

Submittal

General Contractor North American Pipeline Services, LLC

Job Name: SEWER & MANHOLE REHABILITATION

Contract Number: SR-01-15-15B

Submitted to: North American Pipeline Services, LLC
210 Bennett Road, Freehold, NJ 07728

Engineer: Maser Consulting, P.A.
331 Newman Springs Road, Suite 203, Red Bank, NJ 07701
Project No. 15001124A

Allstate Submittal #: 1

Item Submitted: CIPP Liner Designs

Manufacturer: N/A

Date Submitted: March 17, 2016

Specification Section: 02607

Item is as specified
in contract documents: Yes

Certification Statement:

By this submittal, I hereby represent that I have determined and verified all field measurements, field construction criteria, materials, dimensions, catalog numbers and similar data and I have checked and coordinated each item with other applicable approved shop drawings and contract requirements.

Submitted By:



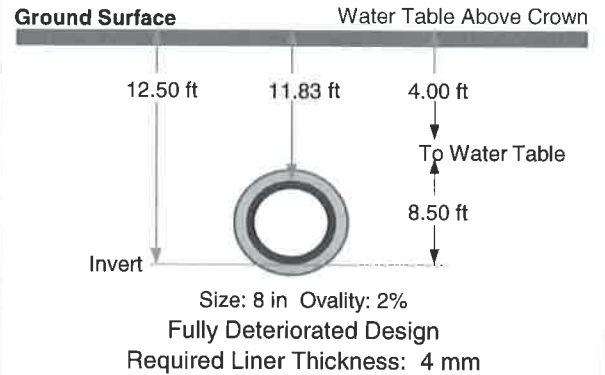
Al Hickson
Allstate Power Vac, Inc



PROJECT INFORMATION

 11-Mar-16
TOMS RIVER MUNICIPAL UTILITIES AUTHORITY
CONTRACT NO. SR-01-15B
SEWER AND MANHOLE REHABILITATION

8-inch



BY ASTM F1216 VERSION	F1216-09	CIPP liner design by Appendix X1 method of ASTM F1216-09	
EXISTING PIPE PARAMETERS	ENTERED	FACTOR SUMMARY - FULLY DETERIORATED	
Design Condition	Fully Det.	Flexural Modulus Design	125,000 psi 50% of Short-term
Inside Dia. of Existing Pipe	8 in	Flexural Strength Design	2,250 psi 50% of Short-term
Depth to Invert	12.5 ft	Minimum Dia for host pipe	7.84 in For 2% ovality
Water Table Below Surface	4 ft	Maximum Dia for host pipe	8.16 in For 2% ovality
Ovality, Δ	2%	Ovality Reduction Factor, C	0.836
Soil Density	120 lb/ft ³	Water Buoyancy Factor, R _w	0.782
Soil Modulus	1,000 psi	Coeff of Elastic Support, B'	0.3504
Live Load	HS-20	Water Pressure, Invert	3.68 psi 8.50 ft Head
Other Load	0 psi	Vacuum Pressure, Invert	0.00 psi
Vacuum Condition	0 psi	Total Design Pressure, Invert	0.00 psi For X1.1 & X1.2
CIPP LINER PARAMETERS	ENTERED	Water Pressure, Overt	3.39 psi 7.83 ft Head
Flexural Modulus short-term	250,000 psi	Soil Pressure, Overt	7.71 psi 11.83 ft Cover
Flexural Strength short-term	4,500 psi	Live Load Pressure, Overt	0.61 psi Note 1
Long-term Retention	50%	Other Load Pressure, Overt	0.00 psi
Enhancement Factor	7	Total Design Pressure, Overt	11.71 psi For Eq X1.3
Poisson's Ratio	0.3	Note 1: AASHTO HS-20. Refer AWWA M11, M23, M55.	
Safety Factor	2		

FULLY DETERIORATED DESIGN REQUIRES SATISFYING F1216-X1 EQUATIONS X1.1, X1.2, X1.3 & X1.4

Equations X1.1, X1.2, X1.3 & X1.4 solved for liner thickness t	t mm	t in	DR
X1.1: $P = [2KE_L / (1 - v^2)] \times [1 / (DR - 1)^3] \times [C/N]$ For load due to groundwater at invert	3.4 mm	0.13 in	59.8
X1.2: $(1.5\Delta/100)(1 + \Delta/100)DR^2 - 0.5(1 + \Delta/100)DR = \sigma_L / (PN)$ For minimum thickness for ovality	1.9 mm	0.07 in	106.9
X1.3: $qt = [1/N] \times [32 \times R_w \times B' \times E' \times C_x (ELxI/D^3)]^{1/2}$ For hydraulic, soil & live loads at overt	4.0 mm	0.16 in	50.8
X1.4: $EI/D^3 = E/[12(DR^3)] \geq 0.093$ For minimum thickness fully deteriorated	3.4 mm	0.13 in	59.8
Required Liner Thickness - Fully Deteriorated	4.0 mm	0.16 in	50.8

t mm is rounded-up to 1 decimal place; t in = t mm/25.4; DR = (Inside Diameter in)/(t mm/25.4) NA - Not Available/Applicable

FLOW COMPARISON PARAMETERS		FLOW COMPARISON FOR: ENTERED LINER THICKNESS	
Liner Thickness - Entered	6.0 mm	Inside Diameter before Lining	8.00 in
Before Lining Manning n	0.0130	Inside Diameter after Lining	7.53 in 6.0 mm liner
After Lining Manning n	0.0100	Flow Capacity after Lining	111% Of before Lining

COMMENTS

 MUST ADD 10% TO THICKNESS FOR RESIN MIGRATION (4MM X 10% = 4.4MM)
WILL INSTALL LINER WITH A THICKNESS OF 6MM

ASTM F1216 APPENDIX X1 CALCULATION DETAILS: FULLY DETERIORATED DESIGN

F1216-09

Fully deteriorated design requires satisfying 4 equations: X1.1, X1.2, X1.3 and X1.4

Check Equation X1.1

$$P = [2KE_L / (1 - \nu^2)] \times [1 / (DR - 1)^3] \times [C / N]$$

P is the maximum allowed external pressure on the liner, with safety factor, from groundwater and any vacuum
Determine P for liner thickness of t = 4.0 mm t is from summary page

K = Enhancement factor = 7

E_L = Flexural Modulus Long-term = (Flexural Modulus Short-term) x (Long-term Retention)
= 250000 x 50% = 125000 psi

ν = Poisson's ratio = 0.3

DR = D/t = 8/(4/25.4) = 50.8 where D = inside diameter of existing pipe as entered

C = Ovality Reduction Factor = $[(1 - \Delta/100) / (1 + \Delta/100)^2]^3$, where Δ is ovality of host pipe as entered.

Δ = 2

$$C = [(1 - 2/100) / (1 + 2/100)^2]^3 = 0.836$$

N = Safety Factor = 2 As entered.

$$P = [(2 \times 7 \times 125000) / (1 - 0.3^2)] \times [1 / (50.8 - 1)^3] \times [0.836 / 2] = 6.51 \text{ psi}$$

Determine actual external pressure on liner, Pa

Pa = Ground water pressure, P_{gw}, + Vacuum pressure, P_v, (if any vacuum)

P_{gw} = 0.433 x H = 0.433 x 8.5 ft = 3.68 psi. Where H is height of water over invert.

P_v = 0 psi As entered.

$$Pa = P_{gw} + P_v = 3.68 + 0 = 3.68 \text{ psi}$$

Compare Pa to P

Actual external pressure on liner, Pa = 3.68 psi

Allowed external pressure for 4 mm liner, P = 6.51 psi

Is P ≥ Pa? Yes. Equation X1.1 is satisfied by 4 mm liner thickness

Check for DR ≤ 100 as per F1216 Appendix X1 Note X1.2

DR = 50.8 as calculated above

Is DR ≤ 100? Yes. Note X1.2 is satisfied by liner DR of 50.8

Check Equation X1.2

$$X1.2: [(1.5 \times \Delta/100) \times (1 + \Delta/100) \times DR^2] - [0.5 \times (1 + \Delta/100) \times DR] = (\sigma_L) / (P \times N)$$

Δ = 2 As shown above in determination of C, Ovality Reduction Factor, above.

