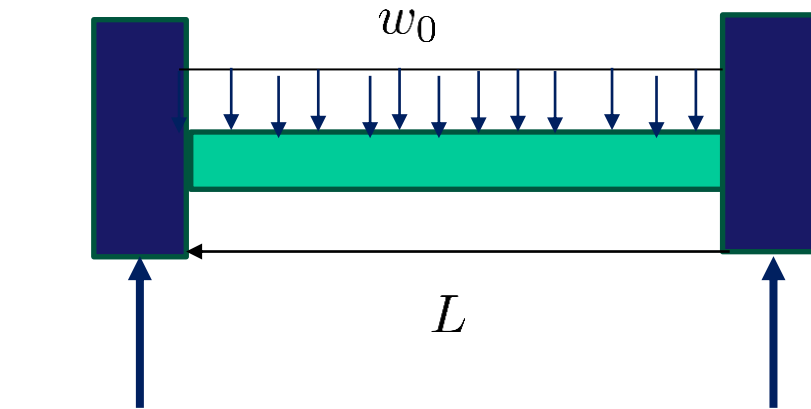


$$y(0) = 0, y'(0) = 0$$

$$y(L) = 0, y'(L) = 0$$



$$EIy^{(iv)} = -w_0$$

$$EIy''' = -w_0x + C_1$$

$$C_1 = \frac{w_0L}{2}$$

$$EIy'' = -\frac{w_0x^2}{2} + \frac{w_0Lx}{2} + C_2,$$

$$EIy' = -\frac{w_0x^3}{6} + \frac{w_0Lx^2}{4} + C_2x + C_3$$

$$y'(0) = 0 \rightarrow C_3 = 0$$

$$-\frac{w_0L^3}{6} + \frac{w_0L^3}{4} + C_2L = 0 \rightarrow C_2 = -\frac{w_0L^2}{12}$$

$$EIy(x) = -\frac{w_0x^4}{24} + \frac{w_0Lx^3}{12} - \frac{w_0L^2x}{12} + C_4$$

$$y(0) = 0 \rightarrow C_4 = 0$$

$$EIy^{(iv)}(x) = -w_0$$

$$EIy'''(x) = V(x) = -w_0x + \frac{w_0L}{2}$$

$$EIy''(x) = M(x) = -\frac{w_0x^2}{2} + \frac{w_0Lx}{2} - \frac{w_0L^2}{12}$$

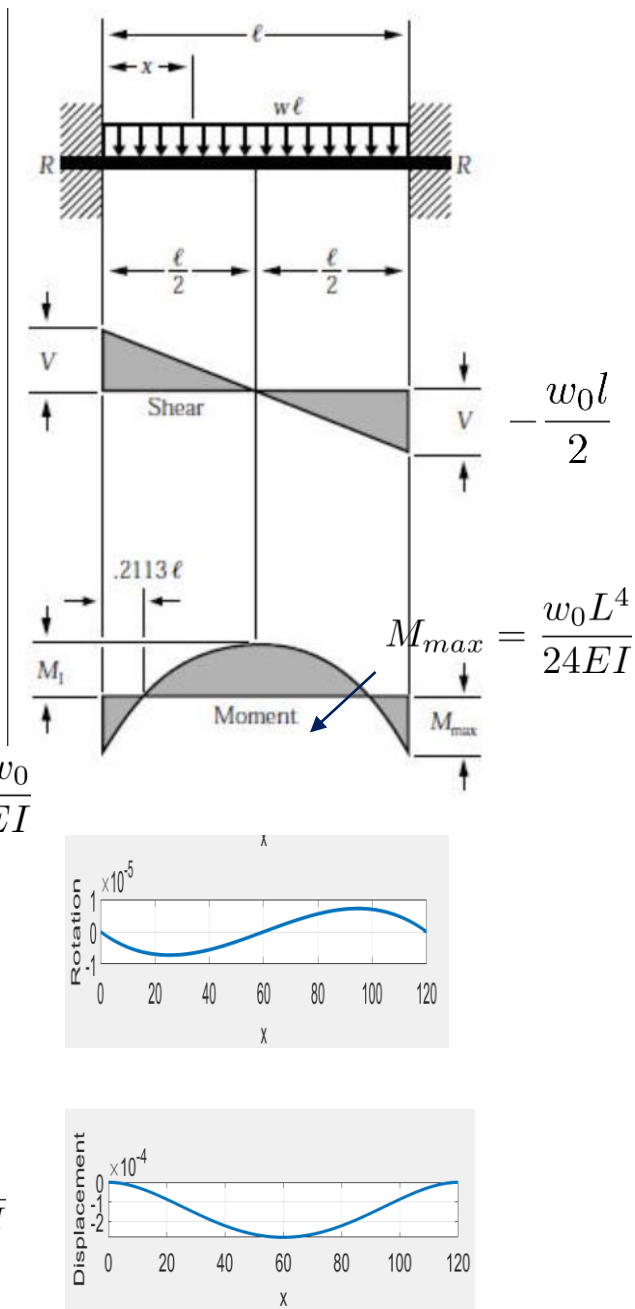
$$y'(x) = \theta(x) = -\frac{w_0}{24EI} \left(4x^3 - 6Lx^2 + 2L^2x \right)$$

