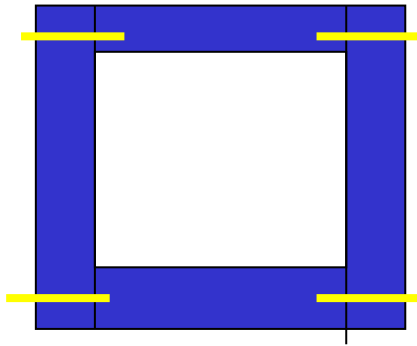
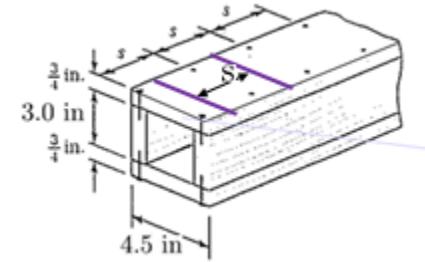
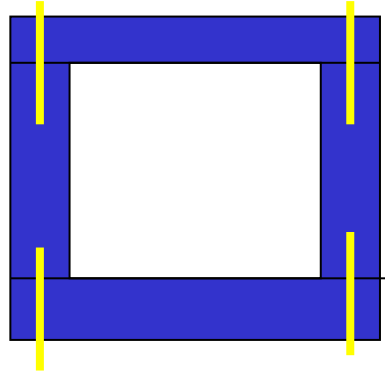


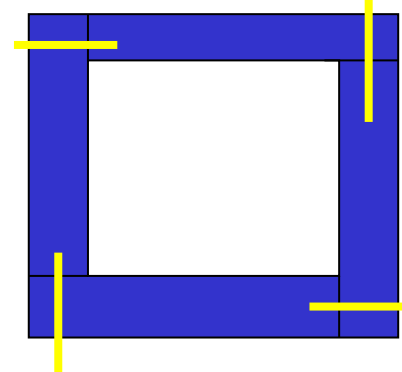
**We have three designs:
Which one is the best design?**



(a)

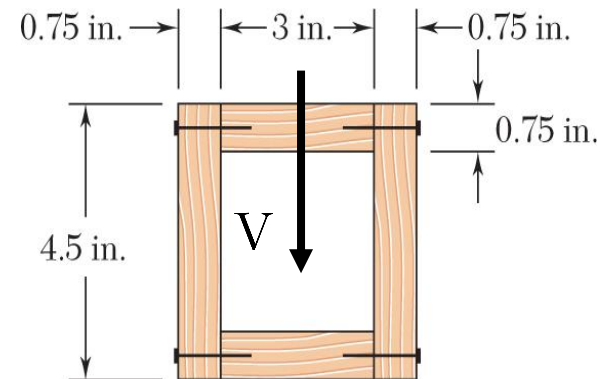


(b)



(c)

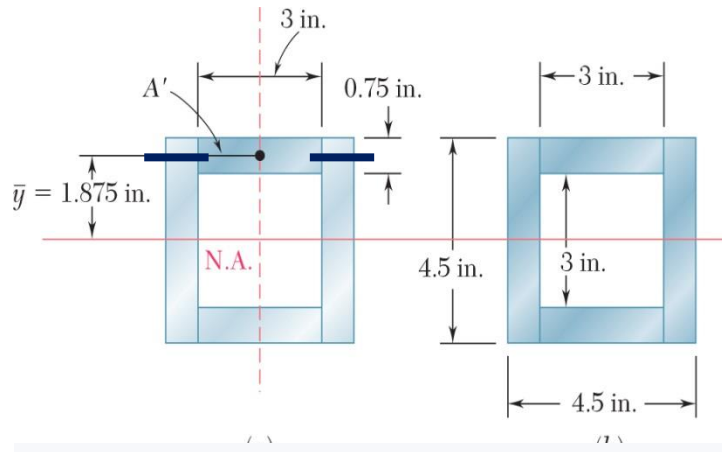
The square box beam is constructed from four planks as shown. Knowing that the spacing between nails is $s = 1.75$ in. and the beam is subjected to a vertical shear of magnitude $V = 600$ lb, **determine the shearing force in each nail for all three designs.**



This is a rated-R problem,

$$A_w = 4.5^2 - 3^2 = 11.25 \text{ in}^2$$

Design 1



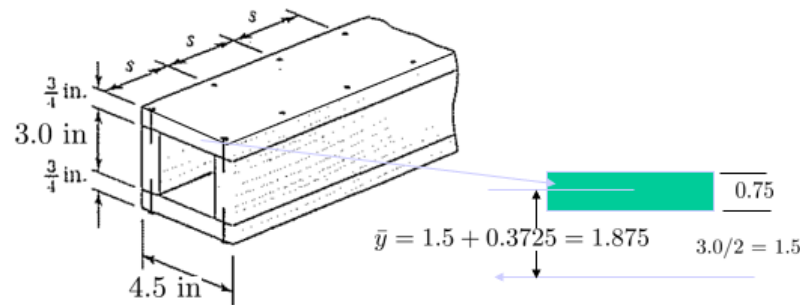
$$I = \frac{1}{12}(4.5\text{ in})^4 - \frac{1}{12}(3\text{ in})^4$$

