOR FOR SIMULATING WIND RESPONSE distribu tion tendency of et and ef is quite different and the magnitude of et is much larg er than that of ef .

문장표절률: 95%

When targeting response is 75th floor acceleration, both values of et and ef are smallest for the targeted 75th floor .BHOJUVEF .

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스템 설계

저자 : 박은천

발행 : 서울: 檀國大學校, 2006

When targeting response is 75th floor acceleration, both values of et and ef are smallest for the targeted 75th floor acceleration, but the value of ef becomes lar ger for the otherstory responses while the value of er generally keeps the smallest except for 30th story acceleration.

문장표절률: 39%

82 Chapter. 3. Design of Excitation System for Simulating Dynamic Loads accel eration, but the value of ef becomes larger for the otherstory responses while the value of ef generally keeps the smallest except for 30th story accel- eration.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스템 설계

저자 : 박은천

발행 : 서울: 檀國大學校, 2006

When targeting response is 75th floor accel eration, both values of et and ef are smallest for the targeted 75th floor accel eration, but the value of ef becomes lar ger for the otherstory responses while the value of er generally keeps the smallest except for 30th story accel eration.

문장표절률: 61%

The larger value of et results from the phase difference between the wind and L MS induced responses since et is obtained based on the response difference at th e same time step.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스템 설계

저자 : 박은천

발행 : 서울: 檀國大學校, 2006

The larger value of et results from the phase difference between wind and LMS i nduced responses since et is obtained based on the response difference at the sa me time step.

문장표절률: 41%

The phase of the wind-induced response is not the important parameter in evalu ating the wind resistance performance of a buildingstructure, ef can be said to b e the more appropriate index for evaluating the wind response reproducing the p erformance of LMS.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스템 설계

저자 : 박은천

발행 : 서울: 檀國大學校, 2006

The phase of the wind induced response is not important parameter in evaluating the wind resistance performance of a buildingstructure, ef can be said to be mor e appropriate index for evaluating the wind response reproducing performance o f LMS.

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저자 : 박은천

발행 : 서울: 단국대학교 대학원, 2007.2

The phase of the wind induced response is not important parameter in evaluating the wind resistance performance of a buildingstructure, ef can be said to be mor e appropriate index for evaluating the wind response reproducing performance o f LMS.

문장표절률: 24%

U U I U I P P P P P S P S S U U B U B S B S H S H F U V O Q S P D F T T F E H F F U U
V V O Q S P D F T T F E U U I I P P S U B S H F U Q O S P Q D S F P T D F T T F E I P P P P S U B
B S T F E U S H H F F U U Q Q S S P P D D F F T T T T F F E E (a) et (b) ef Figure 3.7: Error
distribution according to the filter usage and floor of target response.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시
스템 설계 = Design of excitation systems for simulating dynamic loads and re
al-time hybrid test method of building structures

저자 : 박은천
발행 : 서울 : 단국대학교 대학원, 2007.2

75 70 65 60 55 50 30 i l e Figure 5-7 Error distribution according to the filter u
sage and floor of target response 73 5. DESIGN OF AN ACTUATOR FOR SI
MULATING WIND RESPONSE Figs.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시
스템 설계

저자 : 박은천
발행 : 서울 : 檀國大學校, 2006

Frequency response of 75th floor with LMD forcetargeting DISPLACEMENT
RESPONSE.....71 Figure 5-7 Error distribution accordi
ng to the filter usage and floor of target RESPONSE.....
.....73 Figure 5-8.

문장표절률: 71%

Figures 3.8 and 3.9 compare the time histories of the wind and LMS induced str
uctural responses for the cases that the target responses are, respectively, 75th a
nd 30th-floor accelerations and filter are applied for the design of LMS.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시
스템 설계

저자 : 박은천
발행 : 서울 : 檀國大學校, 2006

5.8 and 5.9 compare the time histories of the wind and LMS induced structural r
esponses for the cases that the target responses are, respectively, 75th and 30th
floor accelerations and filter is applied for the design of LMS.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시
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저자 : 박은천
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5.8 and 5.9 compare the time histories of the wind and LMS induced structural r
esponses for the cases that the target responses are, respectively, 75th and 30th
floor accelerations and filter is applied for the design of LMS.