$$y(x) = -\frac{w_0}{24EI} \left(x^4 - 2Lx^3 + L^2x^2 \right)$$

$$V(0) = \frac{w_0l}{2}$$

$$M(0) = -\frac{L^3w_0}{12EI}$$

$$y_{max} = -\frac{w_0l^4}{384EI}$$



Compare

$$EIy^{(iv)}(x) = -w_0$$

$$EIy'''(x) = V(x) = -w_0x + \frac{w_0L}{2}$$

$$V(0) = \frac{w_0l}{2}$$

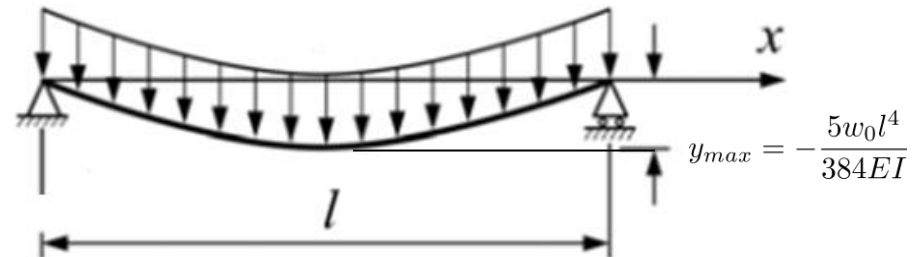
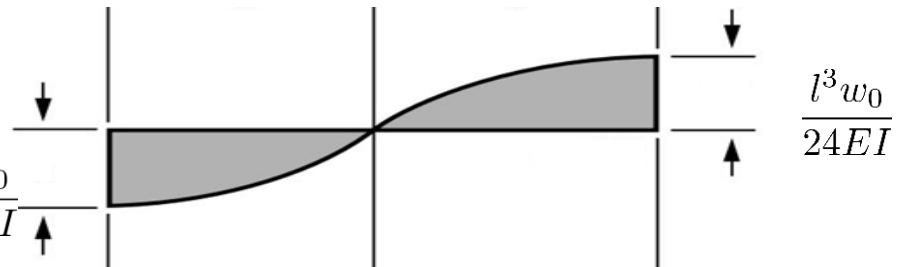
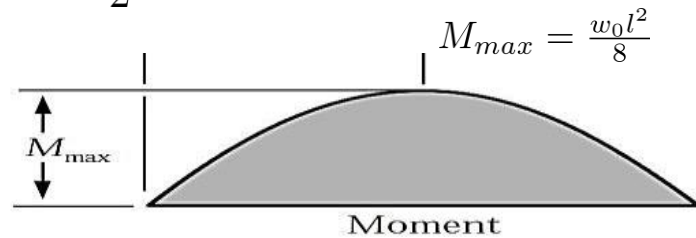
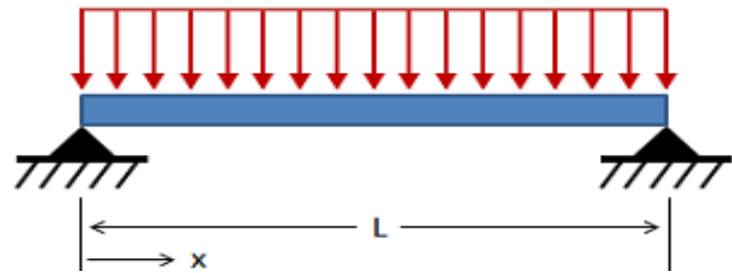
$$EIy''(x) = M(x) = -\frac{w_0x^2}{2} + \frac{w_0Lx}{2}$$