$$= 27.42\text{ in}^4$$

$$V = 600\text{ N}$$

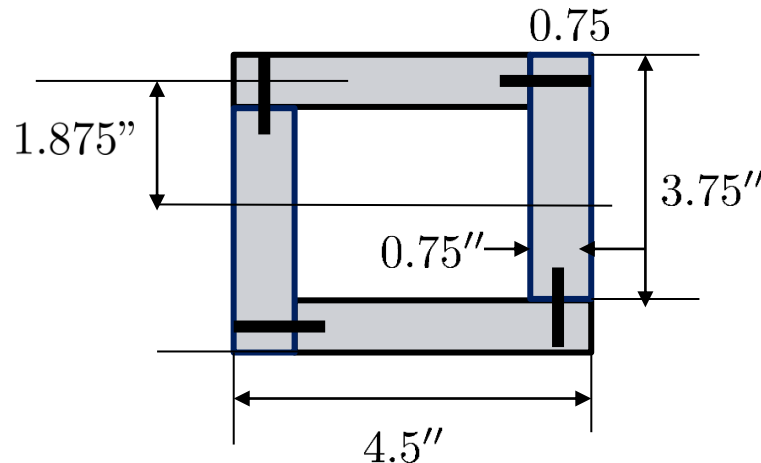
$$s = 1.75\text{ in}$$

Design 2

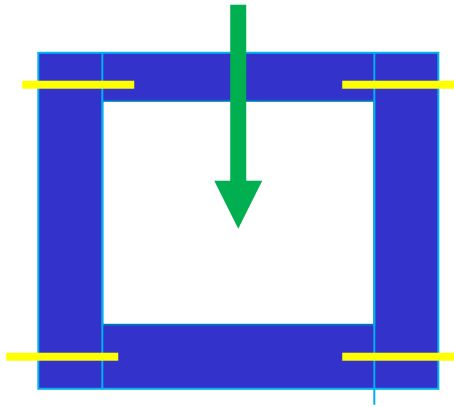


$$2F_{\text{nailed}} = \Delta H = q\Delta x$$

Design 3

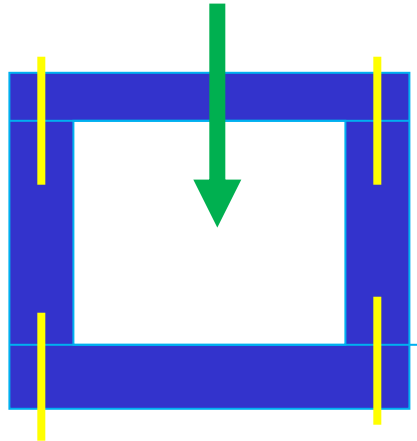


Summary



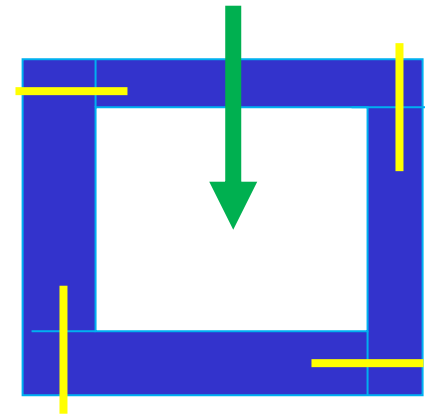
(a)

$$F_{nail} = 80.8 \text{ lb}$$



(b)

$$F_{nail} = 117.931 \text{ lb}$$



(c)

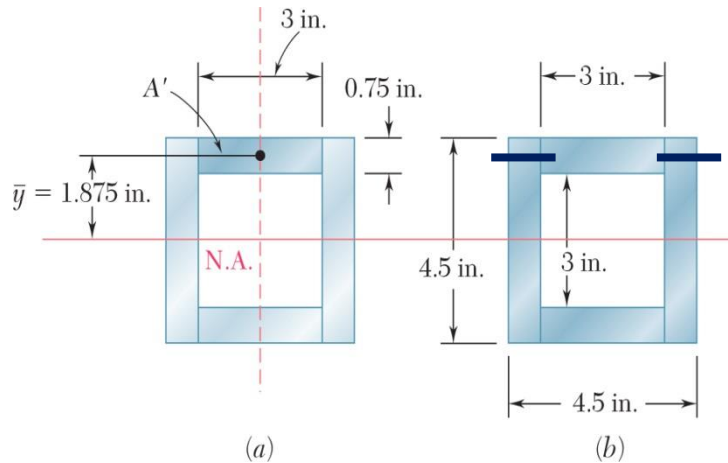
$$F_{nail} = 98.275 \text{ lb}$$

Which design is the optimal design ?

For the fixed F_{nail} , which design has the smallest $\Delta x = s$?