DR, calculated above = 50.8

σ_L = Flex Strength Long-term = (Flex Strength Short-term) x (Long-term Retention) = 4500 x 50% = 2250 psi

P = External pressure on liner = Pa = 3.68 psi See above

N = safety factor = 2

Solve Eq. X1.2 for liner thickness, t. Where DR = (Liner OD)/(t)

$$t = [3 \times (\Delta/100) \times Do] / [0.5 + \{0.25 + (6 \times (\Delta/100) \times (\sigma_L / (P \times N \times (1 + (\Delta/100))))^{0.5}\}]$$

$$t = [3 \times (2/100) \times 8] / [0.5 + \{0.25 + (6 \times (2/100) \times (2250 / (3.68 \times 2 \times (1 + (2/100))))^{0.5}\}] = 1.9 \text{ mm}$$

Compare liner t to t required by Equation X1.2

Liner t: 4.0 mm t is from summary page

Required t: 1.9 mm

Is Liner t ≥ Required t? Yes. Equation X1.2 is satisfied by 4 mm liner thickness.

Fully Deteriorated calculation details continued on next page

FULL FLOW CAPACITY COMPARISON BEFORE & AFTER LINING - For Entered t

Flow = Q = Area x Velocity = $[(\pi \times D^2) / 4] \times [(1.486/n) \times R^{2/3} \times S^{1/2}]$ Manning formula, imperial units

t = 6 mm

S = Slope = same before & after lining; R = Hydraulic Radius = D/4 for full flow (D in ft)

D1 = 8 in = 0.667 ft

$$Q2/Q1 = \{[(\pi \times (D_2^2) / 4) \times [(1.486/n_2) \times (D_2/4)^{2/3}] / \{[(\pi \times (D_1^2) / 4) \times [(1.486/n_1) \times (D_1/4)^{2/3}]\}]\}$$

D2 = 7.53 in = 0.627 ft

$$= \{[(3.142 \times (0.627^2) / 4) \times [(1.486/0.01) \times (0.627/4)^{2/3}] / \{[(3.142 \times (0.667^2) / 4) \times [(1.486/0.013) \times (0.667/4)^{2/3}]\}]\} = 1.11$$

Q1 is existing (before lining). Q2 is after lining. Lined capacity is 111% of existing capacity.

ASTM F1216 APPENDIX X1 FULLY DETERIORATED CALCULATION DETAILS CONT'D

F1216-09

Check Equation X1.3

If F1216-07a, Equation X1.3 is: $q_t = [C/N] \times [32R_w B' E'_s (E_L I / D^3)]^{1/2}$ Not using this equation

If F1216-09, Equation X1.3 is: $q_t = [1/N] \times [32R_w B' E'_s C (E_L I / D^3)]^{1/2}$ Using this equation

Where q_t is the maximum allowed external pressure on the liner from cover, live loads and other loads

Determine q_t for liner thickness of $t = 4.0 \text{ mm}$ t is from summary page

C = Ovality Reduction Factor, calculated on page 1, = 0.836

N = Safety Factor = 2

R_w = Water Bouyancy Factor (0.67 min , 1.0 max) = $1 - 0.33(H_w/H) = 1 - 0.33(7.83/11.83) = 0.782$

Where H_w and H are height of water and height of soil over top of pipe. See F1216 X1.2.2

B' = Coefficient of elastic support = $1/(1 + 4e^{-0.665H}) = 0.3504$ Where $H = 11.83$ and $e = 2.718$

E'_s = Modulus of soil reaction = 1000 psi. As entered.

E_L = Long-term modulus for CIPP, calculated on page 1, = 125000 psi

I = Moment of inertia for liner = $(t^3)/12 = (4/25.4)^3/12 = 0.000325$

D = Inside diameter of existing pipe (as entered) = mean OD of liner = 8 in

$q_t = [1/N] \times [32 \times R_w \times B' \times E'_s \times C \times (E_L \times I / D^3)]^{1/2}$

$q_t = (1/2 \times [32 \times 0.782 \times 0.3504 \times 1000 \times 0.836 \times ((125000 \times 0.000325) / 8^3)]^{1/2} = 12.06 \text{ psi}$

Determine actual external pressure on liner, q_{ta}

$q_{ta} = P_w + P_s + P_l + P_o$

P_w = Water load = $0.433 \times H_w = 0.433 \times 7.83 = 3.39 \text{ psi}$ H_w is water over top of pipe.

P_s = Soil Load = $(w \times H \times R_w) / 144 = (120 \times 11.83 \times 0.782) / 144 = 7.71 \text{ psi}$ H is soil height over top of pipe

P_l = Live load = 0.61 psi Note 1: AASHTO HS-20. Refer AWWA M11, M23, M55.

P_o = Other load = 0 psi As entered

$q_{ta} = 3.39 + 7.71 + 0.61 + 0 = 11.71 \text{ psi}$

Compare q_{ta} to q_t

$q_{ta} = 11.71 \text{ psi}$ Actual external pressure on liner

$q_t = 12.06 \text{ psi}$ Allowed external pressure for 4 mm liner

Is $q_t \geq q_{ta}$? Yes. Equation X1.3 is satisfied by 4 mm liner thickness.

Check Equation X1.4

$(E \times I) / D^3 = E / (12 \times (DR^3)) \geq 0.093$

Determine for liner thickness $t = 4.0 \text{ mm}$ t is from summary page

E = initial (short-term) modulus = 250000 psi

DR = liner dimension ratio = $D/t = 8 / (4 / 25.4) = 50.8$

$E / (12 \times (DR^3)) = 250000 / (12 \times 50.8^3) = 0.15892$

Is $E / (12 \times (DR^3)) \geq 0.093$? Yes. Equation X1.4 is satisfied by 4 mm liner thickness

Summary for Fully Deteriorated Design

Fully Deteriorated design requires satisfying Eqs X1.1, X1.2, X1.3, X1.4

Eq X1.1 Satisfied by 4 mm liner thickness

Eq X1.2 Satisfied by 4 mm liner thickness

Eq X1.3 Satisfied by 4 mm liner thickness

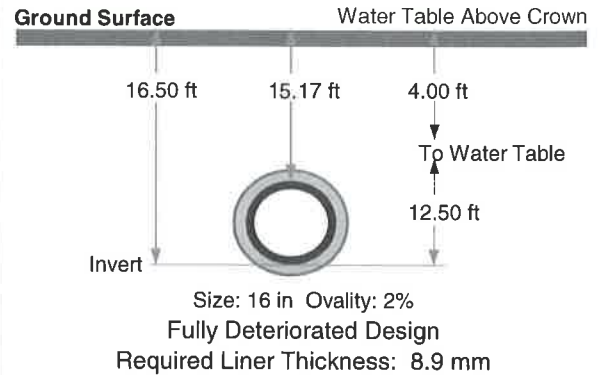
Eq X1.4 Satisfied by 4 mm liner thickness

Required liner thickness for fully deteriorated design is..... 4.0 mm

PROJECT INFORMATION

 11-Mar-16
TOMS RIVER MUNICIPAL UTILITIES AUTHORITY
CONTRACT NO. SR-01-15B
SEWER AND MANHOLE REHABILITATION

16-INCH



BY ASTM F1216 VERSION	F1216-09	CIPP liner design by Appendix X1 method of ASTM F1216-09	
EXISTING PIPE PARAMETERS	ENTERED	FACTOR SUMMARY - FULLY DETERIORATED	
Design Condition	Fully Det.	Flexural Modulus Design	125,000 psi 50% of Short-term
Inside Dia. of Existing Pipe	16 in	Flexural Strength Design	2,250 psi 50% of Short-term
Depth to Invert	16.5 ft	Minimum Dia for host pipe	15.68 in For 2% ovality
Water Table Below Surface	4 ft	Maximum Dia for host pipe	16.32 in For 2% ovality
Ovality, Δ	2%	Ovality Reduction Factor, C	0.836
Soil Density	120 lb/ft ³	Water Buoyancy Factor, R _w	0.757
Soil Modulus	1,000 psi	Coeff of Elastic Support, B'	0.4012
Live Load	HS-20	Water Pressure, Invert	5.41 psi 12.50 ft Head
Other Load	0 psi	Vacuum Pressure, Invert	0.00 psi
Vacuum Condition	0 psi	Total Design Pressure, Invert	0.00 psi For X1.1 & X1.2
CIPP LINER PARAMETERS	ENTERED	Water Pressure, Overt	4.84 psi 11.17 ft Head
Flexural Modulus short-term	250,000 psi	Soil Pressure, Overt	9.57 psi 15.17 ft Cover
Flexural Strength short-term	4,500 psi	Live Load Pressure, Overt	0.28 psi Note 1
Long-term Retention	50%	Other Load Pressure, Overt	0.00 psi
Enhancement Factor	7	Total Design Pressure, Overt	14.69 psi For Eq X1.3
Poisson's Ratio	0.3	Note 1: AASHTO HS-20. Refer AWWA M11, M23, M55.	
Safety Factor	2		