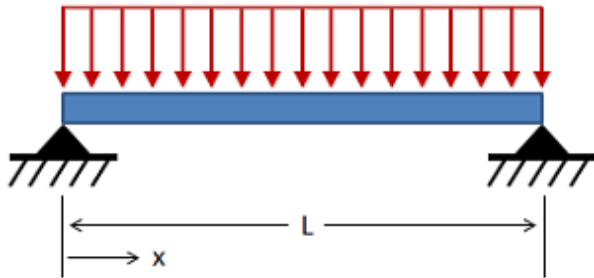
$$y'(x) = \theta(x) = -\frac{w_0}{24EI} (4x^3 - 6Lx^2 + L^3)$$

$$-\frac{l^3w_0}{24EI}$$

$$\frac{l^3w_0}{24EI}$$

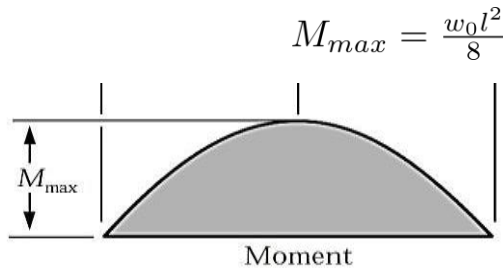
$$y(x) = -\frac{w_0}{24EI} [x^4 - 2Lx^3 + L^3x]$$