Design 1

$$\Delta H = q\Delta x$$



$$I = \frac{1}{12}(4.5\text{in})^4 - \frac{1}{12}(3\text{in})^4$$

$$= 27.42\text{in}^4$$

$$s = 1.75\text{in}$$

$$q = \frac{VQ}{I} = \frac{(600\text{lb})(4.22\text{in}^3)}{27.42\text{in}^4} = 92.3\frac{\text{lb}}{\text{in}}$$

$$f = \frac{q}{2} = 46.15\frac{\text{lb}}{\text{in}}$$

= edge force per unit length

For the upper plank,

$$Q = A'\bar{y} = (0.75\text{in.})(3\text{in.})(1.875\text{in.})$$

$$= 4.22\text{in}^3$$

Based on the spacing between nails,
determine the shear force in each
nail.

$$F = fs = \left(46.15\frac{\text{lb}}{\text{in}}\right)(1.75\text{in})$$

$$F = 80.8\text{lb}$$

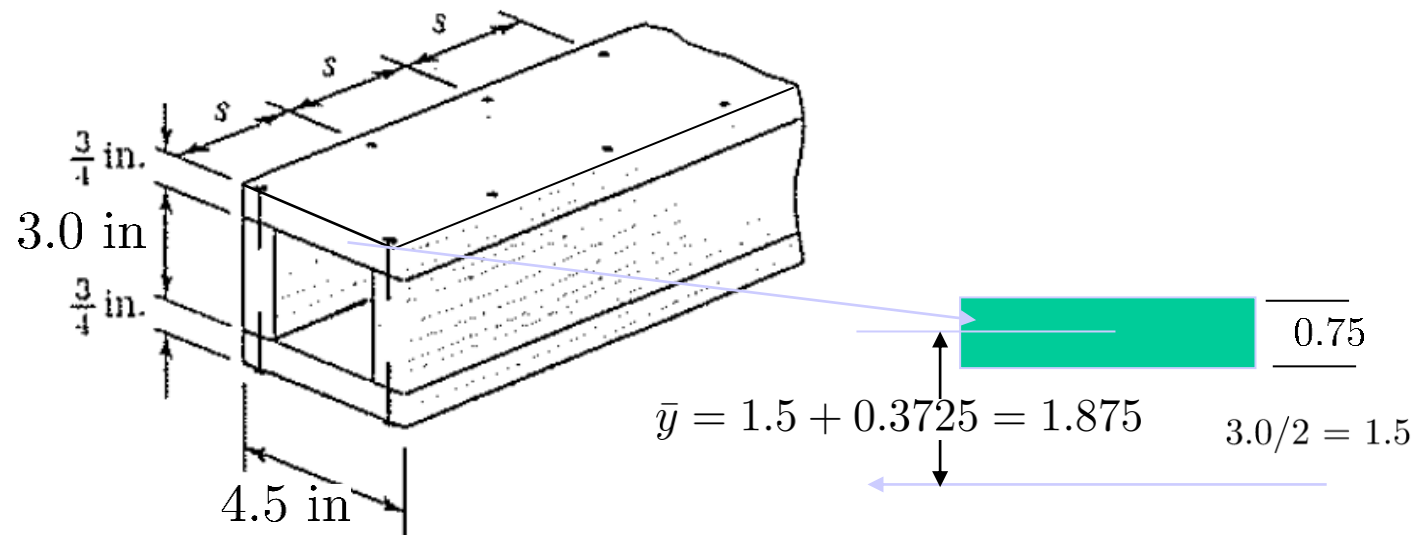
Design 2:

$$V = 600lb$$

$$I = 27.42in^4$$

$$A = 4.5 \times 0.75 = 3.375 in^2$$

$$\Delta x = s = 1.75 in$$



$$Q = \bar{y}A = (1.825)(3.375) = 6.1594in^3$$

$$2F_{nail} = qs$$

$$q = \frac{VQ}{I} = \frac{(600)(6.1594)}{27.42} = 134.778lb/in$$

$$F_{nail} = \frac{(134.778)(1.75)}{2} = 117.931 lb$$

Design 3:

$$V = 600lb$$

$$A = 3.75 \times 0.75 = 2.8125 \text{ in}^2$$

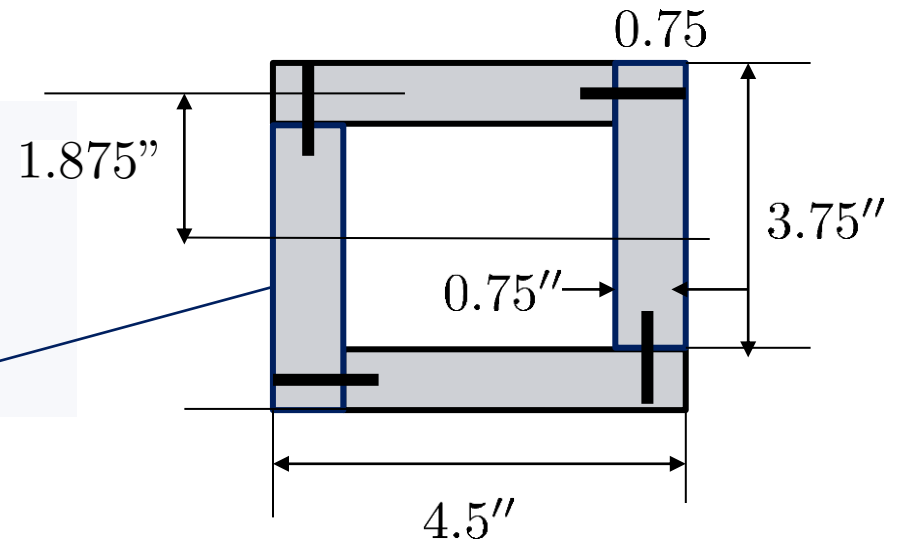
$$s = 1.75 \text{ in}$$

$$I = 27.42 \text{ in}^4, \bar{y} = 1.875$$

$$Q = \bar{y}A = (1.875)(2.815) = 5.1328 \text{ in}^3$$

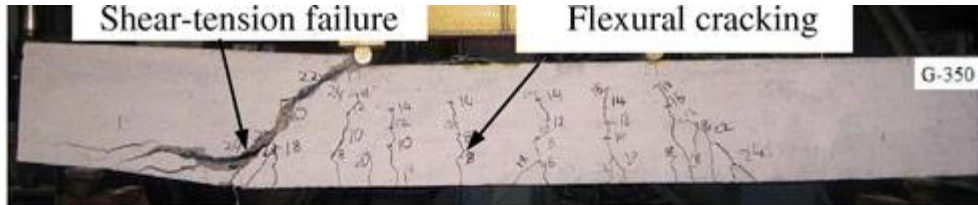
$$q = \frac{VQ}{I} = \frac{(600)(5.1328)}{27.42} = 112.315 \text{ lb/in}$$

$$F_{\text{nail}} = \frac{(112.315)(1.75)}{2} = 98.275 \text{ lb}$$

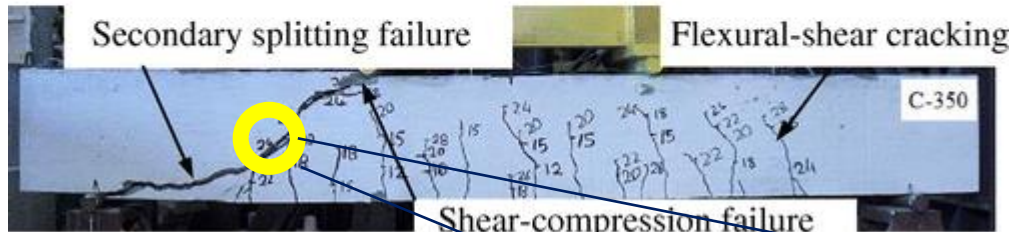


$$\Delta H = 2F_{\text{nail}} = qs$$

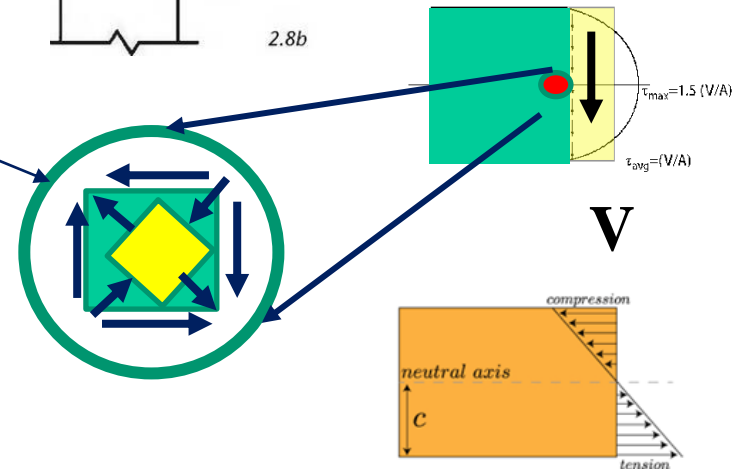
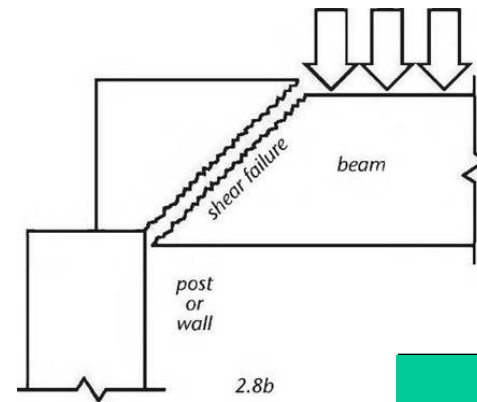
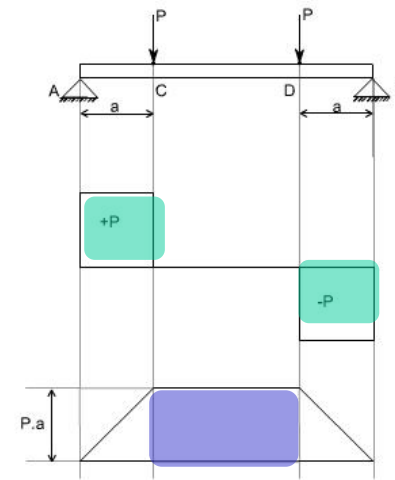
Steel Beam Shear Test



(a)



(b)