FULLY DETERIORATED DESIGN REQUIRES SATISFYING F1216-X1 EQUATIONS X1.1, X1.2, X1.3 & X1.4

Equations X1.1, X1.2, X1.3 & X1.4 solved for liner thickness t	t mm	t in	DR
X1.1: $P = [2KE_L / (1 - \nu^2)] \times [1 / (DR - 1)^3] \times [C / N]$ For load due to groundwater at invert	7.6 mm	0.30 in	53.5
X1.2: $(1.5\Delta / 100)(1 + \Delta / 100)DR^2 - 0.5(1 + \Delta / 100)DR = \sigma_L / (PN)$ For minimum thickness for ovality	4.5 mm	0.18 in	90.3
X1.3: $qt = [1 / N] \times [32 \times R_w \times B' \times E' \times C_x (EL_x / D^3)]^{1/2}$ For hydraulic, soil & live loads at overt	8.9 mm	0.35 in	45.7
X1.4: $EI / D^3 = E / [12(DR^3)] \geq 0.093$ For minimum thickness fully deteriorated	6.7 mm	0.26 in	60.7
Required Liner Thickness - Fully Deteriorated	8.9 mm	0.35 in	45.7
t mm is rounded-up to 1 decimal place; t in = t mm/25.4; DR = (Inside Diameter in)/(t mm/25.4) NA - Not Available/Applicable			

FLOW COMPARISON PARAMETERS		FLOW COMPARISON FOR: ENTERED LINER THICKNESS		
Liner Thickness - Entered	10.5 mm	Inside Diameter before Lining	16.00 in	
Before Lining Manning n	0.0130	Inside Diameter after Lining	15.17 in	10.5 mm liner
After Lining Manning n	0.0100	Flow Capacity after Lining	113%	Of before Lining

COMMENTS

 MUST ADD 10% TO THICKNESS FOR RESIN MIGRATION (8.9MM X 10% = 9.79MM)
WILL INSTALL LINER WITH A THICKNESS OF 10.5MM

ASTM F1216 APPENDIX X1 CALCULATION DETAILS: FULLY DETERIORATED DESIGN

F1216-09

Fully deteriorated design requires satisfying 4 equations: X1.1, X1.2, X1.3 and X1.4

Check Equation X1.1

$$P = [2KE_L / (1 - \nu^2)] \times [1 / (DR - 1)^3] \times [C / N]$$

P is the maximum allowed external pressure on the liner, with safety factor, from groundwater and any vacuum

Determine P for liner thickness of t = 8.9 mm t is from summary page

K = Enhancement factor = 7

E_L = Flexural Modulus Long-term = (Flexural Modulus Short-term) x (Long-term Retention)

$$= 250000 \times 50\% = 125000 \text{ psi}$$

ν = Poisson's ratio = 0.3

DR = D/t = 16/(8.9/25.4) = 45.66 where D = inside diameter of existing pipe as entered

C = Ovality Reduction Factor = $\left(\frac{[1 - \Delta/100] / [1 + \Delta/100]^2}{\Delta} \right)^3$, where Δ is ovality of host pipe as entered.

$$\Delta = 2$$

$$C = \left(\frac{[1 - 2/100] / [1 + 2/100]^2}{2} \right)^3 = 0.836$$

N = Safety Factor = 2 As entered.

$$P = [(2 \times 7 \times 125000) / (1 - 0.3^2)] \times [1 / (45.66 - 1)^3] \times [0.836 / 2] = 9.02 \text{ psi}$$

Determine actual external pressure on liner, Pa

Pa = Ground water pressure, P_{gw}, + Vacuum pressure, P_v, (if any vacuum)

P_{gw} = 0.433 x H = 0.433 x 12.5 ft = 5.41 psi. Where H is height of water over invert.

P_v = 0 psi As entered.

$$Pa = P_{gw} + P_v = 5.41 + 0 = 5.41 \text{ psi}$$

Compare Pa to P

Actual external pressure on liner, Pa = 5.41 psi

Allowed external pressure for 8.9 mm liner, P = 9.02 psi

Is P ≥ Pa? Yes. Equation X1.1 is satisfied by 8.9 mm liner thickness

Check for DR ≤ 100 as per F1216 Appendix X1 Note X1.2

DR = 45.66 as calculated above

Is DR ≤ 100? Yes. Note X1.2 is satisfied by liner DR of 45.7

Check Equation X1.2

$$X1.2: [(1.5 \times \Delta/100) \times (1 + \Delta/100) \times DR^2] - [0.5 \times (1 + \Delta/100) \times DR] = (\sigma_L) / (P \times N)$$

Δ = 2 As shown above in determination of C, Ovality Reduction Factor, above.

DR, calculated above = 45.66

σ_L = Flex Strength Long-term = (Flex Strength Short-term) x (Long-term Retention) = 4500 x 50% = 2250 psi

P = External pressure on liner = Pa = 5.41 psi See above

N = safety factor = 2

Solve Eq. X1.2 for liner thickness, t. Where DR = (Liner OD)/(t)

$$t = [3 \times (\Delta/100) \times Do] / [0.5 + \{0.25 + (6 \times (\Delta/100) \times (\sigma_L / (P \times N \times (1 + (\Delta/100))))^{0.5}\}]$$

$$t = [3 \times (2/100) \times 16] / [0.5 + \{0.25 + (6 \times (2/100) \times (2250 / (5.41 \times 2 \times (1 + (2/100))))^{0.5}\}] = 4.5 \text{ mm}$$

Compare liner t to t required by Equation X1.2

Liner t: 8.9 mm t is from summary page

Required t: 4.5 mm

Is Liner t ≥ Required t? Yes. Equation X1.2 is satisfied by 8.9 mm liner thickness.

Fully Deteriorated calculation details continued on next page

FULL FLOW CAPACITY COMPARISON BEFORE & AFTER LINING - For Entered t

Flow = Q = Area x Velocity = $[(\pi \times D^2) / 4] \times [(1.486/n) \times R^{2/3} \times S^{1/2}]$ Manning formula, imperial units

$$t = 10.5 \text{ mm}$$

S = Slope = same before & after lining; R = Hydraulic Radius = D/4 for full flow (D in ft)

$$D1 = 16 \text{ in} = 1.333 \text{ ft}$$

$$Q2/Q1 = \{[(\pi \times (D_2^2) / 4) \times [(1.486/n_2)] \times (D_2/4)^{2/3}] / [(\pi \times (D_1^2) / 4) \times [(1.486/n_1)] \times (D_1/4)^{2/3}]\}$$

$$D2 = 15.17 \text{ in} = 1.264 \text{ ft}$$

$$= \{[(3.142 \times (1.264^2) / 4) \times [(1.486/0.01)] \times (1.264/4)^{2/3}] / [(3.142 \times (1.333^2) / 4) \times [(1.486/0.013)] \times (1.333/4)^{2/3}]\} = 1.13$$

Q1 is existing (before lining). Q2 is after lining. Lined capacity is 113% of existing capacity.

ASTM F1216 APPENDIX X1 FULLY DETERIORATED CALCULATION DETAILS CONT'D
F1216-09
Check Equation X1.3

 If F1216-07a, Equation X1.3 is: $q_t = [C/N] \times [32R_w B' E'_s (E_L I / D^3)]^{1/2}$ Not using this equation

 If F1216-09, Equation X1.3 is: $q_t = [1/N] \times [32R_w B' E'_s C (E_L I / D^3)]^{1/2}$ Using this equation

 Where q_t is the maximum allowed external pressure on the liner from cover, live loads and other loads

 Determine q_t for liner thickness of $t = 8.9 \text{ mm}$ t is from summary page

C = Ovality Reduction Factor, calculated on page 1, = 0.836

N = Safety Factor = 2

 R_w = Water Bouyancy Factor (0.67 min, 1.0 max) = $1 - 0.33(H_w/H) = 1 - 0.33(11.17/15.17) = 0.757$

 Where H_w and H are height of water and height of soil over top of pipe. See F1216 X1.2.2

 B' = Coefficient of elastic support = $1 / (1 + 4e^{-0.665H}) = 0.4012$ Where $H = 15.17$ and $e = 2.718$
 E'_s = Modulus of soil reaction = 1000 psi. As entered.

 E_L = Long-term modulus for CIPP, calculated on page 1, = 125000 psi

 I = Moment of inertia for liner = $(t^3)/12 = (8.9/25.4)^3/12 = 0.003585$
 D = Inside diameter of existing pipe (as entered) = mean OD of liner = 16 in

 $q_t = [1/N] \times [32 \times R_w \times B' \times E'_s \times C \times (E_L I / D^3)]^{1/2}$
 $q_t = (1/2 \times [32 \times 0.757 \times 0.4012 \times 1000 \times 0.836 \times ((125000 \times 0.003585) / 16^3)]^{1/2} = 14.91 \text{ psi}$
Determine actual external pressure on liner, q_{ta}
 $q_{ta} = P_w + P_s + P_l + P_o$
 P_w = Water load = $0.433 \times H_w = 0.433 \times 11.17 = 4.84 \text{ psi}$ H_w is water over top of pipe.

