AE731

Theory of Elasticity

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upcoming schedule

- Sep 30 Boundary Conditions
- Oct 1 Homework 3 Self-grade Due, Homework 4 Due
- Oct 5 Problem Formulation
- Oct 7 Solution Strategies
- Oct 8 Homework 4 Self-grade Due, Homework 5 Due
- (Oct 12) Fall Break (No Class)

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outline

- field equations
- boundary conditions
- stress formulation
- example

field equations

field equations

$$\begin{split} \epsilon_{ij} &= \frac{1}{2} (u_{i,j} + u_{j,i}) \qquad \text{Strain-Displacement} \\ \sigma_{ij,j} + F_i &= 0 \qquad \text{Equilibrium} \\ \sigma_{ij} &= \lambda \epsilon_{kk} \delta_{ij} + 2\mu \epsilon_{ij} \qquad \text{Constitutive (Hooke's Law)} \\ \epsilon_{ij} &= \frac{1+\nu}{E} \sigma_{ij} - \frac{\nu}{E} \sigma_{kk} \delta_{ij} \end{split}$$

field equations

- There are 15 unique field equations to solve for the 15 unknowns
- 3 displacements (u_i), 6 unique strain tensor terms (ε_{ij}), and 6 unique stress tensor terms (σ_{ij})
- These equations also depend on a knowledge of the material behavior (λ, μ) and body forces (usually gravity or zero)

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compatibility equations

- If continuous, single-valued displacements are specified, differentiation will result in well-behaved strain field
- The inverse relationship, integration of a strain field to find displacement, may not always be true
- There are cases where we can integrate a strain field to find a set of discontinuous displacements

compatibility

- The compatibility equations enforce continuity of displacements to prevent this from occurring
- To enforce this condition we consider the strain-displacement relations:

$$\epsilon_{ij} = \frac{1}{2}(u_{i,j} + u_{j,i})$$

compatibility

• and differentiate with respect to x_k and x_l

$$\epsilon_{ij,kl} = \frac{1}{2}(u_{i,jkl} + u_{j,ikl})$$

Or

$$2\epsilon_{ij,kl} = u_{i,jkl} + u_{j,ikl}$$

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compatibility

 We can eliminate the displacement terms from the equation by interchanging the indexes to generate new equations

$$2\epsilon_{ik,jl} = u_{i,jkl} + u_{k,ijl}$$
$$2\epsilon_{il,ik} = u_{i,ikl} + u_{l,iik}$$

• Solving for $u_{i,jkl}$ and $u_{j,ikl}$

$$u_{i,jkl} = 2\epsilon_{ik,jl} - u_{k,ijl}$$

$$u_{j,ikl} = 2\epsilon_{jl,ik} - u_{l,ijk}$$

compatibility

Substituting these values into the equations gives

$$2\epsilon_{ij,kl} = 2\epsilon_{ik,jl} = u_{k,ijl} + 2\epsilon_{jl,ik} - u_{l,ijk}$$

• We now consider one more change of index equation

$$2\epsilon_{kl,ij} = u_{k,ijl} + u_{l,ijk}$$

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compatibility

• and substituting this result gives

$$2\epsilon_{ij,kl} = 2\epsilon_{ik,jl} + 2\epsilon_{jl,ik} - 2\epsilon_{kl,ij}$$

Or, simplified

$$\epsilon_{ii,kl} + \epsilon_{kl,ii} - \epsilon_{ik,il} - \epsilon_{il,ik} = 0$$

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compatibility

- The so-called Saint-Venant compatibility equations in full are a system of 81 equations, but only six are useful (although even these six are not entirely linearly independent)
- These six are found by setting k=1, or in expanded form