Lecture 30 Differential Equation for Beam Deflection



Objectives: Study the deformation (deflection) of beams

| | | |
|------|------------|---------------------------|
| Bar: | Elongation | $\Delta = \frac{PL}{EA};$ |
|------|------------|---------------------------|

| | | |
|-------|----------------|------------------------|
| Shaft | Angle of Twist | $\Phi = \frac{TL}{GJ}$ |
|-------|----------------|------------------------|

| | | |
|------|------------|---------|
| Beam | Deflection | $y = ?$ |
|------|------------|---------|



Leonhard Euler

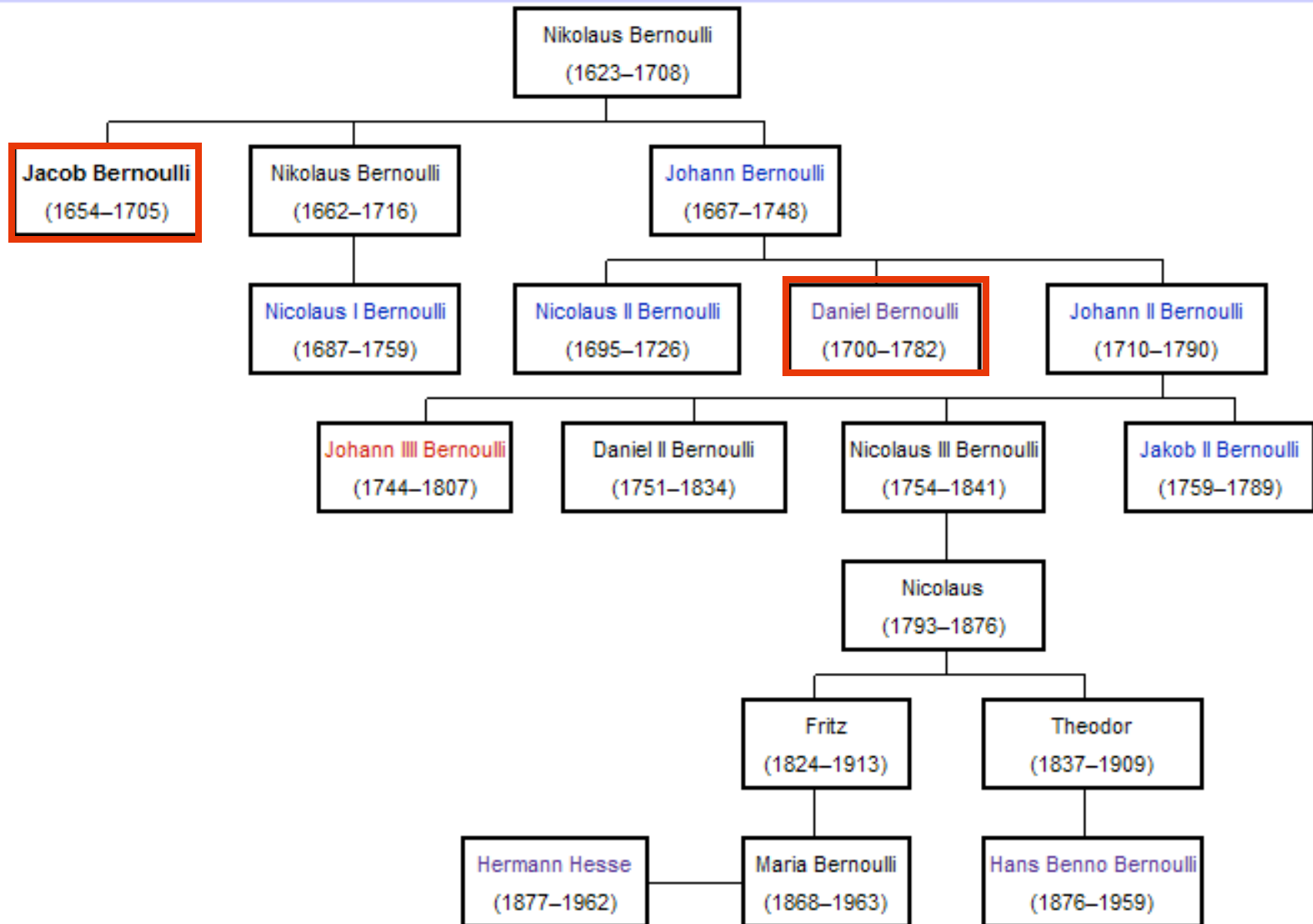


Jacob Bernoulli



Daniel Bernoulli

Bernoulli family tree



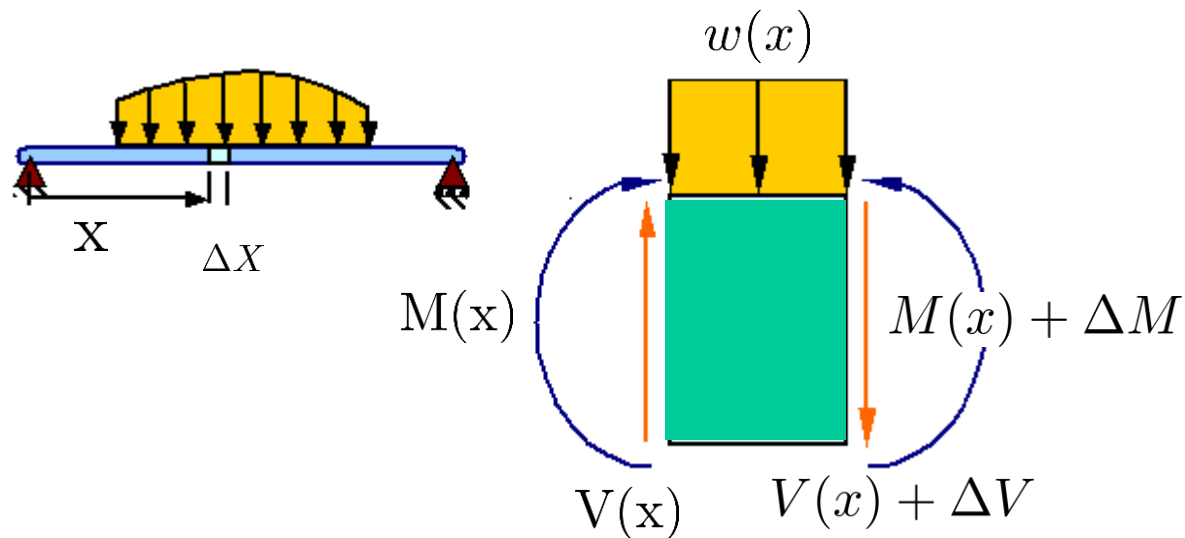
Family tree of the Bernoulli family

Recall: From equilibrium

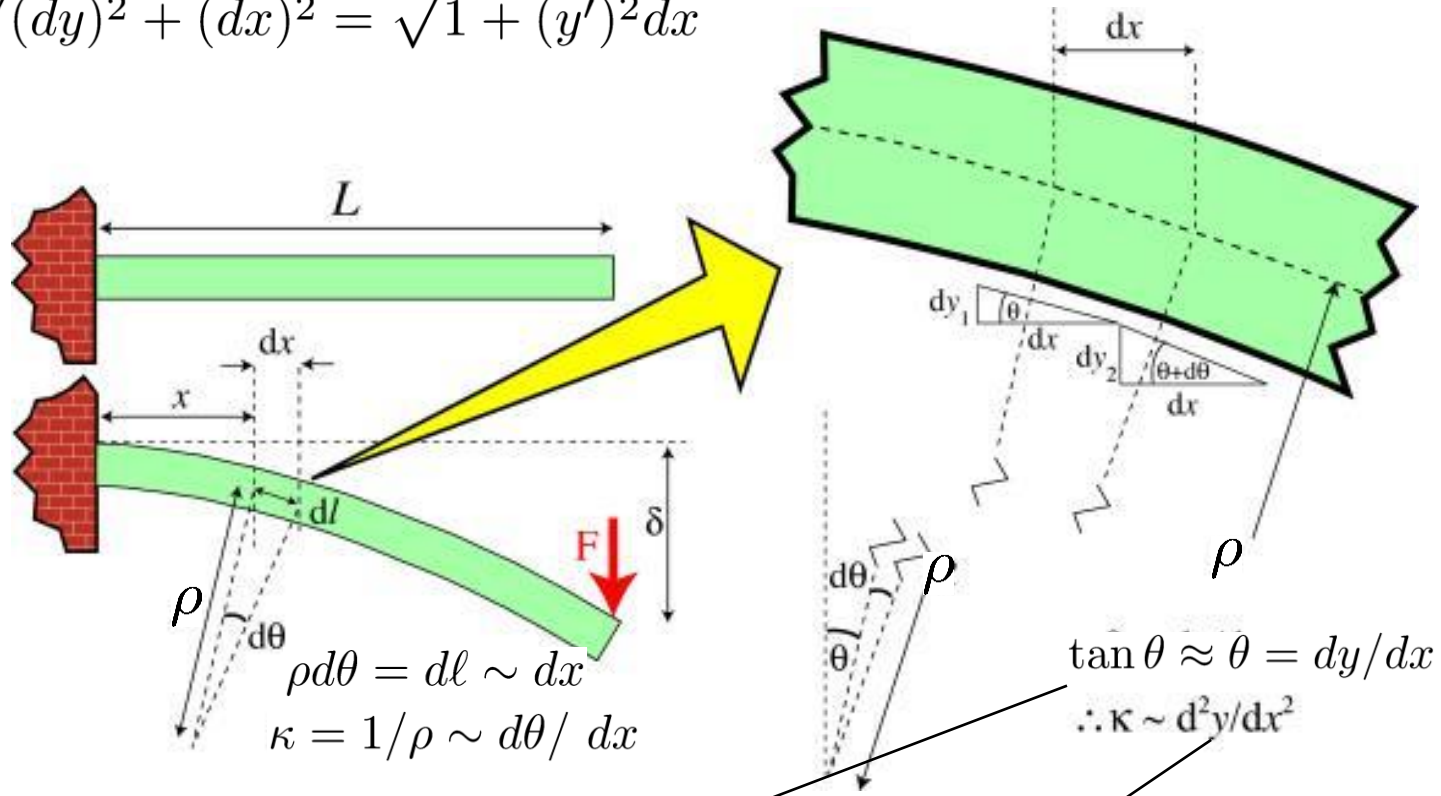
$$\frac{dV}{dx} = -w(x); \quad \frac{dM}{dx} = V(x);$$