 P_s = Soil Load = $(w \times H \times R_w) / 144 = (120 \times 15.17 \times 0.757) / 144 = 9.57 \text{ psi}$ H is soil height over top of pipe

 P_l = Live load = 0.28 psi Note 1: AASHTO HS-20. Refer AWWA M11, M23, M55.

 P_o = Other load = 0 psi As entered

 $q_{ta} = 4.84 + 9.57 + 0.28 + 0 = 14.69 \text{ psi}$
Compare q_{ta} to q_t
 $q_{ta} = 14.69 \text{ psi}$ Actual external pressure on liner

 $q_t = 14.91 \text{ psi}$ Allowed external pressure for 8.9 mm liner

 Is $q_t \geq q_{ta}$? Yes. Equation X1.3 is satisfied by 8.9 mm liner thickness.

Check Equation X1.4
 $(E \times I) / D^3 = E / (12 \times (DR^3)) \geq 0.093$

 Determine for liner thickness $t = 8.9 \text{ mm}$ t is from summary page

 E = initial (short-term) modulus = 250000 psi

 DR = liner dimension ratio = $D/t = 16 / (8.9 / 25.4) = 45.66$
 $E / (12 \times (DR^3)) = 250000 / (12 \times 45.66^3) = 0.21885$

 Is $E / (12 \times (DR^3)) \geq 0.093$? Yes. Equation X1.4 is satisfied by 8.9 mm liner thickness

Summary for Fully Deteriorated Design

Fully Deteriorated design requires satisfying Eqs X1.1, X1.2, X1.3, X1.4

Eq X1.1 Satisfied by 8.9 mm liner thickness

Eq X1.2 Satisfied by 8.9 mm liner thickness

Eq X1.3 Satisfied by 8.9 mm liner thickness

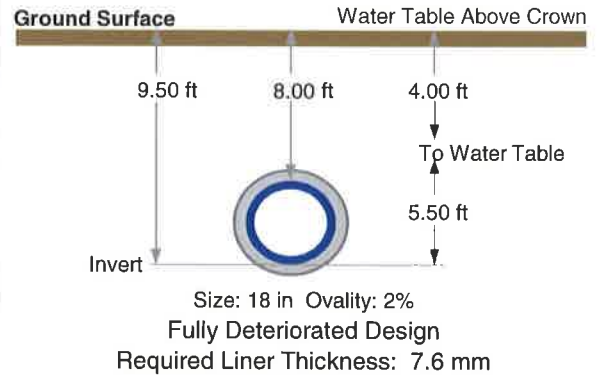
Eq X1.4 Satisfied by 8.9 mm liner thickness

Required liner thickness for fully deteriorated design is..... 8.9 mm

PROJECT INFORMATION

11-Mar-16
TOMS RIVER MUNICIPAL UTILITIES AUTHORITY
CONTRACT NO. SR-01-15B
SEWER AND MANHOLE REHABILITATION

18-inch



BY ASTM F1216 VERSION	F1216-09	CIPP liner design by Appendix X1 method of ASTM F1216-09		
EXISTING PIPE PARAMETERS	ENTERED	FACTOR SUMMARY - FULLY DETERIORATED		
Design Condition	Fully Det.	Flexural Modulus Design	125,000 psi	50% of Short-term
Inside Dia. of Existing Pipe	18 in	Flexural Strength Design	2,250 psi	50% of Short-term
Depth to Invert	9.5 ft	Minimum Dia for host pipe	17.64 in	For 2% ovality
Water Table Below Surface	4 ft	Maximum Dia for host pipe	18.36 in	For 2% ovality
Ovality, Δ	2%	Ovality Reduction Factor, C	0.836	
Soil Density	120 lb/ft ³	Water Buoyancy Factor, R _w	0.835	
Soil Modulus	1,000 psi	Coeff of Elastic Support, B'	0.296	
Live Load	HS-20	Water Pressure, Invert	2.38 psi	5.50 ft Head
Other Load	0 psi	Vacuum Pressure, Invert	0.00 psi	
Vacuum Condition	0 psi	Total Design Pressure, Invert	0.00 psi	For X1.1 & X1.2
CIPP LINER PARAMETERS	ENTERED	Water Pressure, Overt	1.73 psi	4.00 ft Head
Flexural Modulus short-term	250,000 psi	Soil Pressure, Overt	5.57 psi	8.00 ft Cover
Flexural Strength short-term	4,500 psi	Live Load Pressure, Overt	1.05 psi	Note 1
Long-term Retention	50%	Other Load Pressure, Overt	0.00 psi	
Enhancement Factor	7	Total Design Pressure, Overt	8.35 psi	For Eq X1.3
Poisson's Ratio	0.3	Note 1: AASHTO HS-20. Refer AWWA M11, M23, M55.		
Safety Factor	2			

FULLY DETERIORATED DESIGN REQUIRES SATISFYING F1216-X1 EQUATIONS X1.1, X1.2, X1.3 & X1.4

Equations X1.1, X1.2, X1.3 & X1.4 solved for liner thickness t	t mm	t in	DR
X1.1: $P = [2KE_L / (1 - v^2)] \times [1 / (DR - 1)^3] \times [C / N]$ For load due to groundwater at invert	6.5 mm	0.26 in	70.3
X1.2: $(1.5\Delta / 100)(1 + \Delta / 100)DR^2 - 0.5(1 + \Delta / 100)DR = \sigma_L / (PN)$ For minimum thickness for ovality	3.5 mm	0.14 in	130.6
X1.3: $qt = [1 / N] \times [32 \times R_w \times B' \times E' \times s \times C_x (ELxI / D^3)]^{1/2}$ For hydraulic, soil & live loads at overt	7.3 mm	0.29 in	62.6
X1.4: $EI / D^3 = E / [12(DR^3)] \geq 0.093$ For minimum thickness fully deteriorated	7.6 mm	0.30 in	60.2
Required Liner Thickness - Fully Deteriorated	7.6 mm	0.30 in	60.2
t mm is rounded-up to 1 decimal place; t in = t mm/25.4; DR = (Inside Diameter in)/(t mm/25.4) NA - Not Available/Applicable			

FLOW COMPARISON PARAMETERS		FLOW COMPARISON FOR: ENTERED LINER THICKNESS	
Liner Thickness - Entered	9.0 mm	Inside Diameter before Lining	18.00 in
Before Lining Manning n	0.0130	Inside Diameter after Lining	17.29 in
After Lining Manning n	0.0100	Flow Capacity after Lining	117%
		9.0 mm liner Of before Lining	

COMMENTS

MUST ADD 10% TO THICKNESS FOR RESIN MIGRATION (7.6MM X 10% = 8.36MM)
WILL INSTALL LINER WITH A THICKNESS OF 9MM

3/15/16

ASTM F1216 APPENDIX X1 CALCULATION DETAILS: FULLY DETERIORATED DESIGN

F1216-09

Fully deteriorated design requires satisfying 4 equations: X1.1, X1.2, X1.3 and X1.4

Check Equation X1.1

$$P = [2KE_L / (1 - \nu^2)] \times [1 / (DR - 1)^3] \times [C / N]$$

P is the maximum allowed external pressure on the liner, with safety factor, from groundwater and any vacuum

Determine P for liner thickness of t = 7.6 mm t is from summary page

K = Enhancement factor = 7

E_L = Flexural Modulus Long-term = (Flexural Modulus Short-term) x (Long-term Retention)

$$= 250000 \times 50\% = 125000 \text{ psi}$$

ν = Poisson's ratio = 0.3

DR = D/t = 18/(7.6/25.4) = 60.16 where D = inside diameter of existing pipe as entered

C = Ovality Reduction Factor = $[(1 - \Delta/100) / (1 + \Delta/100)]^3$, where Δ is ovality of host pipe as entered.

$$\Delta = 2$$

$$C = [(1 - 2/100) / (1 + 2/100)]^3 = 0.836$$

N = Safety Factor = 2 As entered.

$$P = [(2 \times 7 \times 125000) / (1 - 0.3^2)] \times [1 / (60.16 - 1)^3] \times [0.836 / 2] = 3.88 \text{ psi}$$

Determine actual external pressure on liner, Pa

Pa = Ground water pressure, Pgw, + Vacuum pressure, Pv, (if any vacuum)

Pgw = 0.433 x H = 0.433 x 5.5 ft = 2.38 psi. Where H is height of water over invert.