문장표절률: 28%

Figure 3.8 shows that the LMS induced acceleration response 83 Chapter. 3. De
sign of Excitation System for Simulating Dynamic Loads including targeted 75th
-floor acceleration agree well with wind-induced ones while displacement respo
nses at all floors are underestimated in Figure 3.9 that the shaker simulated tar
geted 30th-floor ac- celeration as well as displacement responses but overestim
ates 75th-floor acceleration.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시
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Fig. 5.8 shows that the LMS induced acceleration response including targeted 75
th floor acceleration agree well with wind-induced ones while displacement resp
onses at all floors are underestimated by LMS- II is observed in Fig.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시
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발행 : 서울 : 檀國大學校, 2006

Fig. 5.8 shows that the LMS induced acceleration response including targeted 75
th floor acceleration agree well with wind-induced ones while displacement resp
onses at all floors are underestimated by LMS.

문장표절률: 13%

DFMFSBUJPO N T QMBDFNFOU N 5J N F T F D PO E 5 JN F TF DPO E (c)
a) 76th story acceleration response (b) 76th story displacement response %JTQ
MB QMBDFNFOU N 5JN F T F DPO E 5 JN F T F DPO E (c) 50th story ac
celeration response (d) 50th story displacement response %JTQMB X -JO4E JJ
OOEEVDDFFEE QMBDFNFOU N XFYJODJEU FJOS EJOVEDVFDEF 5 J
N F T F DPO E 5 JN F T F DPO E (e) 30th story acceleration response (f) 30t
h story displacement response Figure 3.8: Wind and LMS induced acceleration r
esponses (when the target is 75th floor acceleration).

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시
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저자 : 박은천
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Wind and LMS induced acceleration responses (when the target is 75th floor ac
celeration).....74 Figure 5-9. Wind and LMS induced acc
eleration responses (when the target is 30th floor acceleration).....
.....75 Figure 5-10.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시
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Error distribution according to the filter usage and floor of target RESPONSE....
.....th.....73 Figure 5-8 Wind and LMS induced acceler
ation responses (when the target is 75th floor acceleration).....
.....74 Figure 5-9.

문장표절률: 95%

3.1.4 ATMD excitation In the 76th story building benchmark problem, ATMD is
used as an example controller, which is composed of spring and viscous dampe
r in addition to the mass and actuator of the LMS.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시
스템 설계

저자 : 박은천
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Wind and LMS induced acceleration responses (when the target is 30th floor ac
celeration) 5.4.4 ATMD excitation In the 76th story building benchmark problem
, ATMD is used as an example controller, which is composed of spring and visc
ous damper in addition to the mass and actuator of the LMS.

문장표절률: 91%

Figure 3.11 shows the time history comparison between wind induced acceleration and ATMD induced one targeting on 75th story acceleration response.

문장표절률: 100%

All acceleration responses coincide well with each other, but in displacement response, there exists slight underestimation and overestimation according to the time ranges.

문장표절률: 93%

Figure 3.12 shows the comparison between the frequency responses of 75th floor acceleration and it is observed that wind and ATMD induced responses show good agreement overall frequency range.

문장표절률: 94%

Figure 3.13 shows the error distribution according to the floor of targeted acceleration response. From Figure 3.13(b) showing that ef has the smallest value over floors when the target response is 75th floor acceleration, and the corresponding value ranges only between 1% and 10%, ATMD targeting 75th floor acceleration can be said to provide the best performance, and it can exactly reproduce the wind-induced acceleration response of all floors including targeted 75th floor.

문장표절률: 58%

3.1.5 Comparison between LMS and ATMD Figure 3.14 shows time history of the actuator forces in LMS and ATMD excitation systems.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

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발행 : 서울 : 단국대학교 대학원, 2007.2

Wind and LMS induced acceleration responses (when the target is 30th floor acceleration) 5-4.4 ATMD excitation In the 76th story building benchmark problem, ATMD is used as an example controller, which is composed of spring and viscous damper in addition to the mass and actuator of the LMS.