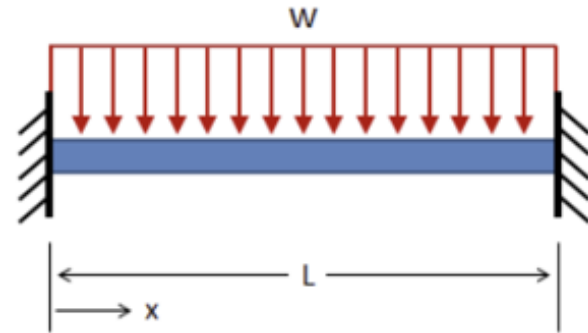


$$x=0, y(0) = 0;$$

$$x=L, M(L) = 0$$

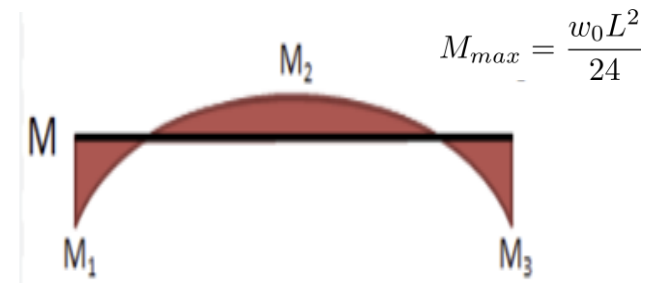


$$M(x) = -\frac{w_0 x^2}{2} + \frac{w_0 Lx}{2}$$



$$x=0, y(0) = 0;$$

$$x=0, \theta(0) = 0$$

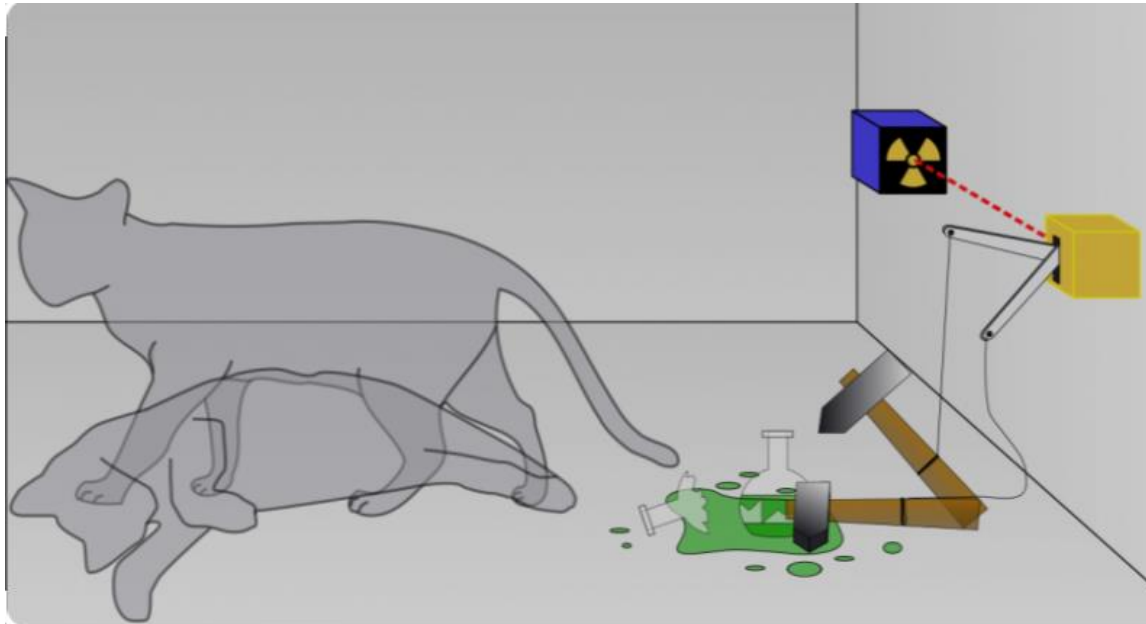


$$M(x) = -\frac{w_0 x^2}{2} + \frac{w_0 Lx}{2} - \frac{w_0 L^2}{12}$$

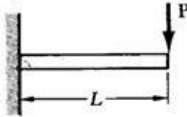
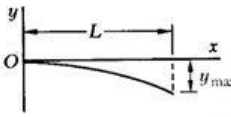
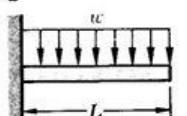
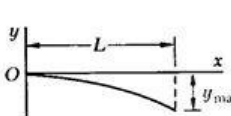
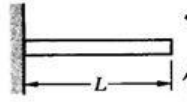
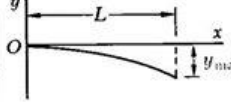
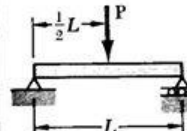
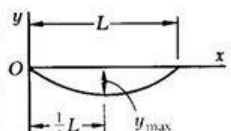
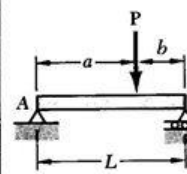
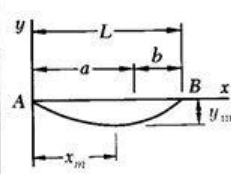
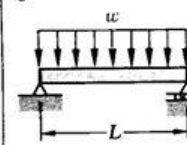
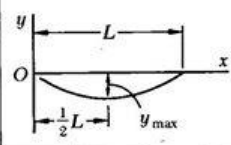
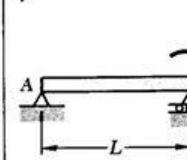
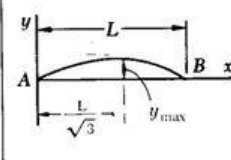
For the same external load, the statically indeterminate system has lower value of the maximum internal force. However,

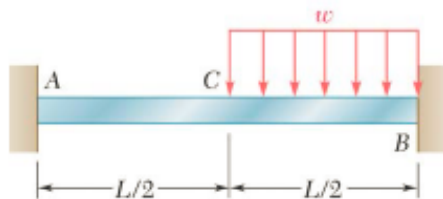
Lecture 32 Beam Deflection (III)