Pv = 0 psi As entered.

$$Pa = Pgw + Pv = 2.38 + 0 = 2.38 \text{ psi}$$

Compare Pa to P

Actual external pressure on liner, Pa = 2.38 psi

Allowed external pressure for 7.6 mm liner, P = 3.88 psi

Is P ≥ Pa? **Yes. Equation X1.1 is satisfied by 7.6 mm liner thickness**

Check for DR ≤ 100 as per F1216 Appendix X1 Note X1.2

DR = 60.16 as calculated above

Is DR ≤ 100? **Yes. Note X1.2 is satisfied by liner DR of 60.2**

Check Equation X1.2

$$X1.2: [(1.5 \times \Delta/100) \times (1 + \Delta/100) \times DR^2] - [0.5 \times (1 + \Delta/100) \times DR] = (\sigma_L) / (P \times N)$$

$\Delta = 2$ As shown above in determination of C, Ovality Reduction Factor, above.

DR, calculated above = 60.16

σ_L = Flex Strength Long-term = (Flex Strength Short-term) x (Long-term Retention) = 4500 x 50% = 2250 psi

P = External pressure on liner = Pa = 2.38 psi See above

N = safety factor = 2

Solve Eq. X1.2 for liner thickness, t. Where DR = (Liner OD)/(t)

$$t = [3 \times (\Delta/100) \times Do] / [0.5 + \{0.25 + (6 \times (\Delta/100) \times (\sigma_L / (P \times N \times (1 + (\Delta/100))))^{0.5}\}]$$

$$t = [3 \times (2/100) \times 18] / [0.5 + \{0.25 + (6 \times (2/100) \times (2250 / (2.38 \times 2 \times (1 + (2/100))))^{0.5}\}] = 3.5 \text{ mm}$$

Compare liner t to t required by Equation X1.2

Liner t: 7.6 mm t is from summary page

Required t: 3.5 mm

Is Liner t ≥ Required t? **Yes. Equation X1.2 is satisfied by 7.6 mm liner thickness.**

Fully Deteriorated calculation details continued on next page

FULL FLOW CAPACITY COMPARISON BEFORE & AFTER LINING - For Entered t

Flow = Q = Area x Velocity = $[(\pi \times D^2) / 4] \times [(1.486/n) \times R^{2/3} \times S^{1/2}]$ Manning formula, imperial units

$$t = 9 \text{ mm}$$

S = Slope = same before & after lining; R = Hydraulic Radius = D/4 for full flow (D in ft)

$$D1 = 18 \text{ in} = 1.5 \text{ ft}$$

$$Q2/Q1 = [(\pi \times (D_2^2) / 4) \times [(1.486/n_2) \times (D_2/4)^{2/3}] / [(\pi \times (D_1^2) / 4) \times [(1.486/n_1) \times (D_1/4)^{2/3}]]$$

$$D2 = 17.29 \text{ in} = 1.441 \text{ ft}$$

$$= [(3.142 \times (1.441^2) / 4) \times [(1.486/0.01) \times (1.441/4)^{2/3}] / [(3.142 \times (1.5^2) / 4) \times [(1.486/0.013) \times (1.5/4)^{2/3}]] = 1.17$$

Q1 is existing (before lining). Q2 is after lining. Lined capacity is 117% of existing capacity.

ASTM F1216 APPENDIX X1 FULLY DETERIORATED CALCULATION DETAILS CONT'D

F1216-09

Check Equation X1.3

If F1216-07a, Equation X1.3 is: $q_t = [C/N] \times [32R_w B' E'_s (E_L I/D^3)]^{1/2}$

Not using this equation

If F1216-09, Equation X1.3 is: $q_t = [1/N] \times [32R_w B' E'_s C (E_L I/D^3)]^{1/2}$

Using this equation

Where q_t is the maximum allowed external pressure on the liner from cover, live loads and other loads

Determine q_t for liner thickness of $t = 7.6 \text{ mm}$

t is from summary page

C = Ovality Reduction Factor, calculated on page 1, = 0.836

N = Safety Factor = 2

R_w = Water Bouyancy Factor (0.67 min, 1.0 max) = $1 - 0.33(H_w/H) = 1 - 0.33(4/8) = 0.835$

Where H_w and H are height of water and height of soil over top of pipe. See F1216 X1.2.2

B' = Coefficient of elastic support = $1/(1 + 4e^{-0.665H}) = 0.296$ Where $H = 8$ and $e = 2.718$

E'_s = Modulus of soil reaction = 1000 psi. As entered.

E_L = Long-term modulus for CIPP, calculated on page 1, = 125000 psi

I = Moment of inertia for liner = $(t^3)/12 = (7.6/25.4)^3/12 = 0.002232$

D = Inside diameter of existing pipe (as entered) = mean OD of liner = 18 in

$q_t = [1/N] \times [32 \times R_w \times B' \times E'_s \times C \times (E_L \times I/D^3)]^{1/2}$

$q_t = (1/2 \times [32 \times 0.835 \times 0.296 \times 1000 \times 0.836 \times ((125000 \times 0.002232)/18^3)]^{1/2} = 8.89 \text{ psi}$

Determine actual external pressure on liner, q_{ta}

$q_{ta} = P_w + P_s + P_l + P_o$

P_w = Water load = $0.433 \times H_w = 0.433 \times 4 = 1.73 \text{ psi}$ H_w is water over top of pipe.

P_s = Soil Load = $(w \times H \times R_w)/144 = (120 \times 8 \times 0.835)/144 = 5.57 \text{ psi}$ H is soil height over top of pipe

P_l = Live load = 1.05 psi

Note 1: AASHTO HS-20. Refer AWWA M11, M23, M55.

P_o = Other load = 0 psi As entered

$q_{ta} = 1.73 + 5.57 + 1.05 + 0 = 8.35 \text{ psi}$

Compare q_{ta} to q_t

$q_{ta} = 8.35 \text{ psi}$

Actual external pressure on liner

$q_t = 8.89 \text{ psi}$

Allowed external pressure for 7.6 mm liner

Is $q_t \geq q_{ta}$?

Yes. Equation X1.3 is satisfied by 7.6 mm liner thickness.

Check Equation X1.4

$(E \times I)/D^3 = E/(12 \times (DR^3)) \geq 0.093$

Determine for liner thickness $t = 7.6 \text{ mm}$

t is from summary page

E = initial (short-term) modulus = 250000 psi

DR = liner dimension ratio = $D/t = 18 / (7.6 / 25.4) = 60.16$

$E/(12 \times (DR^3)) = 250000/(12 \times 60.16^3) = 0.09568$

Is $E/(12 \times (DR^3)) \geq 0.093$?

Yes. Equation X1.4 is satisfied by 7.6 mm liner thickness

Summary for Fully Deteriorated Design

Fully Deteriorated design requires satisfying Eqs X1.1, X1.2, X1.3, X1.4

Eq X1.1 Satisfied by 7.6 mm liner thickness

Eq X1.2 Satisfied by 7.6 mm liner thickness

Eq X1.3 Satisfied by 7.6 mm liner thickness

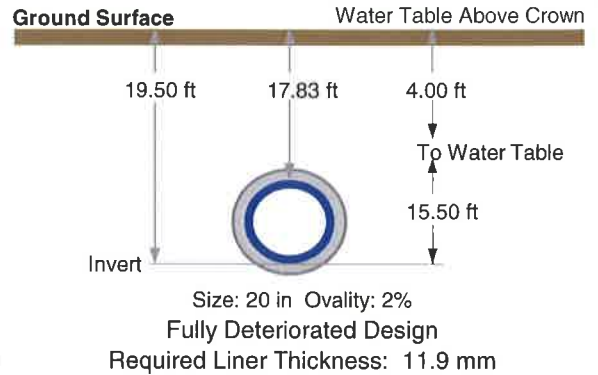
Eq X1.4 Satisfied by 7.6 mm liner thickness