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발행 : 서울 : 단국대학교 대학원, 2007.2

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저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

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저자 : 박은천

발행 : 서울 :檀國大學校, 2006

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저자 : 박은천

발행 : 서울 :檀國大學校, 2006

Error Distribution with ATMD excitation 5.4.5 Comparison between LMS and ATMD Fig. 5.14 shows time history of the actuator forces in LMS and ATMD ex

문장표절률: 50%

The peak actuator force required for ATMD is larger than that for LMS. Figure 3.15 compares the stroke of LMS with/without filter and ATMD.

문장표절률: 100%

The stroke of LMS with filter is much smaller than those of LMS without filter and ATMD. This fact implies that ATMD requires largestroke to show good performance in wind-induced response realization and this stroke requirements should be checked in the design of ATMD.

인용포함 문장

문장표절률: 13%

86 Chapter. 3. Design of Excitation System for Simulating Dynamic Loads (a) 76th story acceleration response (b) 76th story displacement response (c) 50th story acceleration response (d) 50th story displacement response (e) 30th story acceleration response (f) 30th story displacement response Figure 3.11 : Wind and ATMD induced acceleration responses (when the target is 75th floor acceleration).

문장표절률: 70%

Table 3.2 shows the numerical values of the error, actuator force, and actuator stroke in LMS with/without the filter, and ATMD systems.

citation systems.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

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저자 : 박은천

발행 : 서울 :檀國大學校, 2006

The peak actuator force required for ATMD is larger than that for LMS. Fig. 5.15 compares the stroke of LMS with/without filter and ATMD.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

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This fact implies that ATMD requires largestroke in order to show good performance in wind-induced response realization and this stroke requirements should be checked in the design of ATMD.

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저자 : 박은천

발행 : 서울 :檀國大學校, 2006

ATMD Excitation.....76 Figure 5-11 Wind and ATMD induced acceleration responses (when the target is 75th floor acceleration).....77 Figure 5-12.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

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Wind and ATMD induced acceleration responses (when the target is 75th FLOOR ACCELERATION).....77 Figure 5-12. Frequency response of wind and ATMD induced 75th floor ACCELERATIONS.....78 Figure 5-13.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울 :檀國大學校, 2006

Comparison between actuator forces in LMS and ATMD systems Table 5.2 shows the numerical values of the error, actuator force, and actuator stroke in LMS with/without filter, and ATMD systems.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

Stroke Comparison of SOHs between LMS and ATMD systems Table 5.2 shows the numerical values of the error, actuator force, and actuator stroke in LMS with/without filter, and ATMD systems.

문장표절률: 100%

The errors are obtained when the target and evaluation responses are identical, and actuator force and stroke are obtained when the target response is 75th-floor acceleration.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스템 설계

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The errors are obtained when the target and evaluation responses are identical, and actuator force and stroke are obtained when the target response is 75th floor acceleration.

문장표절률: 56%

The facts observed from Figures 3.14 and 3.15 that the performance of LMS can be enhanced using the band-stop filter and ATMD reproduces wind-induced response better than LMS, but ATMD requires larger actuator force and stroke can be identified once more.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스템 설계

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The facts observed from Figs. 5.14 and 5.15 that the performance of LMS can be enhanced using band-stop filter and ATMD reproduces wind-induced response better than LMS but ATMD requires larger actuator force and stroke can be identified once more.

문장표절률: 80%

3.1.6 Summary The design of excitation systems for simulating wind-induced responses of a building structure was presented as a preliminary study for evaluating wind-resistance characteristics of building structures.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천
발행 : 서울: 단국대학교 대학원, 2007.2

Table 5-2. Comparison between LMS and ATMD LMS (unfiltered) LMS (filtered) ATMD 75th floor acceleration σ 0.117 0.192 0.074 σ 0.054 0.011 0.003 50th floor acceleration σ 0.081 0.081 0.083 σ 0.219 σ 0.213 0.525 30th floor acceleration σ 0.082 0.082 0.082 σ 0.034 0.033 0.065 Stroke Peak (m) 1.402 0.519 1.265 RMS (m) 0.436 0.183 0.333 Actuator force Peak (kN) 1068.22 441.68 655.78 RMS (kN) 323.92 136.44 170.39 5.5 Concluding Remarks Design of excitation systems for simulating wind induced responses of a building structure was presented as a preliminary study for evaluating wind-resistance characteristics of practical building structures.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스템 설계

저자 : 박은천
발행 : 서울: 檀國大學校, 2006

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문장표절률: 100%

The actuator forces of the LMS and ATMD were obtained using the inverse transfer function of structural responses. Also, band stop filter was used in LMS to remove zero of the transfer functions such that undesirable modal excitation is prevented and envelop function was used to reduce the error occurring in transient initial states in both LMS and ATMD.