1. Superposition method;
2. Statically indeterminate problem;



Superposition of living and dead cats

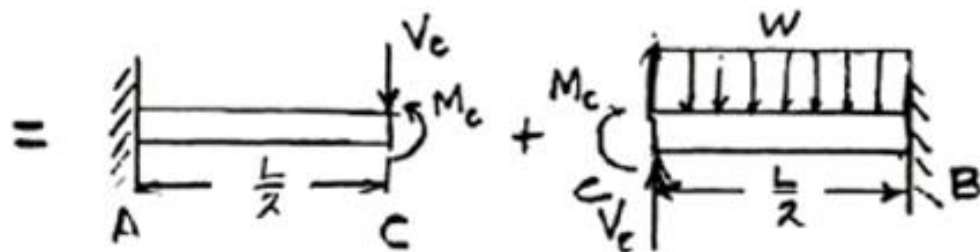
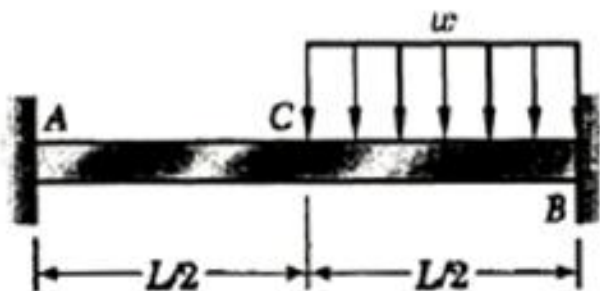
Beam and Loading	Elastic Curve	Maximum Deflection	Slope at End	Equation of Elastic Curve
1 		$-\frac{PL^3}{3EI}$	$-\frac{PL^2}{2EI}$	$y = \frac{P}{6EI}(x^3 - 3Lx^2)$
2 		$-\frac{wL^4}{8EI}$	$-\frac{wL^3}{6EI}$	$y = -\frac{w}{24EI}(x^4 - 4Lx^3 + 6L^2x^2)$
3 		$-\frac{ML^2}{2EI}$	$-\frac{ML}{EI}$	$y = -\frac{M}{2EI}x^2$
4 		$-\frac{PL^3}{48EI}$	$\pm \frac{PL^2}{16EI}$	For $x \leq \frac{1}{2}L$: $y = \frac{P}{48EI}(4x^3 - 3L^2x)$
5 		For $a > b$: $-\frac{Pb(L^2 - b^2)^{3/2}}{9\sqrt{3}EI}$ at $x_m = \sqrt{\frac{L^2 - b^2}{3}}$	$\theta_A = -\frac{Pb(L^2 - b^2)}{6EIL}$ $\theta_B = +\frac{Pa(L^2 - a^2)}{6EIL}$	For $x < a$: $y = \frac{Pb}{6EIL}[x^3 - (L^2 - b^2)x]$ For $x = a$: $y = -\frac{Pa^2b^2}{3EIL}$
6 		$-\frac{5wL^4}{384EI}$	$\pm \frac{wL^3}{24EI}$	$y = -\frac{w}{24EI}(x^4 - 2Lx^3 + L^3x)$
7 		$\frac{ML^2}{9\sqrt{3}EI}$	$\theta_A = +\frac{ML}{6EI}$ $\theta_B = -\frac{ML}{3EI}$	$y = -\frac{M}{6EIL}(x^3 - L^2x)$



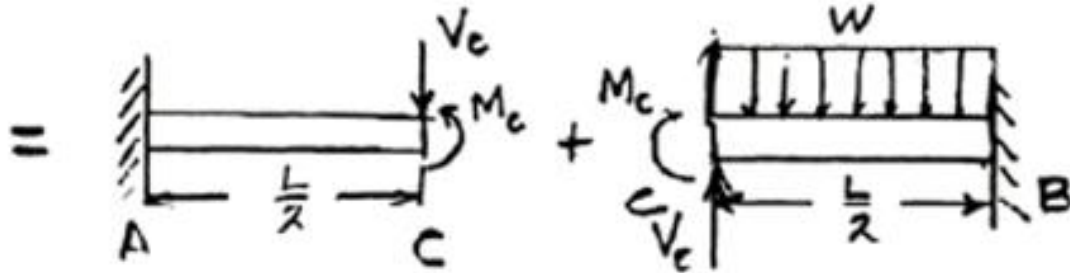
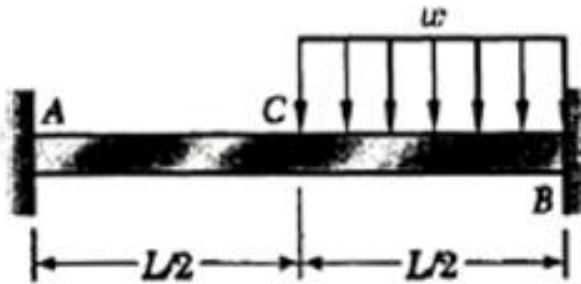
PROBLEM 15.43

For the beam shown, determine the reaction at B .

This problem is rated R!