Required liner thickness for fully deteriorated design is..... 7.6 mm

PROJECT INFORMATION

11-Mar-16
TOMS RIVER MUNICIPAL UTILITIES AUTHORITY
CONTRACT NO. SR-01-15B
SEWER AND MANHOLE REHABILITATION

20-inch



BY ASTM F1216 VERSION	F1216-09	CIPP liner design by Appendix X1 method of ASTM F1216-09		
EXISTING PIPE PARAMETERS	ENTERED	FACTOR SUMMARY - FULLY DETERIORATED		
Design Condition	Fully Det.	Flexural Modulus Design	125,000 psi	50% of Short-term
Inside Dia. of Existing Pipe	20 in	Flexural Strength Design	2,250 psi	50% of Short-term
Depth to Invert	19.5 ft	Minimum Dia for host pipe	19.60 in	For 2% ovality
Water Table Below Surface	4 ft	Maximum Dia for host pipe	20.40 in	For 2% ovality
Ovality, Δ	2%	Ovality Reduction Factor, C	0.836	
Soil Density	120 lb/ft ³	Water Buoyancy Factor, R _w	0.744	
Soil Modulus	1,000 psi	Coeff of Elastic Support, B'	0.4434	
Live Load	HS-20	Water Pressure, Invert	6.71 psi	15.50 ft Head
Other Load	0 psi	Vacuum Pressure, Invert	0.00 psi	
Vacuum Condition	0 psi	Total Design Pressure, Invert	0.00 psi	For X1.1 & X1.2
CIPP LINER PARAMETERS	ENTERED	Water Pressure, Overt	5.99 psi	13.83 ft Head
Flexural Modulus short-term	250,000 psi	Soil Pressure, Overt	11.06 psi	17.83 ft Cover
Flexural Strength short-term	4,500 psi	Live Load Pressure, Overt	0.02 psi	Note 1
Long-term Retention	50%	Other Load Pressure, Overt	0.00 psi	
Enhancement Factor	7	Total Design Pressure, Overt	17.06 psi	For Eq X1.3
Poisson's Ratio	0.3	Note 1: AASHTO HS-20. Refer AWWA M11, M23, M55.		
Safety Factor	2			

FULLY DETERIORATED DESIGN REQUIRES SATISFYING F1216-X1 EQUATIONS X1.1, X1.2, X1.3 & X1.4

Equations X1.1, X1.2, X1.3 & X1.4 solved for liner thickness t	t mm	t in	DR
X1.1: $P = [2KE_L / (1 - v^2)] \times [1 / (DR - 1)^3] \times [C / N]$ For load due to groundwater at invert	10.2 mm	0.40 in	49.8
X1.2: $(1.5\Delta / 100)(1 + \Delta / 100)DR^2 - 0.5(1 + \Delta / 100)DR = \sigma_L / (PN)$ For minimum thickness for ovality	6.2 mm	0.24 in	81.9
X1.3: $qt = [1/N] \times [32 \times R_w \times B' \times E' \times C_x (ELxI / D^3)]^{1/2}$ For hydraulic, soil & live loads at overt	11.9 mm	0.47 in	42.7
X1.4: $EI / D^3 = E / [12(DR^3)] \geq 0.093$ For minimum thickness fully deteriorated	8.4 mm	0.33 in	60.5
Required Liner Thickness - Fully Deteriorated	11.9 mm	0.47 in	42.7
t mm is rounded-up to 1 decimal place; t in = t mm/25.4; DR = (Inside Diameter in)/(t mm/25.4) NA - Not Available/Applicable			

FLOW COMPARISON PARAMETERS		FLOW COMPARISON FOR: ENTERED LINER THICKNESS		
Liner Thickness - Entered	13.5 mm	Inside Diameter before Lining	20.00 in	
Before Lining Manning n	0.0130	Inside Diameter after Lining	18.94 in	13.5 mm liner
After Lining Manning n	0.0100	Flow Capacity after Lining	112%	Of before Lining

COMMENTS

MUST ADD 10% TO THICKNESS FOR RESIN MIGRATION (11.9MM X 10% = 13.09MM)
WILL INSTALL LINER WITH A THICKNESS OF 13.5MM

ASTM F1216 APPENDIX X1 CALCULATION DETAILS: FULLY DETERIORATED DESIGN

F1216-09

Fully deteriorated design requires satisfying 4 equations: X1.1, X1.2, X1.3 and X1.4

Check Equation X1.1

$$P = [2KE_L / (1 - \nu^2)] \times [1 / (DR - 1)^3] \times [C / N]$$

P is the maximum allowed external pressure on the liner, with safety factor, from groundwater and any vacuum

Determine P for liner thickness of t = 11.9 mm t is from summary page

K = Enhancement factor = 7

E_L = Flexural Modulus Long-term = (Flexural Modulus Short-term) x (Long-term Retention)

$$= 250000 \times 50\% = 125000 \text{ psi}$$

ν = Poisson's ratio = 0.3

$DR = D/t = 20 / (11.9 / 25.4) = 42.69$ where D = inside diameter of existing pipe as entered

C = Ovality Reduction Factor = $[(1 - \Delta/100) / (1 + \Delta/100)]^3$, where Δ is ovality of host pipe as entered. $\Delta = 2$

$$C = [(1 - 2/100) / (1 + 2/100)]^3 = 0.836$$

N = Safety Factor = 2 As entered.

$$P = [(2 \times 7 \times 125000) / (1 - 0.3^2)] \times [1 / (42.69 - 1)^3] \times [0.836 / 2] = 11.09 \text{ psi}$$

Determine actual external pressure on liner, Pa

Pa = Ground water pressure, Pgw, + Vacuum pressure, Pv, (if any vacuum)

Pgw = $0.433 \times H = 0.433 \times 15.5 \text{ ft} = 6.71 \text{ psi}$. Where H is height of water over invert.

Pv = 0 psi As entered.

$$Pa = Pgw + Pv = 6.71 + 0 = 6.71 \text{ psi}$$

Compare Pa to P

Actual external pressure on liner, Pa = 6.71 psi

Allowed external pressure for 11.9 mm liner, P = 11.09 psi

Is $P \geq Pa$? Yes. Equation X1.1 is satisfied by 11.9 mm liner thickness

Check for $DR \leq 100$ as per F1216 Appendix X1 Note X1.2

DR = 42.69 as calculated above

Is $DR \leq 100$? Yes. Note X1.2 is satisfied by liner DR of 42.7

Check Equation X1.2

$$X1.2: [(1.5 \times \Delta/100) \times (1 + \Delta/100) \times DR^2] - [0.5 \times (1 + \Delta/100) \times DR] = (\sigma_L) / (P \times N)$$

$\Delta = 2$ As shown above in determination of C, Ovality Reduction Factor, above.

DR, calculated above = 42.69

σ_L = Flex Strength Long-term = (Flex Strength Short-term) x (Long-term Retention) = $4500 \times 50\% = 2250 \text{ psi}$

P = External pressure on liner = Pa = 6.71 psi See above

N = safety factor = 2

Solve Eq. X1.2 for liner thickness, t. Where $DR = (\text{Liner OD}) / (t)$

$$t = [3 \times (\Delta/100) \times Do] / [0.5 + \{0.25 + (6 \times (\Delta/100) \times (\sigma_L / (P \times N \times (1 + (\Delta/100))))^{0.5}\}]$$

$$t = [3 \times (2/100) \times 20] / [0.5 + \{0.25 + (6 \times (2/100) \times (2250 / (6.71 \times 2 \times (1 + (2/100))))^{0.5}\}] = 6.2 \text{ mm}$$

Compare liner t to t required by Equation X1.2

Liner t: 11.9 mm t is from summary page

Required t: 6.2 mm

Is Liner t \geq Required t? Yes. Equation X1.2 is satisfied by 11.9 mm liner thickness.

Fully Deteriorated calculation details continued on next page

FULL FLOW CAPACITY COMPARISON BEFORE & AFTER LINING - For Entered t

Flow = Q = Area x Velocity = $[(\pi \times D^2) / 4] \times [(1.486/n) \times R^{2/3} \times S^{1/2}]$ Manning formula, imperial units

t = 13.5 mm

S = Slope = same before & after lining; R = Hydraulic Radius = D/4 for full flow (D in ft)

D1 = 20 in = 1.667 ft

$$Q2/Q1 = \{[(\pi \times (D_2^2) / 4) \times [(1.486/n_2)] \times (D_2/4)^{2/3}] / [(\pi \times (D_1^2) / 4) \times [(1.486/n_1)] \times (D_1/4)^{2/3}]\}$$

D2 = 18.94 in = 1.578 ft

$$= \{[(3.142 \times (1.578^2) / 4) \times [(1.486/0.01)] \times (1.578/4)^{2/3}] / [[(3.142 \times (1.667^2) / 4) \times [(1.486/0.013)] \times (1.667/4)^{2/3}]]\} = 1.12$$

Q1 is existing (before lining). Q2 is after lining. Lined capacity is 112% of existing capacity.