$$\frac{d^2 M}{dx^2} = -w(x);$$

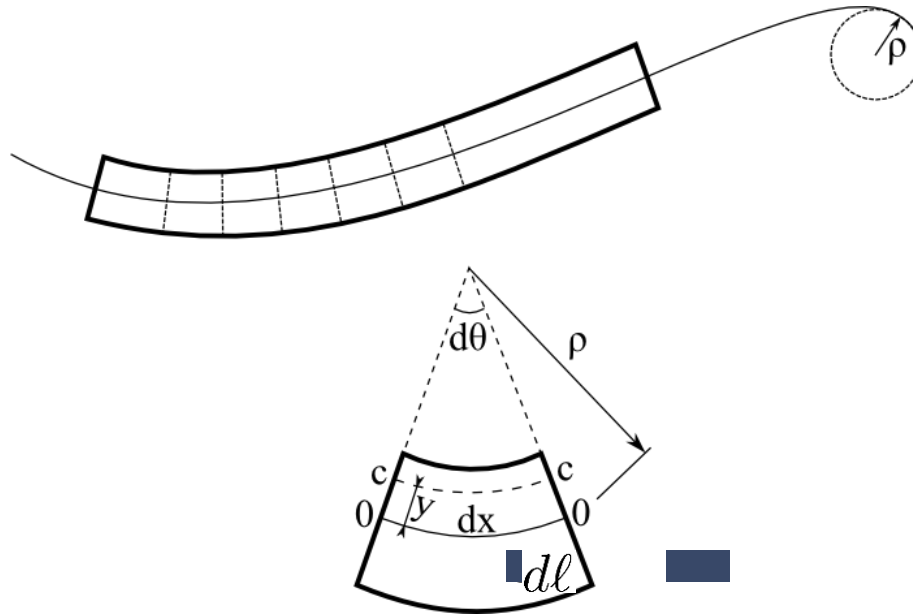


$$d\ell = \sqrt{(dy)^2 + (dx)^2} = \sqrt{1 + (y')^2} dx$$



$$\kappa = \frac{1}{\rho} = \frac{d\theta}{d\ell} \approx \frac{d\theta}{dx} \Rightarrow \kappa \approx \frac{d^2y}{dx^2} = \frac{M(x)}{EI}, \quad (I)$$

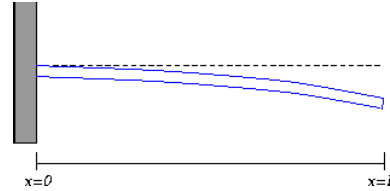
$$\frac{dV}{dx} = \frac{d^2M}{dx^2} = -w(x) \Rightarrow \frac{d^2}{dx^2}(EI \frac{d^2y}{dx^2}) = -w(x), \quad (II)$$




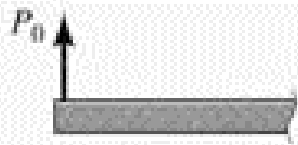
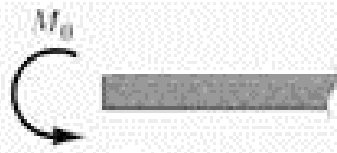


$$\frac{d^2}{dX^2} \left(EI \frac{d^2 y}{dX^2} \right) = -w(x), \quad 0 < X < L$$

How to integrate beam deflection ?

Boundary Conditions



| Type | Symbol* |
|---------------------|--|
| Fixed End |  $X=0$ |
| Simple Support |  |
| Free End |  |
| Concentrated Force |  |
| Concentrated Couple |  |

1: $y(0) = 0, \theta(0) = y'(0) = 0;$

2: $y(0) = 0, \kappa(0) = M(0) = EIy''(0) = 0;$

3: $M(0) = EIy''(0) = 0, V(0) = EIy'''(0) = 0;$

4: $M(0) = EIy''(0) = 0, V(0) = EIy'''(0) = P_0;$

5: $M(0) = EIy''(0) = -M_0, V(0) = EIy'''(0) = 0;$

Equation of the Elastic Curve (II) $EIy^{(iv)}(x) = -w(x)$:

$$(1) V(x) = EIy'''(x) = \int_0^x -w(t_1)dt_1 + C_1, \text{ so } C_1 = V(0) = EIy'''(0);$$

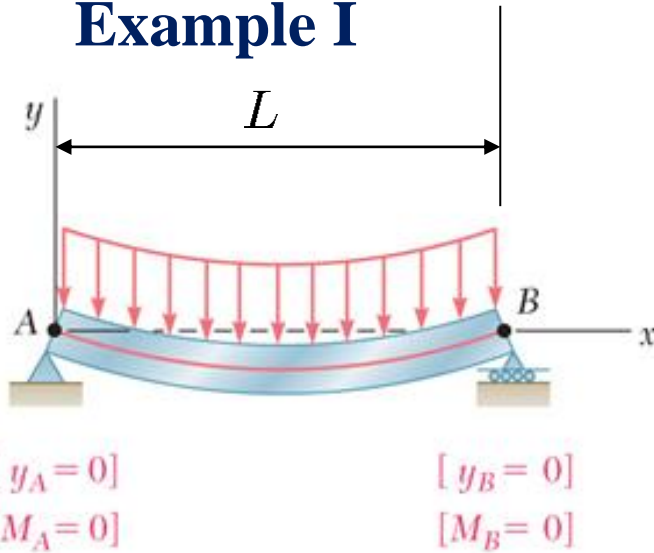
$$(2) M(x) = EIy''(x) = \int_0^x \int_0^{t_2} -w(t_1)dt_1dt_2 + C_1x + C_2, \\ \text{so } C_2 = M(0) = EIy''(0);$$