$$\begin{split} \frac{\partial^{2} \epsilon_{\mathbf{x}}}{\partial y^{2}} &+ \frac{\partial^{2} \epsilon_{\mathbf{y}}}{\partial x^{2}} = 2 \frac{\partial^{2} \epsilon_{\mathbf{x} \mathbf{y}}}{\partial x \partial y} \\ \frac{\partial^{2} \epsilon_{\mathbf{y}}}{\partial z^{2}} &+ \frac{\partial^{2} \epsilon_{\mathbf{z}}}{\partial y^{2}} = 2 \frac{\partial^{2} \epsilon_{\mathbf{y} \mathbf{z}}}{\partial y \partial \mathbf{z}} \\ \frac{\partial^{2} \epsilon_{\mathbf{z}}}{\partial x^{2}} &+ \frac{\partial^{2} \epsilon_{\mathbf{x}}}{\partial z^{2}} = 2 \frac{\partial^{2} \epsilon_{\mathbf{z} \mathbf{x}}}{\partial z \partial \mathbf{x}} \\ \frac{\partial^{2} \epsilon_{\mathbf{x}}}{\partial y \partial z} &= \frac{\partial}{\partial x} \left(-\frac{\partial \epsilon_{\mathbf{y} \mathbf{z}}}{\partial x} + \frac{\partial \epsilon_{\mathbf{z} \mathbf{x}}}{\partial y} + \frac{\partial \epsilon_{\mathbf{x} \mathbf{y}}}{\partial z} \right) \\ \frac{\partial^{2} \epsilon_{\mathbf{y}}}{\partial z \partial x} &= \frac{\partial}{\partial y} \left(-\frac{\partial \epsilon_{\mathbf{z} \mathbf{x}}}{\partial y} + \frac{\partial \epsilon_{\mathbf{x} \mathbf{y}}}{\partial z} + \frac{\partial \epsilon_{\mathbf{y} \mathbf{z}}}{\partial x} \right) \\ \frac{\partial^{2} \epsilon_{\mathbf{z}}}{\partial x \partial y} &= \frac{\partial}{\partial z} \left(-\frac{\partial \epsilon_{\mathbf{x} \mathbf{y}}}{\partial z} + \frac{\partial \epsilon_{\mathbf{y} \mathbf{z}}}{\partial x} + \frac{\partial \epsilon_{\mathbf{y} \mathbf{z}}}{\partial x} \right) \end{split}$$

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compatibility

- The compatibility equations are necessary to ensure that the strain field is valid and will produce a continuous displacement field
- While these equations are important and necessary in solving elasticity problems, they are not sufficient
- 15 equations with 15 "unknowns" but each of these "unknowns" could actually be a function with many more unknowns, we need to develop framework for simplifying the problem into something we can solve

boundary conditions

boundary conditions

- Boundary conditions commonly specify how a body is supported and/or how it is loaded
- Mathematically we treat this conditions as displacements or tractions at boundary points.
- Symmetry boundary conditions are also common, can reduce computational cost and simplify analytic solutions.

boundary conditions

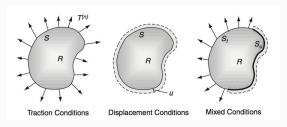


Figure 1: illustration of boundary conditions

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symmetric boundaries

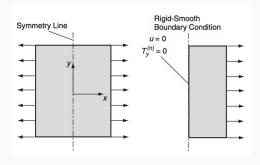


Figure 2: illustration of symmetric boundaries

coordinate systems

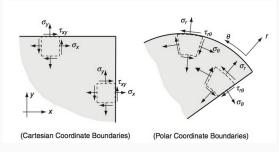
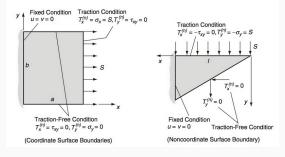


Figure 3: coordinate systems

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boundaries

 In many systems, the boundaries are parallel to the coordinate system, but this is not always the case



boundaries

- We often translate traction boundary conditions into stress boundary conditions using Cauchy's Stress Theorem
- When the condition is on a face parallel to the coordinate system, this gives a zero-stress condition

$$t_i = \sigma_{ii} n_i$$

• This results in $\sigma_{xy} = \sigma_{yy} = 0$

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boundaries

 When the boundary is not parallel to the coordinate system, we do not necessarily have any zero-stress conditions

$$t_x = \sigma_x n_x + \tau_{xy} n_y = 0$$

$$t_y = \tau_{xy} n_x + \sigma_y n_y = 0$$

interfaces

- When we deal with multiple materials, we must prescribe conditions at the interface of these materials
- Some common interface conditions are
 - Perfectly bonded interface where displacements and tractions are continuous at the interface
 - Slip interface where only normal displacements and tractions are continuous at the interface, with no tangential traction and potentially discontinuous tangential displacement

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stress formulation

stress formulation

- For traction problems (i.e. traction is defined on all surfaces) it is convenient to re-formulate field equations in terms of stress only
- Since displacements are eliminated, we will need to use the compatibility equations to ensure a continuous displacement field
- It is desirable for this formulation to write the compatibility equations in terms of stress

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stress formulation

• We start by using Hooke's law for each of the strain terms

$$\epsilon_{ij} = \frac{1+\nu}{F}\sigma_{ij} - \frac{\nu}{F}\sigma_{kk}\delta_{ij}$$

• After some tedious algebra, we find

$$\sigma_{ij,kk} + \sigma_{kk,ij} - \sigma_{ik,jk} - \sigma_{jk,ik} = \frac{\nu}{1+\nu} (\sigma_{mm,kk} \delta_{ij} + \sigma_{mm,ij} \delta_{kk} - \sigma_{mm,jk} \delta_{ik} - \sigma_{mm,ik}$$