ASTM F1216 APPENDIX X1 FULLY DETERIORATED CALCULATION DETAILS CONT'D

F1216-09

Check Equation X1.3

If F1216-07a, Equation X1.3 is: $q_t = [C/N] \times [32R_w B' E'_s (E_L I / D^3)]^{1/2}$ Not using this equation

If F1216-09, Equation X1.3 is: $q_t = [1/N] \times [32R_w B' E'_s C (E_L I / D^3)]^{1/2}$ Using this equation

Where q_t is the maximum allowed external pressure on the liner from cover, live loads and other loads

Determine q_t for liner thickness of $t = 11.9$ mm t is from summary page

C = Ovality Reduction Factor, calculated on page 1, = 0.836

N = Safety Factor = 2

R_w = Water Bouyancy Factor (0.67 min, 1.0 max) = $1 - 0.33(H_w/H) = 1 - 0.33(13.83/17.83) = 0.744$

Where H_w and H are height of water and height of soil over top of pipe. See F1216 X1.2.2

B' = Coefficient of elastic support = $1 / (1 + 4e^{-0.665H}) = 0.4434$ Where $H = 17.83$ and $e = 2.718$

E'_s = Modulus of soil reaction = 1000 psi. As entered.

E_L = Long-term modulus for CIPP, calculated on page 1, = 125000 psi

I = Moment of inertia for liner = $(t^3)/12 = (11.9/25.4)^3/12 = 0.00857$

D = Inside diameter of existing pipe (as entered) = mean OD of liner = 20 in

$q_t = [1/N] \times [32 \times R_w \times B' \times E'_s \times C \times (E_L \times I / D^3)]^{1/2}$

$q_t = (1/2) \times [32 \times 0.744 \times 0.4434 \times 1000 \times 0.836 \times ((125000 \times 0.00857) / 20^3)]^{1/2} = 17.19$ psi

Determine actual external pressure on liner, q_{ta}

$q_{ta} = P_w + P_s + P_l + P_o$

P_w = Water load = $0.433 \times H_w = 0.433 \times 13.83 = 5.99$ psi H_w is water over top of pipe.

P_s = Soil Load = $(w \times H \times R_w) / 144 = (120 \times 17.83 \times 0.744) / 144 = 11.06$ psi H is soil height over top of pipe

P_l = Live load = 0.02 psi **Note 1: AASHTO HS-20. Refer AWWA M11, M23, M55.**

P_o = Other load = 0 psi As entered

$q_{ta} = 5.99 + 11.06 + 0.02 + 0 = 17.06$ psi

Compare q_{ta} to q_t

$q_{ta} = 17.06$ psi Actual external pressure on liner

$q_t = 17.19$ psi Allowed external pressure for 11.9 mm liner

Is $q_t \geq q_{ta}$? **Yes. Equation X1.3 is satisfied by 11.9 mm liner thickness.**

Check Equation X1.4

$(E \times I) / D^3 = E / (12 \times (DR^3)) \geq 0.093$

Determine for liner thickness $t = 11.9$ mm t is from summary page

E = initial (short-term) modulus = 250000 psi

DR = liner dimension ratio = $D/t = 20 / (11.9 / 25.4) = 42.69$

$E / (12 \times (DR^3)) = 250000 / (12 \times 42.69^3) = 0.26778$

Is $E / (12 \times (DR^3)) \geq 0.093$? **Yes. Equation X1.4 is satisfied by 11.9 mm liner thickness**

Summary for Fully Deteriorated Design

Fully Deteriorated design requires satisfying Eqs X1.1, X1.2, X1.3, X1.4

Eq X1.1 Satisfied by 11.9 mm liner thickness

Eq X1.2 Satisfied by 11.9 mm liner thickness

Eq X1.3 Satisfied by 11.9 mm liner thickness

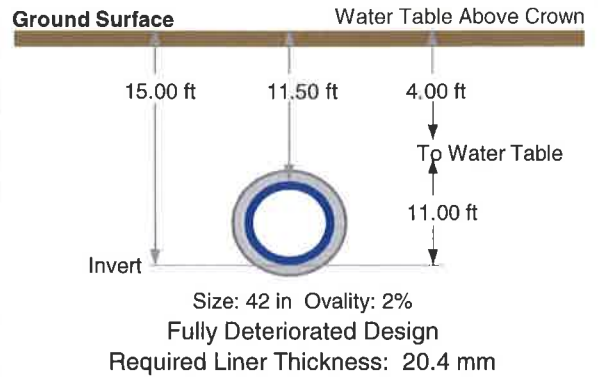
Eq X1.4 Satisfied by 11.9 mm liner thickness

Required liner thickness for fully deteriorated design is..... 11.9 mm

PROJECT INFORMATION

11-Mar-16
TOMS RIVER MUNICIPAL UTILITIES AUTHORITY
CONTRACT NO. SR-01-15B
SEWER AND MANHOLE REHABILITATION

42-inch



BY ASTM F1216 VERSION	F1216-09	CIPP liner design by Appendix X1 method of ASTM F1216-09	
EXISTING PIPE PARAMETERS	ENTERED	FACTOR SUMMARY - FULLY DETERIORATED	
Design Condition	Fully Det.	Flexural Modulus Design	125,000 psi 50% of Short-term
Inside Dia. of Existing Pipe	42 in	Flexural Strength Design	2,250 psi 50% of Short-term
Depth to Invert	15 ft	Minimum Dia for host pipe	41.16 in For 2% ovality
Water Table Below Surface	4 ft	Maximum Dia for host pipe	42.84 in For 2% ovality
Ovality, Δ	2%	Ovality Reduction Factor, C	0.836
Soil Density	120 lb/ft ³	Water Buoyancy Factor, R _w	0.785
Soil Modulus	1,000 psi	Coeff of Elastic Support, B'	0.3455
Live Load	HS-20	Water Pressure, Invert	4.76 psi 11.00 ft Head
Other Load	0 psi	Vacuum Pressure, Invert	0.00 psi
Vacuum Condition	0 psi	Total Design Pressure, Invert	0.00 psi For X1.1 & X1.2
CIPP LINER PARAMETERS	ENTERED	Water Pressure, Overt	3.25 psi 7.50 ft Head
Flexural Modulus short-term	250,000 psi	Soil Pressure, Overt	7.52 psi 11.50 ft Cover
Flexural Strength short-term	4,500 psi	Live Load Pressure, Overt	0.64 psi Note 1
Long-term Retention	50%	Other Load Pressure, Overt	0.00 psi
Enhancement Factor	7	Total Design Pressure, Overt	11.41 psi For Eq X1.3
Poisson's Ratio	0.3	Note 1: AASHTO HS-20. Refer AWWA M11, M23, M55.	
Safety Factor	2		

FULLY DETERIORATED DESIGN REQUIRES SATISFYING F1216-X1 EQUATIONS X1.1, X1.2, X1.3 & X1.4

Equations X1.1, X1.2, X1.3 & X1.4 solved for liner thickness t	t mm	t in	DR
X1.1: $P = [2KE_L / (1 - \nu^2)] \times [1 / (DR - 1)^3] \times [C / N]$ For load due to groundwater at invert	19.0 mm	0.75 in	56.1
X1.2: $(1.5\Delta / 100)(1 + \Delta / 100)DR^2 - 0.5(1 + \Delta / 100)DR = \sigma_L / (PN)$ For minimum thickness for ovality	11.1 mm	0.44 in	96.1
X1.3: $qt = [1 / N] \times [32 \times R_w \times B' \times E' \times s \times C_x (ELxI / D^3)]^{1/2}$ For hydraulic, soil & live loads at overt	20.4 mm	0.80 in	52.3
X1.4: $EI / D^3 = E / [12(DR^3)] \geq 0.093$ For minimum thickness fully deteriorated	17.6 mm	0.69 in	60.6
Required Liner Thickness - Fully Deteriorated	20.4 mm	0.80 in	52.3
t mm is rounded-up to 1 decimal place; t in = t mm/25.4; DR = (Inside Diameter in)/(t mm/25.4) NA - Not Available/Applicable			

FLOW COMPARISON PARAMETERS		FLOW COMPARISON FOR: ENTERED LINER THICKNESS	
Liner Thickness - Entered	22.5 mm	Inside Diameter before Lining	42.00 in
Before Lining Manning n	0.0130	Inside Diameter after Lining	40.23 in
After Lining Manning n	0.0100	Flow Capacity after Lining	116% 22.5 mm liner Of before Lining

COMMENTS

MUST ADD 10% TO THICKNESS FOR RESIN MIGRATION (20.4MM X 10% = 22.44MM)
WILL INSTALL LINER WITH A THICKNESS OF 22.5MM

3/15/16

ASTM F1216 APPENDIX X1 CALCULATION DETAILS: FULLY DETERIORATED DESIGN

F1216-09

Fully deteriorated design requires satisfying 4 equations: X1.1, X1.2, X1.3 and X1.4