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저자 : 박은천
발행 : 서울: 단국대학교 대학원, 2007.2

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저자 : 박은천
발행 : 서울: 檀國大學校, 2006

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저자 : 박은천

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저자 : 박은천

발행 : 서울 : 檀國大學校, 2006

The 81 5. DESIGN OF AN ACTUATOR FOR SIMULATING WIND RESPONSE numerical analyses results from a 76-story benchmark building confirmed that the structural responses of a buildingstnicture excited by wind loads acting at all floors could be reproduced by the proposed excitation systems installed at a specific floor.

문장표절률: 100%

Secondly, valuable data, which are available for the verification of structural seismic performance and the evaluation of the availability of vibration technique required in structural health monitoring, can be acquired from the test.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

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문장표절률: 72%

Thirdly, large shaking table is not required to evaluate the seismic response of real-scaled build-ing structures, since in this study the earthquake response is simulated by actuating a structure with an HMD installed at the upper story.

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발행 : 서울 : 단국대학교 대학원, 2007.2

Thirdly, large shaking table is not required to evaluate the seismic response of real-scaled build ing structures, since in this study the earthquake response is simulated by exciting a structure with a HMD installed at upper story.

문장표절률: 84%

3.2.2 Experimental Real-Scale Building Model The experimental model, which is shown in Figures 3.16 and 3.17, is a full-scale five-story steel structure which has the story height of 6m, the plan of 6m x 6m, and the story mass of 20,000kg.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천

발행 : 서울 : 단국대학교 대학원, 2007.2

6.2 Experimental Real-Scale Building Model The experimental model, which is shown in Figure 3, is a full-scale five-story steel structure which has the story height of 6m, the plan of 6m x 6m, and the story mass of 20,000kg* Each floor is composed of four identical wide-flange type steel columns.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자 : 박은천

발행 : 서울 : 檀國大學校, 2006

6.2 Experimental Real-Scale Building Model The experimental model, which is shown in Figure 3, is a fhll-scale five-story steel structure which has the story height of 6m, the plan of 6m x 6m, and the story mass of 20,000kg.

문장표절률: 41%

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

Each floor is composed of four identical wide-flange type steel columns. An **HMD using larger scale linear motor damper which was also designed as a passive damper has** a moving mass on the fifth floor excited the modelstructure.

문장표절률: 45%

Because the columns consist of I-shaped steel, the HMD was installed in minor axis of the structure. 3.2.3 Field measurement and experimental system Field measurement and experimental system is shown in Figures 3.18 and 3.19.

문장표절률: 94%

In order **to minimize the latency between the excitation and the measurement, one lap-top PC in the experimental system was used for simultaneously implementing the excitation and the measurement.**

문장표절률: 100%

The measurement system has accelerometers, PCB Corp. 393B12 model was **installed at the center of the 2nd 5th floor and KYOWA Corp.**

문장표절률: 22%

AS-2GB model was attached to the HMD. PCB Monitran and KYOWA DPM-711B was used 91 Chapter. 3. Design of Excitation System for Simulating Dynamic Loads 99 : : (*35) 9(- ! \$ 9 9 9 (300' \$ \$ \$ (- # (# 5) \$ \$ \$ (- (5) \$ \$ \$ (- (3% \$ \$ \$ \$ (- (/% \$ \$ \$ \$ (- 45 '\$ \$ ' 9 9 \$(*35) (- ! 9 9 9 (300' \$ \$ \$ (- (# 5) \$ \$ \$ (- (5) \$ \$ \$ (- (3% \$ \$ \$ (- (/% \$ \$ \$ (- 45 '\$ \$ ' 4 . * / 03 ' 9 & 4 4 . " + 03 ' 9 & 4 (a) minor axis (b) major axis Figure 3.16: Elevation view of the targetstructure.