 If we also include the equilibrium equations (σ_{ij,j} - F_i) in the formulation, we find

$$\sigma_{ij,kk} + \frac{1}{1+\nu}\sigma_{kk,ij} = \frac{\nu}{1+\nu}\sigma_{mm,kk}\delta_{ij} - F_{i,j} - F_{j,i}$$

 We can further simplify the equation by considering the case when i=i and noting that

$$\sigma_{ii,kk} = -\frac{1+\nu}{1-\nu} F_{i,i}$$

stress formulation

• Which we can substitute into the equation to find

$$\sigma_{ij,kk} + \frac{1}{1+\nu} \sigma_{kk,ij} = -\frac{\nu}{1+\nu} \delta_{ij} F_{k,k} - F_{i,j} - F_{j,i}$$

- The compatibility equations in terms of stress are commonly known as the Beltrami-Michell compatibility equations
- When there are no body forces, we can write the six expanded form equations as

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$$(1+\nu)\nabla^{2}\sigma_{x} + \frac{\partial^{2}}{\partial x^{2}}(\sigma_{x} + \sigma_{y} + \sigma_{z}) = 0 \ (1+\nu)\nabla^{2}\sigma_{y} + \frac{\partial^{2}}{\partial y^{2}}(\sigma_{x} + \sigma_{y} + \sigma_{z}) + (1+\nu)\nabla^{2}\sigma_{z} + \frac{\partial^{2}}{\partial z^{2}}(\sigma_{x} + \sigma_{y} + \sigma_{z}) = 0$$

$$(1+\nu)\nabla^{2}\sigma_{xy} + \frac{\partial^{2}}{\partial x \partial y}(\sigma_{x} + \sigma_{y} + \sigma_{z}) = 0$$

$$(1+\nu)\nabla^{2}\tau_{yz} + \frac{\partial^{2}}{\partial y \partial z}(\sigma_{x} + \sigma_{y} + \sigma_{z}) = 0$$

$$(1+\nu)\nabla^{2}\tau_{zx} + \frac{\partial^{2}}{\partial z \partial y}(\sigma_{x} + \sigma_{y} + \sigma_{z}) = 0$$

stress formulation

- When working with traction boundary problems, these compatibility equations, together with the equilibrium equations, are sufficient to solve the problem
- These partial differential equations are not easy to solve, and analytic problems approached this way are often solved only in 2D
- Solutions are also commonly based on stress functions, which gives a base equation form that automatically satisfies equilibrium

solution methods

- Direct method
 - Solved via direction integration
 - Limited to very simple geometries
- Inverse method
 - Choose a basic form for the solution based on our knowledge of the problem
 - Solve for coefficients
 - Usually we know the answer before we know the problem, it can be difficult to find useful problems for our solution

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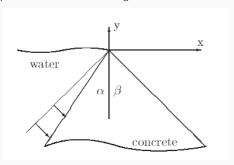
solution methods

- Semi-inverse method
 - Only part of the solution is assumed
 - Use direct integration to find the rest

example

Levy's problem

• Find the stresses in a semi-infinite wedge due to fluid pressure and its own self-weight



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Levy's problem

 Since pressure varies linearly with depth, we will assume a linear state of stress

$$\sigma_{x} = a_{1}x + b_{1}y + c_{1}$$

$$\sigma_{y} = a_{2}x + b_{2}y + c_{2}$$

$$\tau_{xy} = a_{12}x + b_{12}y + c_{12}$$

This leaves 9 coefficients to be determined

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Levy's problem

- First let us consider the boundary conditions at the apex of the dam
- If we let the origin be at the apex of the dam, which must be traction free, we find

$$c_1 = c_2 = c_{12} = 0$$

Levy's problem

• Next let us consider the equilibrium equations

$$\sigma_{x,x} + \tau_{xy,y} + \rho b_x = 0$$

$$\tau_{xy,x} + \sigma_{y,y} + \rho b_y = 0$$

Which in this case become

$$a_1 + b_{12} + 0 = 0$$
$$a_{12} + b_2 - \rho g = 0$$

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Levy's problem

The stresses can now be written as

$$\sigma_x = a_1 x + b_1 y$$

$$\sigma_y = a_2 x + b_2 y$$

$$\tau_{xy} = -b_2 x + \rho g x - a_1 y$$

Levy's problem

- The compatibility equations are all satisfied, as these linear functions will all go to zero when taking second derivatives
- We now consider the boundary conditions along both faces