

HW K problem 1, # 16.5, 16.11, 16.17, 16.29, 16.55

①

Problem 1.

As shown in Fig. 1, the Cauchy stress tensor of a plane stress state at a material point is given as

$$\begin{bmatrix} 4 & 4 \\ 4 & -2 \end{bmatrix} \text{ MPa}$$

1. Draw its Mohr's circle;
2. Find all the principal stresses and maximum and minimum shear stresses;
3. The corresponding θ_p and θ_s ;
4. Use the method of pole to draw the element in the physical space in the principal directions and in the maximum/minimum shear stress directions.

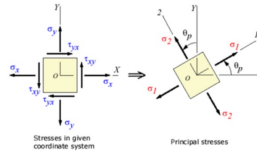
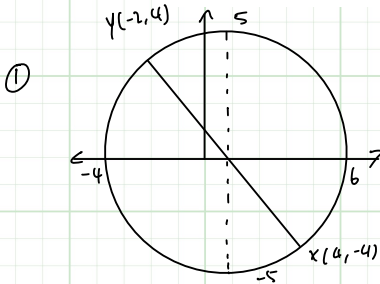


Figure 1: Initial orientation of the infinitesimal element and the element in the principal direction.



② $\sigma_{ave} = \frac{4-2}{2} = 1 \text{ MPa} = C$

$$R = \sqrt{\frac{(\sigma_x - \sigma_y)^2}{4} + \tau_{xy}^2} = \sqrt{\left(\frac{4-(-2)}{2}\right)^2 + 4^2} = 5$$

$\sigma_{max} = 1 + 5 = 6$

$\sigma_{min} = 1 - 5 = -4$

$\tau_{max} = R = 5$

$\tau_{min} = -R = -5$

Method of Pole

③ $\tan(2\theta_p) = \frac{2\tau_{xy}}{\sigma_x - \sigma_y} = \frac{2(4)}{4-(-2)} = \frac{4}{3}$

$\tan^{-1}\left(\frac{4}{3}\right) \rightarrow 2\theta_{pmax} = 53.13^\circ$

$2\theta_{pmin} = 53.13 + 180 = 233.13$

$\theta_{pmax} = \theta_{pmax} = \frac{53.13}{2} = 26.565^\circ$

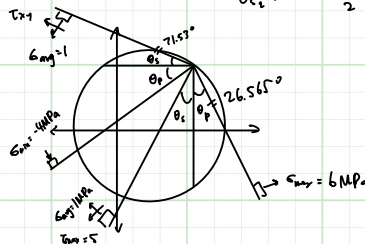
$\theta_{pmin} = \theta_{pmin} = \frac{233.13}{2} = 116.565^\circ$

$\tan(2\theta_s) = \frac{-\sigma_x - \sigma_y}{2\tau_{xy}} = \frac{4-(-2)}{2(4)} = -0.75$

$\tan^{-1}(-0.75) = 2\theta_s = 142.06^\circ$

$\theta_s = \frac{142.06}{2} = 71.53^\circ$

$\theta_{s1} = \frac{142.06 + 180}{2} = 161.53$



16.5

16.5

The rigid rod AB is attached to a hinge at A and to two springs, each of constant $k = 2 \text{ kips/in.}$, that can act in either tension or compression. Knowing that $h = 2 \text{ ft}$, determine the critical load.

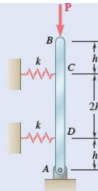


Fig. P16.5

$$\begin{aligned}x_0 &= h\theta \\x_c &= 3h\theta \\x_B &= 4h\theta\end{aligned}$$

$$F_c = kx_c = 3kh\theta$$

$$F_0 = kx_0 = kh\theta$$

$$\sum M_A = 0 \quad +\curvearrowright$$

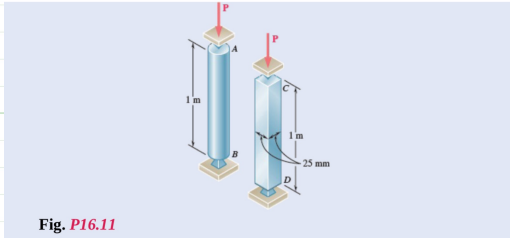
$$hF_0 + 3hF_c - Px_B = 0$$

$$kh^2\theta + 9kh^2\theta - 4hP = 0$$

$$P = \frac{5}{2}kh = \frac{5}{2}(2)(24) = 120 \text{ kips}$$

16.11

16.11 Determine the radius of the round strut so that the round and square struts have the same cross-sectional area, and compute the critical load for each. Use $E = 200 \text{ GPa}$.



$$A_{CD} = 25^2 = 625 \text{ mm}^2$$

$$A_{AB} = \pi r^2$$

$$r = \sqrt{\frac{625}{\pi}} = 14.104 \text{ mm}$$

CD:

$$I = \frac{25^4}{12} = 32.55 \times 10^{-9} \text{ m}^4$$

$$P_{cr} = \frac{\pi^2 EI}{(L)^2} = \frac{\pi^2 (200 \times 10^6) (32.55 \times 10^{-9})}{1^2}$$

$$P_{cr} = 64.3 \text{ kN}$$

AB:

$$I = \frac{\pi}{4} (14.104)^4 = 31.02 \times 10^{-9} \text{ m}^4$$

$$P_{cr} = \frac{\pi^2 (200 \times 10^6) (31.02 \times 10^{-9})}{1^2} = 61.24 \text{ kN}$$

16.17

16.17

Knowing that a factor of safety of 2.6 is required, determine the largest load P that can be applied to the structure shown. Use $E = 200$ GPa and consider only buckling in the plane of the structure.