문장표절률: 55%

to amplify their measured signals. The data cabling connection system has BNC cable with lengths of 25m and 50m. **In the data acquisitionsystem, the DAQ board of NI DAQCard-6036E with 16bit-range was used to perform the AD and**

스텝 설계

저자 : 박은천

발행 : 서울: 檀國大學校, 2006

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저자 : 박은천

발행 : 서울: 檀國大學校, 2006

Because the columns consist of I-shaped steel, the HMD was installed in minor axis of the structure. 6. FORCED VIBRATION TEST OF REAL-SCALE FIVE STORY STRUCTURE (5) C-150X75X25X3. : ^ 1500 3.L + 30100 MINOR AXES 100 MAJOR AXES 100 (a) minor axis (b) major axis Figure 6-1.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스텝 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

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발행 : 서울: 檀國大學校, 2006

6.3 and 6.4. **To minimize the latency between the excitation and the measurement, one lap-top PC in the experimental system was used for simultaneously implementing the excitation and the measurement.**

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저자 : 박은천

DA conversion.

문장표절률: 92%

Also, it was connected to the signal conditioner of NI SCC-2345 by using both the input modules and the output module. 92 Chapter.

문장표절률: 29%

%JTQMBDFNFOU NN &)B,SU%I JROVEBVLDF JEO E&VYDQFFES J N" OFBOMUZ TJT (e) 5th floor acceleration of Hachinohe (f) 1st floor displacement of Hachinohe earthquake excitation earthquake excitation Figure 3.29: Time history comparison of Hachinohe earthquake response in the analysis and experimental models.

문장표절률: 54%

(when the target response is the 5th-floor acceleration). frequency was shown, but it would not reflect the responses at the third mode or higher mode frequency in Figs. 3.30(a)–3.30(c).

문장표절률: 88%

3.2.7 Error Analysis The normalized RMS error between the absolute acceleration induced by HMD and the base acceleration could be express as Eq.

문장표절률: 89%

(3.23). Figure 3.31 is shown the floor distribute errors corresponding to the target responses of the analysis FE model. All responses are contained 10–20% error values and the minimal error value presented when the target response consider the 5th-floor acceleration.

발행: 서울: 단국대학교 대학원, 2007.2

In the data acquisition system, the DAQ board of NI DAQCard-6036E with 16 bit-range was used to perform the AD and DA conversion.

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저자: 박은천

발행: 서울: 단국대학교 대학원, 2007.2

Also, it was connected to the signal conditioner of NI SCC-2345 by using both the input modules and the output module. In this excitation system, both excitation and measurement signals are voltage signals and this excitation signal transfer equivalent thrust generated according to the input voltage signal through the inverter of HMD, to the mass of HMD.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자: 박은천

발행: 서울:檀國大學校, 2006

Time history comparison of Hachinohe earthquake response in the analysis and experimental models, (when the target response is the 5th floor acceleration) Fig.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

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저자: 박은천

발행: 서울: 단국대학교 대학원, 2007.2

6.15 compare the result in frequency domain of Hachinohe earthquake response in analysis model and experimental one. It is observed that the coincidence of the response at the first mode and second mode frequency was shown, but it would not reflect the responses at the third mode or higher mode frequency in Figs. 6.15 (a)–(c) 104 6.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

저자: 박은천

발행: 서울:檀國大學校, 2006

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저자: 박은천

발행: 서울:檀國大學校, 2006

FORCED VIBRATION TEST OF REAL-SCALE FIVE STORY STRUCTURE 6. 6.2 Error Analysis The normalized RMS error between the absolute acceleration induced by HMD and the base acceleration could be express as Eq.(6.13).

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저자: 박은천

발행: 서울: 단국대학교 대학원, 2007.2

FORCED VIBRATION TEST OF REAL-SCALE FIVE STORY STRUCTURE 6. 6.2 Error Analysis The normalized RMS error between the absolute acceleration induced by HMD and the base acceleration could be express as Eq.(6–13).