	y_B	θ_B
1 	$-\frac{PL^3}{3EI}$	$-\frac{PL^2}{2EI}$
2 	$-\frac{wL^4}{8EI}$	$-\frac{wL^3}{6EI}$
3 	$-\frac{ML^2}{2EI}$	$-\frac{ML}{EI}$



	y_B	θ_B
1 	$-\frac{PL^3}{3EI}$	$-\frac{PL^2}{2EI}$
2 	$-\frac{wL^4}{8EI}$	$-\frac{wL^3}{6EI}$
3 	$-\frac{ML^2}{2EI}$	$-\frac{ML}{EI}$

$$y_c = -\frac{V_c(L/2)^3}{3EI} + \frac{M_C(L/2)^2}{2EI}$$

$$y_c = +\frac{V_c(L/2)^3}{3EI} + \frac{M_C(L/2)^2}{2EI} - \frac{w(L/2)^4}{8EI}$$

$$V_c = \frac{3wL}{32}$$

$$\theta_c = -\frac{V_c(L/2)^2}{2EI} + \frac{M_C(L/2)}{EI}$$

$$\theta_c = -\frac{V_c(L/2)^2}{2EI} - \frac{M_C(L/2)}{EI} + \frac{w(L/2)^3}{6EI}$$

$$M_c = \frac{wL}{48}$$

Today's Lecture Attendance Password is: Superposition

Problem 1.

This is a MATLAB homework problem. Consider an elastic beam with Young's modulus, $E = 30 \times 10^6 Psi$, the cross section moment inertia $I_z = 256 in^4$, and the length of the beam $L = 120 in$. The beam has a built-in boundary conditions at $x = 0$, i.e. $y(0) = 0$ and $\theta(0) = 0$; and at $x = L$, $y(L) = 0$ and $\theta(L) = 0$ as shown in Fig. 1.

The differential equation that governs the equilibrium of the bar has been derived as follows,

$$\frac{d^2}{dx^2} \left(EI \frac{d^2 y}{dx^2} \right) + w(x) = 0, \quad 0 < x < L,$$

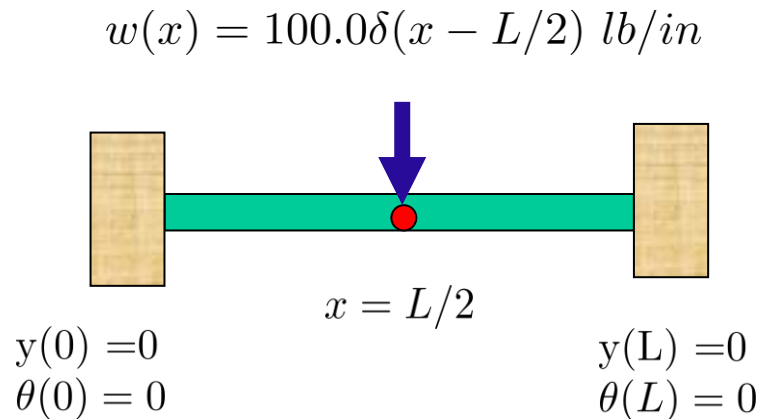
where $y(x)$ is the deflection field.

The beam is subjected a concentrated load at $x = L/2$, i.e.

$$w(x) = P\delta(x - L/2)$$

where $P = 100 lb$.

Modify the template MATLAB code, *beam_model.m*, to find shear diagram, moment diagram, rotation diagram, and the deflection profile.



Problem 1.



Matlab-code-
HW12.zip

Monday Monday SHAO... 4 KB



Matlab-P1.zip

Mar 5, Mar 5, SHAO... 75
2024 2024 KB



$E =$
 $20in.$
 $= L,$

WS,

where $y(x)$ is the deflection field.

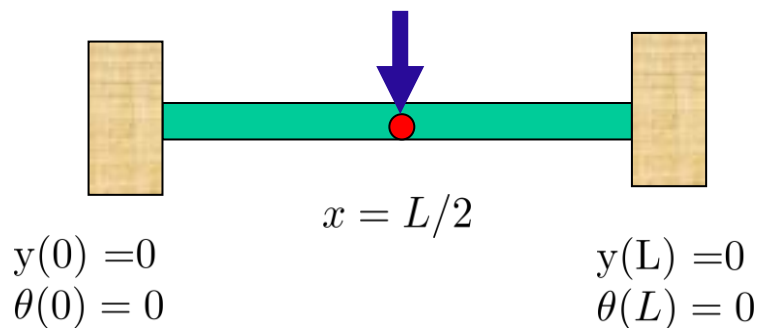
The beam is subjected a concentrated load at $x = L/2$, i.e.

$$w(x) = P\delta(x - L/2)$$

where $P = 100lb$.