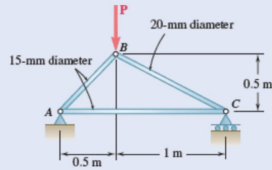


Fig. P16.17

$$L_{BC} : \sqrt{1^2 + 0.5^2} = 1.118 \text{ m}$$

$$L_{AB} : \sqrt{0.5^2 + 0.5^2} = 0.71 \text{ m}$$

$$I_{BC} : \frac{\pi}{64} (20)^4 = 7.854 \times 10^{-9} \text{ m}^4$$

$$I_{AB} : \frac{\pi}{64} (15)^4 = 2.485 \times 10^{-9} \text{ m}^4$$

$$P_{cr} = \frac{\pi^2 EI}{L^2}$$

$$P_{cr, AB} : \frac{\pi^2 (200 \times 10^9) (2.485 \times 10^{-9})}{0.71^2} = 9.812 \text{ kN}$$

$$P_{cr, BC} : \frac{\pi^2 (200 \times 10^9) (7.854 \times 10^{-9})}{1.118^2} = 12.40 \text{ kN}$$

$$F_{GH} : \frac{P_{cr}}{F_s}$$

$$F_{GH, AB} : \frac{9.812}{2.6} = 3.773 \text{ kN}$$

$$F_{GH, BC} : \frac{12.40}{2.6} = 4.77 \text{ kN}$$

$$\sum F_x = 0 : \frac{0.5}{0.71} F_{AB} - \frac{1}{1.118} F_{BC} = 0$$

$$F_{BC} = 0.7905 F_{AB}$$

$$\sum F_y = 0$$

$$\frac{0.5}{0.71} F_{AB} + \frac{0.5}{1.118} F_{BC} - P = 0$$

$$P = 1.061 F_{AB}$$

$$P : \frac{1.061 F_{BC}}{0.7905} = 1.342 F_{BC}$$

$$P = 1.061 (3.773) = 4.000 \text{ kN}$$

16.24

16.24

Each of the five struts shown consists of a solid steel rod. (a) Knowing that strut (1) is of a 0.8-in. diameter, determine the factor of safety with respect to buckling for the loading shown. (b) Determine the diameter of each of the other struts for which the factor of safety is the same as the factor of safety obtained in part a. Use

$$E = 29 \times 10^6 \text{ psi.}$$

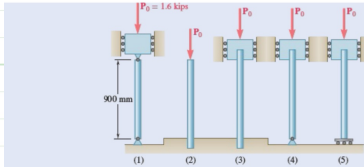


Fig. P16.24

$$\begin{aligned} a) \quad P_0 &= 1.6 \times 10^3 \text{ lb} & P_{cr} &= \frac{\pi^2 EI}{L^2} \\ E &= 29 \times 10^6 \frac{\text{lb}}{\text{in}^2} \\ L &= 900 \text{ mm} \end{aligned}$$

$$I = \frac{\pi}{64} (d_1^4) = \frac{\pi}{64} (0.8)^4 = 0.0201 \text{ in}^4$$

$$P_{cr} = \frac{\pi^2 (29 \times 10^6) (0.0201)}{36^2} = 4439.04 \text{ lb}$$

$$\begin{aligned} \text{FOS} &= \frac{P_{cr}}{P_0} = \frac{4439.04}{1600} = 2.78 \\ d_1 &= 0.800 \text{ in} \end{aligned}$$

$$\begin{aligned} d_4 &= 4 \sqrt{\frac{4439.04 (36)^2}{\pi^2 (29 \times 10^6) (0.0201) (2)}} \\ d_4 &= 0.669 \text{ in} \end{aligned}$$

$$b) \quad P_{cr} = \frac{\pi^2 E (0.049)^4}{4L^2}$$

$$d_2 = 4 \sqrt{\frac{4439.04 (36)^2}{\pi^2 (29 \times 10^6) (1.049)^2}} = 1.12 \text{ in}$$

$$d_3 = 4 \sqrt{\frac{4439.04 (36)^2}{\pi^2 (29 \times 10^6) (0.049)^4}} = 0.566 \text{ in}$$

$$\begin{aligned} d_5 &= 4 \sqrt{\frac{4439.04 (36)^2}{\pi^2 (29 \times 10^6) (0.049)^2}} \\ d_5 &= 0.800 \text{ in} \end{aligned}$$

16.55

16.55 (a) Considering only buckling in the plane of the structure shown and using Euler's formula, determine the value of θ between 0 and 90° for which the allowable magnitude of the load P is maximum. (b) Determine the corresponding maximum value of P knowing that a factor of safety of 3.2 is required. Use $E = 29 \times 10^6$ psi.

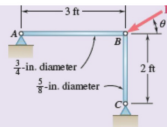


Fig. P16.55

$$a) P_{cr, AB} = \frac{\pi EI}{L_c^2}$$

$$I = \frac{\pi (\frac{3}{4})^4}{64} = 15.53 \times 10^{-3} \text{ in}^4$$

$$L_c = 3(12) = 36 \text{ in}$$

$$P_{cr, AB} = \frac{\pi^2 EI}{L^2} = \frac{\pi^2 (29 \times 10^6) (15.53 \times 10^{-3})}{36^2} = 3430.11 \text{ lb}$$

$$P_{cr, BC} = \frac{\pi^2 (\frac{5}{8})^4 (\frac{\pi}{64}) (29 \times 10^6)}{24^2} = 3721.90 \text{ lb}$$

$$\theta = \tan^{-1} \left(\frac{3721.90}{3430.11} \right) = 47.33^\circ$$

$$b) P_{cr} = \sqrt{3430.11^2 + 3721.90^2} = 5061.44 \text{ lb}$$

$$P = \frac{P_{cr}}{FS} = \frac{5061.44}{3.2} = 1581.7 \text{ lb}$$