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발행: 서울: 단국대학교 대학원, 2007.2

Fig. 6.16 is shown the floor distribute errors corresponding to the target responses of the analysis FE model. All responses are contained 10–20% error values and the minimal error value presented when the target response consider the 5th floor acceleration.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

문장표절률: 36%

$\Sigma \sqrt{\dot{y}_g(t)^2 + \dot{y}_h(t)^2}$ Normalized RMS Error(%) = $\sqrt{\frac{1}{T} \int_0^T \dot{y}_g(t)^2 + \dot{y}_h(t)^2 dt}$ (3.23) where \dot{y}_g , \dot{y}_h are the structural acceleration responses which are the base acceleration and the HMD acceleration induced.

문장표절률: 55%

Then the MR damper can control the building structure with the generated current. Finally, the MR damper-based semiactive feedback control system, which has 12 inputs and two outputs, can be formed as shown in Figure 3.3.5.

문장표절률: 48%

The CO, as well as the cost function-based SNC algorithm, needs full state or the specific state of the structural system in order to calculate the appropriate desired control force.

문장표절률: 77%

The control implementation requires a lot of sensors, and that would be even impossible. Therefore the Kalman filters as an observer for state estimation is desirable.

문장표절률: 21%

Consider a controllable and observable system and assume that full state estimation is required. Then 111 Chapter. 3. Design of Excitation System for Simulating Dynamic Loads $t' \dots 1PXFS TQFDUSBM EFOTJUZ VFODZ$ [5JNF T' SF R (a) Scaled Northridge earthquake 1PXFS TQFDUSBM EFOTJUZ ' 5JNF T' S F RVFODZ) [(b) Kanai-Tajimi filtered artificial earthquake Figure 3.35: Acceleration of HMD and PSD used as external excitation load.

문장표절률: 23%

the main purpose of the general Kalman filter is constructing a state estimate $\hat{x}(t)$ that minimizes the steady-state error covariance which is defined by: $P = \lim_{t \rightarrow \infty} E[x - \hat{x}][x - \hat{x}]^T$ (3.24) The optimal estimator for the state of interest is the Kalman filter which can be expressed as: $\dot{\hat{x}} = (A - [L]C)\hat{x} + [L]y$ (3.25) where filter gain L is determined by solving an algebraic Riccati equation.

문장표절률: 71%

The Kalman filter uses the known input u and the measurements y to generate the output and state estimates, that is, \hat{y} and \hat{x} .

문장표절률: 57%

The lower part of the entire structure or whole building structure is modeled numerically. In order to verify the validity and accuracy of the proposed technique, a shaking table test was conducted.

스텝 설계

저자 : 박은천
발행 : 서울:檀國大學校, 2006

Fig. 6.16 is shown the floor distribute errors corresponding to the target responses of the analysis FE model. All responses are contained 10~200/o error values and the minimal error value presented when the target response consider the 5th floor acceleration.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스텝 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천
발행 : 서울:단국대학교 대학원, 2007.2

where y_h are the structural acceleration responses which are the base acceleration and the HMD acceleration induced. T is total excitation time of the earthquakes.

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Then the MR damper is able to control the building structure with the generated current. Finally the MR damper-based semiactive feedback control system, which has 12 inputs and 2 outputs, can be formed as Fig.

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5. The clipped-optimal control algorithm as well as the cost function-based semiactive neuro-control algorithm [5] needs full state or specific state of the structural system in order to calculate the appropriate desired control force.

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Actually the control implementation requires a large amount of sensors and that would be even impossible. Therefore the Kalman filters as an observer for state estimation is desirable.

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Consider a controllable and observable system and assume that full state estimation is required. Then the main purpose of the general Kalman filter is constructing a state estimate $\hat{x}^*(t)$ that minimizes the steady-state error covariance which is defined by: $P = \lim_{t \rightarrow \infty} E[(x - \hat{x}^*)(x - \hat{x}^*)^T]$.

[ijav.org] Unified Semiactive Control System Based on MR Damper for Cable

(2) 302 4 ICSV15 · 6-10 July 2008 · Daejeon · Korea Fig. 5. MR damper-based semiactive feedback control system The optimal estimator for the state of interest is the Kalman filter which can be expressed as $\dot{\hat{x}} = (A - [L]C)\hat{x} + [L]y$ (3.25) where filter gain L is determined by solving an algebraic Riccati equation.

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(3) $\dot{\hat{x}} = (A - [L]C)\hat{x} + [L]y$ where filter gain L is determined by solving an algebraic Riccati equation. The Kalman filter uses the known inputs u and the measurements y to generate the output and state estimates, that is, \hat{y} and \hat{x} .