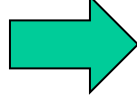
Modify the template MATLAB code, *beam_model.m*, to find shear diagram, moment diagram, rotation diagram, and the deflection profile.

$$w(x) = 100.0\delta(x - L/2) \text{ lb/in}$$



$\frac{d^2}{dx^2} \left(EI \frac{d^2 y}{dx^2} \right) = -w(x)$ can be decomposed into four first order ODE :

$$\frac{d\mathbf{y}}{dx} = \mathbf{f}(\mathbf{y}, x)$$



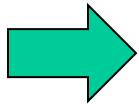
$$\theta = \frac{dy}{dx} \quad (1)$$

$$\kappa = \frac{d\theta}{dx} = y'' = \frac{M(x)}{EI} \quad (2)$$

$$\frac{dM}{dx} = V(x) \quad (3)$$

$$\frac{dV}{dx} = -w(x) \quad (4)$$

$$\begin{bmatrix} y_1 \\ y_2 \\ y_3 \\ y_4 \end{bmatrix}$$

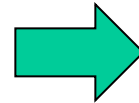


$$y(x)$$

$$\theta = \frac{dy}{dx}$$

$$M(x) = EI_z \frac{d^2 y}{dx^2}$$

$$V(x) = EI_z \frac{d^3 y}{dx^3}$$



$$\frac{d}{dx} \begin{bmatrix} y \\ \theta \\ M \\ V \end{bmatrix} = \begin{bmatrix} \theta \\ \frac{M}{EI_z(x)} \\ V \\ -w(x) \end{bmatrix}$$

one obtains the canonical form of the first-order vector ODE,

$$\frac{d}{dx}\mathbf{y} = \frac{d}{dx} \begin{bmatrix} y_1 \\ y_2 \\ y_3 \\ y_4 \end{bmatrix} = \begin{bmatrix} y_2 \\ \frac{y_3}{E(x)I_z(x)} \\ y_4 \\ -w(x) \end{bmatrix} \rightarrow \mathbf{f} = \begin{bmatrix} y_2 \\ \frac{y_3}{E(x)I_z(x)} \\ y_4 \\ -w(x) \end{bmatrix}$$



Matlab-code-
HW12.zip

Yesterday Yesterday SHAOF... 4 KB

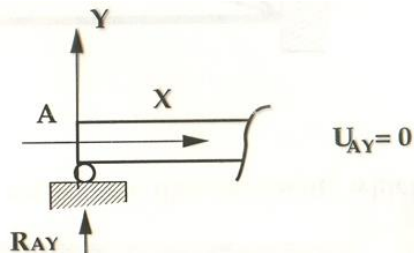


beam_T1

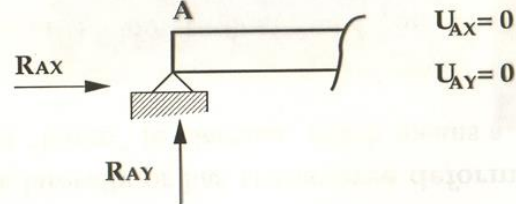


beam_T2

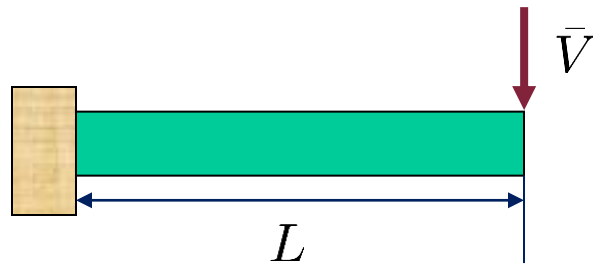
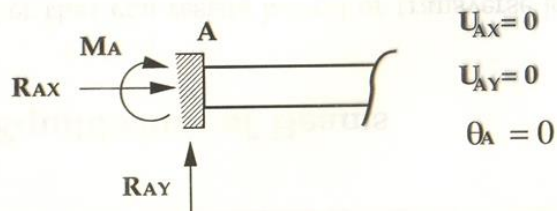
(1) Roller Support:



(2) Pinned Support:

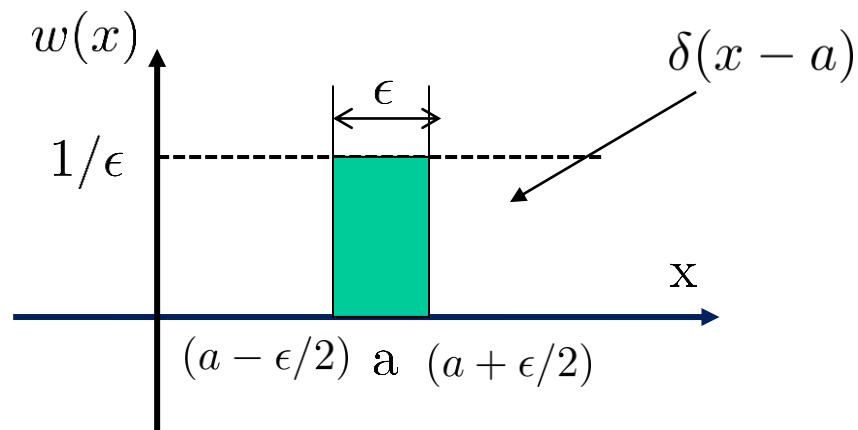


(3) Clamped Support:



$$\begin{bmatrix} v(0) - 0 \\ \theta(0) - 0 \\ M(L) - 0 \\ V(L) - \bar{V}_L \end{bmatrix} \Rightarrow g(y(0), y(L)) := \begin{bmatrix} g_1(y(0), y(L)) \\ g_2(y(0), y(L)) \\ g_3(y(0), y(L)) \\ g_4(y(0), y(L)) \end{bmatrix} = \begin{bmatrix} y_1(0) - 0 \\ y_2(0) - 0 \\ y_3(L) - 0 \\ y_4(L) - \bar{V}_L \end{bmatrix}$$

How to represent the Dirac delta function ?



The Dirac delta function can be defined as,

$$\delta(x - a) := \lim_{\epsilon \rightarrow 0} \begin{cases} \frac{1}{\epsilon}, & a - \epsilon/2 \leq x \leq a + \epsilon/2 \\ 0, & \text{otherwise} \end{cases}$$

```

66 function [fxy] = beam1d_ode(x,y)
67
68 % % -- Define material property and geometry
69 % w = 0;          % load, in lb/in
70 %
71 E = 30e6;        % Young's Modulus, in psi
72 I = 256;         % Second moment of inertia, in in^4
73 a = 120/2;       % Length of beam, in in
74
75 % Define point moment
76 epL = .1; %how small can epL be?
77 if (x<= a+epL/2 && x>=a-epL/2)
78     w = 1/epL;
79 else
80     w = 0;
81 end
82
83 % -- Define differential function here
84 fxy = [ y(2)
85         y(3)/(E*I)
86         y(4)
87         -w];
88 end

```

```

%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
%
% C30/ME85 Matlab Solver for Beam
%
% HW12 Problem Template
%
% -- Function to solve Beam problem
% using MATLAB ODE solver BVP4C
%
%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%%
function [sol]=beam_JP

% -- Define geometry
% L = 1000; % Question 1 -- Length of the beam
% L = 36; % Question 2 -- length of the beam, in in
L = 120; % Question 3 -- length in in.

% -- Set solver parameters
nvar = 4; % -- Number of variables
np = 1001; % -- Initial Number of points on [0,L]
xp = linspace(0,L,np); % -- Initial Points at which to satisfy ODE

% -- Set initial solution for the solver
solinit = bvpinit(xp,zeros(1,nvar));

```


%%

```
function [res] = beam1d_bc(ya,yb)
```

```
% -- Boundary Conditions (BC)
```

```
%   u: displacement
```

```
%   f: force
```

```
% ua = 0;      % -- Fixed      at x=a
```

```
% ma = 0;      % -- Zero moment at x=a
```

```
% ub = 0;      % -- Fixed      at x=b
```

```
% mb = 0;      % -- Zero moment at x=b
```

```
% res= [ya(1)-ua; ya(3)-ma;
```

```
%       yb(1)-ub; yb(3)-mb];
```

```
% % -- Problem 2 cantilever beam
```

```
% ua = 0;      % -- Fixed at x=a
```

```
% ta = 0;      % -- No rotation at x=a
```

```
% mb = 0;      % -- Zero moment at x=b
```

```
% Vb = -10e3; % -- -10 kip shear load at x=b
```

```
% res= [ya(1)-ua; ya(2)-ta;
```

```
%       yb(3)-mb; yb(4)-Vb];
```

```
% -- Simply supported beam
```

```
ua = 0;      % -- Fixed at x=a
```

```
ma = 0;      % -- No rotation at x=a
```

```
ub = 0;      % -- Fixed at x=b
```

```
mb = 0;      % -- No rotation at x=b
```

```
res= [ya(1)-ua; ya(3)-ma;
```

```
      yb(1)-ub; yb(3)-mb];
```

```
end
```