Check Equation X1.1

$$P = [2KE_L / (1 - \nu^2)] \times [1 / (DR - 1)^3] \times [C / N]$$

P is the maximum allowed external pressure on the liner, with safety factor, from groundwater and any vacuum

Determine P for liner thickness of t = 20.4 mm t is from summary page

K = Enhancement factor = 7

E_L = Flexural Modulus Long-term = (Flexural Modulus Short-term) x (Long-term Retention)

$$= 250000 \times 50\% = 125000 \text{ psi}$$

ν = Poisson's ratio = 0.3

$DR = D/t = 42 / (20.4 / 25.4) = 52.29$ where D = inside diameter of existing pipe as entered

C = Ovality Reduction Factor = $[(1 - \Delta/100) / (1 + \Delta/100)^2]^3$, where Δ is ovality of host pipe as entered.

$$\Delta = 2$$

$$C = [(1 - 2/100) / (1 + 2/100)^2]^3 = 0.836$$

N = Safety Factor = 2 As entered.

$$P = [(2 \times 7 \times 125000) / (1 - 0.3^2)] \times [1 / (52.29 - 1)^3] \times [0.836 / 2] = 5.96 \text{ psi}$$

Determine actual external pressure on liner, Pa

Pa = Ground water pressure, Pgw, + Vacuum pressure, Pv, (if any vacuum)

Pgw = $0.433 \times H = 0.433 \times 11 \text{ ft} = 4.76 \text{ psi}$. Where H is height of water over invert.

Pv = 0 psi As entered.

$$Pa = Pgw + Pv = 4.76 + 0 = 4.76 \text{ psi}$$

Compare Pa to P

Actual external pressure on liner, Pa = 4.76 psi

Allowed external pressure for 20.4 mm liner, P = 5.96 psi

Is $P \geq Pa$? **Yes. Equation X1.1 is satisfied by 20.4 mm liner thickness**

Check for $DR \leq 100$ as per F1216 Appendix X1 Note X1.2

DR = 52.29 as calculated above

Is $DR \leq 100$? **Yes. Note X1.2 is satisfied by liner DR of 52.3**

Check Equation X1.2

$$X1.2: [(1.5 \times \Delta/100) \times (1 + \Delta/100) \times DR^2] - [0.5 \times (1 + \Delta/100) \times DR] = (\sigma_L) / (P \times N)$$

$\Delta = 2$ As shown above in determination of C, Ovality Reduction Factor, above.

DR, calculated above = 52.29

σ_L = Flex Strength Long-term = (Flex Strength Short-term) x (Long-term Retention) = $4500 \times 50\% = 2250 \text{ psi}$

P = External pressure on liner = Pa = 4.76 psi See above

N = safety factor = 2

Solve Eq. X1.2 for liner thickness, t. Where $DR = (\text{Liner OD}) / (t)$

$$t = [3 \times (\Delta/100) \times D_o] / [0.5 + \{0.25 + (6 \times (\Delta/100) \times (\sigma_L / (P \times N \times (1 + (\Delta/100))))^{0.5}\}]$$

$$t = [3 \times (2/100) \times 42] / [0.5 + \{0.25 + (6 \times (2/100) \times (2250 / (4.76 \times 2 \times (1 + (2/100))))^{0.5}\}] = 11.1 \text{ mm}$$

Compare liner t to t required by Equation X1.2

Liner t: 20.4 mm t is from summary page

Required t: 11.1 mm

Is $\text{Liner } t \geq \text{Required } t$? **Yes. Equation X1.2 is satisfied by 20.4 mm liner thickness.**

Fully Deteriorated calculation details continued on next page

FULL FLOW CAPACITY COMPARISON BEFORE & AFTER LINING - For Entered t

Flow = Q = Area x Velocity = $[(\pi \times D^2) / 4] \times [(1.486/n) \times R^{2/3} \times S^{1/2}]$ Manning formula, imperial units

$$t = 22.5 \text{ mm}$$

S = Slope = same before & after lining; R = Hydraulic Radius = D/4 for full flow (D in ft)

$$D1 = 42 \text{ in} = 3.5 \text{ ft}$$

$$Q2/Q1 = \{[(\pi \times (D_2^2) / 4) \times [(1.486/n_2)] \times (D_2/4)^{2/3}] / [(\pi \times (D_1^2) / 4) \times [(1.486/n_1)] \times (D_1/4)^{2/3}]\}$$

$$D2 = 40.23 \text{ in} = 3.352 \text{ ft}$$

$$= \{[(3.142 \times (3.352^2) / 4) \times [(1.486/0.01)] \times (3.352/4)^{2/3}] / [(3.142 \times (3.5^2) / 4) \times [(1.486/0.013)] \times (3.5/4)^{2/3}]\} = 1.16$$

Q1 is existing (before lining). Q2 is after lining. Lined capacity is 116% of existing capacity.

ASTM F1216 APPENDIX X1 FULLY DETERIORATED CALCULATION DETAILS CONT'D

F1216-09

Check Equation X1.3

If F1216-07a, Equation X1.3 is: $q_t = [C/N] \times [32R_w B' E'_s (E_L I / D^3)]^{1/2}$ Not using this equation

If F1216-09, Equation X1.3 is: $q_t = [1/N] \times [32R_w B' E'_s C (E_L I / D^3)]^{1/2}$ Using this equation

Where q_t is the maximum allowed external pressure on the liner from cover, live loads and other loads

Determine q_t for liner thickness of $t = 20.4 \text{ mm}$ t is from summary page

C = Ovality Reduction Factor, calculated on page 1, = 0.836

N = Safety Factor = 2

R_w = Water Bouyancy Factor (0.67 min, 1.0 max) = $1 - 0.33(H_w/H) = 1 - 0.33(7.5/11.5) = 0.785$

Where H_w and H are height of water and height of soil over top of pipe. See F1216 X1.2.2

B' = Coefficient of elastic support = $1/(1 + 4e^{-0.665H}) = 0.3455$ Where $H = 11.5$ and $e = 2.718$

E'_s = Modulus of soil reaction = 1000 psi. As entered.

E_L = Long-term modulus for CIPP, calculated on page 1, = 125000 psi

I = Moment of inertia for liner = $(t^3)/12 = (20.4/25.4)^3/12 = 0.043173$

D = Inside diameter of existing pipe (as entered) = mean OD of liner = 42 in

$q_t = [1/N] \times [32 \times R_w \times B' \times E'_s \times C \times (E_L \times I / D^3)]^{1/2}$

$q_t = (1/2 \times [32 \times 0.785 \times 0.3455 \times 1000 \times 0.836 \times ((125000 \times 0.043173) / 42^3)]^{1/2} = 11.49 \text{ psi}$

Determine actual external pressure on liner, q_{ta}

$q_{ta} = P_w + P_s + P_l + P_o$

P_w = Water load = $0.433 \times H_w = 0.433 \times 7.5 = 3.25 \text{ psi}$ H_w is water over top of pipe.

P_s = Soil Load = $(w \times H \times R_w) / 144 = (120 \times 11.5 \times 0.785) / 144 = 7.52 \text{ psi}$ H is soil height over top of pipe

P_l = Live load = 0.64 psi

Note 1: AASHTO HS-20. Refer AWWA M11, M23, M55.

P_o = Other load = 0 psi As entered

$q_{ta} = 3.25 + 7.52 + 0.64 + 0 = 11.41 \text{ psi}$

Compare q_{ta} to q_t

$q_{ta} = 11.41 \text{ psi}$

Actual external pressure on liner

$q_t = 11.49 \text{ psi}$

Allowed external pressure for 20.4 mm liner

Is $q_t \geq q_{ta}$?

Yes. Equation X1.3 is satisfied by 20.4 mm liner thickness.

Check Equation X1.4

$(E \times I) / D^3 = E / (12 \times (DR^3)) \geq 0.093$

Determine for liner thickness $t = 20.4 \text{ mm}$ t is from summary page

E = initial (short-term) modulus = 250000 psi

DR = liner dimension ratio = $D/t = 42 / (20.4 / 25.4) = 52.29$

$E / (12 \times (DR^3)) = 250000 / (12 \times 52.29^3) = 0.14571$

Is $E / (12 \times (DR^3)) \geq 0.093$? Yes. Equation X1.4 is satisfied by 20.4 mm liner thickness

Summary for Fully Deteriorated Design

Fully Deteriorated design requires satisfying Eqs X1.1, X1.2, X1.3, X1.4

Eq X1.1 Satisfied by 20.4 mm liner thickness

Eq X1.2 Satisfied by 20.4 mm liner thickness

Eq X1.3 Satisfied by 20.4 mm liner thickness

Eq X1.4 Satisfied by 20.4 mm liner thickness

Required liner thickness for fully deteriorated design is..... 20.4 mm