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스텝 설계

저자 : 박은천
발행 : 서울:檀國大學校, 2006

In order to verify the validity and accuracy of the proposed technique, a shaking table test was conducted. The result of the study can be summarized as follows

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시 스텝 설계 = Design of excitation systems for simulating dynamic loads and real-time hybrid test method of building structures

저자 : 박은천
발행 : 서울:단국대학교 대학원, 2007.2

In order to verify the validity and accuracy of the proposed technique, a shaking

문장표절률: 75%

The result of the study can be summarized as follows. 1. To reduce the distortion of the interface acceleration, the inverse transfer function of the shaking table was identified, and its statespace realization was implemented in the shaking table controller.

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저자 : 박은천

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저자 : 박은천

발행 : 서울: 단국대학교 대학원, 2007.2

(1 To reduce the distortion of the interface acceleration, the inverse transfer function of the shaking table was identified and its statespace realization was implemented in the shaking table controller.

문장표절률: 66%

2. In this paper, the linear transfer function approach for controlling the motion of a shaking table was considered to verify the proposed method for a linear experimental part experimentally.

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문장표절률: 53%

However, this approach would be inappropriate in a coupled non-linear system leading to experimental instability. Therefore, in such case, the controller using the inverse transfer function of shaking table, shown in Figure 2.6, would be modified to compensate an experimental instability.

[Copykiller] 건축구조물의 동적하중 구현 및 실시간 하이브리드 실험을 위한 가진시스템 설계

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발행 : 서울: 단국대학교 대학원, 2007.2

However, this approach would be inappropriate for a coupled non-linear system leading to experimental instability. Therefore, in such case the controller using the inverse transfer function of shaking table would be modified to compensate an experimental instability.

문장표절률: 95%

3. The interface force between the experimental and numerical substructures was obtained using only acceleration measurement and mass information so that high-capacity loads cell and installation jigs are not required in the substructuring technique.

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저자 : 박은천

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문장표절률: 73%

4. The proposed method basing the interface force measurement on acceleration measurements from an experimental substructure is partially available only when the mass distribution is discrete - for example, this technique would be applicable

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저자 : 박은천

able to the TMD as an experimental part.

문장표절률: 100%

Also, the interface force measurement using force transducers is required to perform the proposed method when wind forces are applied to the experimental substructure.

문장표절률: 83%

147 Chapter. 5. Conclusions 6. An unexpected vibration of the experimental substructure can be induced by the feedback of responses including its inherent natural modes and then by the error occurred in calculating the numerical substructure.

문장표절률: 66%

7. It is considered that to minimize the effect of natural modes of an experimental substructure on the substructured system, the structural model as heavily-damped as possible would be used as an experimental part.

문장표절률: 73%

8. The proposed technique can be extended to the substructuring technique with the middle part of a whole structure in combination with the conventional substructuring technique employing lower part as the experimental substructure.

발행: 서울: 단국대학교 대학원, 2007.2

(4 The proposed method basing the interface force measurement on acceleration measurements from an experimental substructure is partially available only when the mass distribution is discrete - for example this would be applicable to the TMD as an experimental part.

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저자: 박은천

발행: 서울: 단국대학교 대학원, 2007.2

REAL-TIME SUBSTRUCTURING TECHNIQUE possible would be used as an experimental part. (8 The proposed technique can be extended to the real-time substructuring technique with the middle part of a whole structure in combination with the conventional substructuring technique employing lower part as the experimental substructure.

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문장표절률: 82%

9. The TLD installed on the top floor of the structure is physically tested, and simultaneously numerical calculation is carried out for the assumed analytical structural model.

문장표절률: 26%

10. Comparison between the structural responses obtained by the hybrid testing method and the conventional shaking table test of a single story steel frame with TLD and TLCD indicates that the performance of the TLD and TLCD can be accurately evaluated using the hybrid testing method without the physical structural model.

문장표절률: 88%

11. The uncontrolled and TLD-controlled structural responses of a three-story structure are obtained by the hybrid testing method in both time and frequency domains, showing that TLD can effectively mitigate the seismic responses of building structures and the hybrid testing method can reproduce the dynamic behavior of TLD-structure interaction systems for both the uncontrolled and controlled case.