$$(3) EI\theta = EIy'(x) = \int_0^x \int_0^{t_3} \int_0^{t_2} -w(t_1)dt_1dt_2dt_3 + C_1\frac{x^2}{2} + C_2x + C_3, \\ \text{so } C_3 = EI\theta(0);$$

$$(4) EIy(x) = \int_0^x \int_0^{t_4} \int_0^{t_3} \int_0^{t_2} -w(t_1)dt_1dt_2dt_3dt_4 + C_1\frac{x^3}{6} + C_2\frac{x^2}{2} \\ + C_3x + C_4, \text{ so } C_4 = EIy(0);$$

Remark: In a given problem, one cannot know all four boundary conditions at one end. One can only find two boundary conditions at a given end.

Example I



$$w(x) = w_0 \quad EI y^{(iv)} = -w_0$$

[Solution]

$$EI y''' = -w_0 x + C_1$$

$$EI y'' = -\frac{w_0 x^2}{2} + C_1 x + C_2, \quad (C_2 = M(0) = 0)$$

Based on $M(L) = 0$,

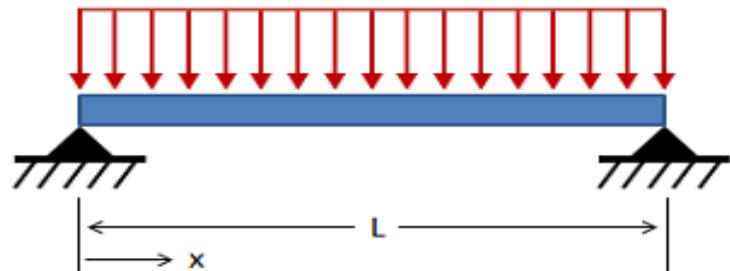
$$\rightarrow -w_0 L^2/2 + C_1 L = 0, \quad \rightarrow C_1 = \frac{1}{2} w_0 L = V(0)$$

$$EI y' = -\frac{w_0 x^3}{6} + \frac{w_0 L x^2}{4} + C_3$$

$$EI y(x) = -\frac{w_0 x^4}{24} + \frac{w_0 L x^3}{12} + C_3 x + C_4 \quad y(0) = 0, \quad \rightarrow C_4 = 0$$

$$EI y(L) = -\frac{w_0 L^4}{24} + \frac{w_0 L^4}{12} + C_3 L = 0 \quad \rightarrow C_3 = -\frac{w_0 L^3}{24} = \theta(0)$$

$$EIy^{(iv)}(x) = -w_0$$

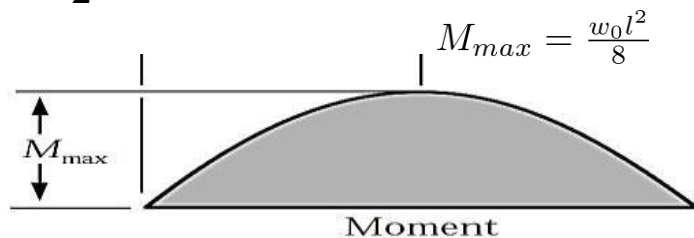


$$EIy'''(x) = V(x) = -w_0x + \frac{w_0L}{2}$$

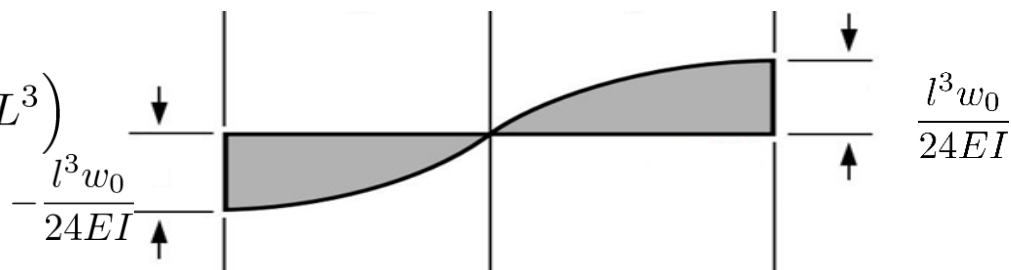
$$V(0) = \frac{w_0l}{2}$$



$$EIy''(x) = M(x) = -\frac{w_0x^2}{2} + \frac{w_0Lx}{2}$$



$$y'(x) = \theta(x) = -\frac{w_0}{24EI} \left(4x^3 - 6Lx^2 + L^3 \right)$$



$$y(x) = -\frac{w_0}{24EI} [x^4 - 2Lx^3 + L^3x]$$