문장표절률: 64%

Secondary, this paper presents the design of excitation systems for simulating dynamic loads. The controller design of an actuator is presented to simulate the responses of a building structure subjected to such dynamic excitations as earthquakes and wind loads.

저자 : 박은천
발행 : 서울: 檀國大學校, 2006

REAL-TIME SUB STRUCTURING TECHNIQUE possible would be used as an experimental part. (8 The proposed technique can be extended to the real-time substructuring technique with the middle part of a whole structure in combination with the conventional substructuring technique employing lower part as the experimental substructure.

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저자 : 박은천
발행 : 서울: 檀國大學校, 2006

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저자 : 박은천
발행 : 서울: 단국대학교 대학원, 2007.2

The TLCD installed at the top floor of the structure is physically tested, and simultaneously numerical calculation is carried out for the assumed analytical structural model.

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저자 : 박은천
발행 : 서울: 檀國大學校, 2006

Comparison between the structural responses obtained by the RHSTTM and the conventional shaking table test of a single story steel frame with TLD/TLCD indicates that the performance of the TLD/TLCD can be accurately evaluated using the RHSTTM without the physical structural model.

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저자 : 박은천
발행 : 서울: 단국대학교 대학원, 2007.2

Finally the uncontrolled and TLD-controlled structural responses of a three story structure are obtained by the RHSTTM in both time and frequency domains, showing that TLD can effectively mitigate the seismic responses of building structures and the RHSTTM can reproduce the dynamic behavior of TLD-structure interaction systems for both the uncontrolled and controlled case.

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The controller design of an actuator is presented to simulate the responses of a building structure subjected to such dynamic excitations as earthquakes and wind loads.

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The controller design of an actuator is presented to simulate the responses of a

문장표절률: 73%

The result of the study can be summarized as follows. 1. A design of a shaker for simulating wind-induced responses of a building structure was presented as a preliminary study for evaluating wind-resistance characteristics of practical building structures.

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109 7. Conclusions (5) A design of a shaker for simulating wind induced responses of a building structure was presented as a preliminary study for evaluating wind-resistance characteristics of practical building structures.

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109 □. Conclusions (5) A design of a shaker for simulating wind induced responses of a building structure was presented as a preliminary study for evaluating wind-resistance characteristics of practical building structures.

문장표절률: 76%

148 Chapter. 5. Conclusions 2. The force of the shaker was obtained using the inverse transfer function of structural responses.

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저자 : 박은천

발행 : 서울: 檀國大學校, 2006

(6) The force of the shaker was obtained using the inverse transfer function of structural responses. Also, band stop filter was used to remove zero of the transfer functions such that undesirable modal excitation is prevented, and envelop function was used to reduce the error occurring in transient initial states.

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발행 : 서울: 檀國大學校, 2006

(6) The force of the shaker was obtained using the inverse transfer function of structural responses. Also, band stop filter was used to remove zero of the transfer functions such that undesirable modal excitation is prevented, and envelop function was used to reduce the error occurring in transient initial states.

문장표절률: 98%

3. The numerical analyses results from a 76-story benchmark building confirmed that the structural responses of a building structure excited by wind loads acting at all floors could be reproduced by the proposed linear shaker installed at a specific floor.

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저자 : 박은천

발행 : 서울: 단국대학교 대학원, 2007.2

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ion of the target structural response for which acceleration response was suitable because targeting displacement response required large and high-speed changing control force.

문장표절률: 86%

5. In order to enhance practical applicability of the wind responsesimulating excitation systems, finite element model updating based on measured data, and the scaling or restriction of the excitation force or stroke limit for avoiding damage to the practical building etc.

문장표절률: 88%

should be considered 6. The field measurementsystem of full-scale structure and excitation system were established, and thenforced vibration test was carried out using the Hybrid Mass Damper designed to simulate seismic load.

문장표절률: 57%

7. System identification of full-scalestructure was carried out through white noise test, and the finite element modelis updated from measured data.

문장표절률: 95%

8. The seismic excitation system was accomplished through inverse transfer functions of structure and HMD by the system identifications.

저자 : 박은천
발행 : 서울: 檀國大學校, 2006

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