ME 335A

Finite Element Analysis

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Problems Set #5–Solutions

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A Variational Method with an almost Spectral Basis (53)

Let $\Omega = \{(x_1, x_2) \in \mathbb{R} \mid x_1^2 + x_2^2 < R^2\}$ for R = 1, $\partial \Omega_D = \partial \Omega \cap \{(x_1, x_2) \in \mathbb{R}^2 \mid x_1 \ge 0, x_2 \ge 0\}$, and $\partial \Omega_N = \partial \Omega \setminus \partial \Omega_D$. Consider the problem: Find $u \in \Omega \to \mathbb{R}$ such that

$$-\frac{1}{2}\Delta u = \frac{2}{R^2} \qquad \text{in } \Omega \tag{1a}$$

$$u = 0$$
 on $\partial \Omega_D$ (1b)

$$\frac{1}{2}\nabla u \cdot \check{n} = -\frac{1}{R} \qquad \text{on } \partial\Omega_N. \tag{1c}$$

1. (10) Construct a variational equation that u satisfies, following the standard recipe.

Solution: In the following, we use indicial notation, in which $u_{,i}$ indicates $\partial u/\partial x_i$, and an index repeated twice implies sum over that index.

Setting R = 1, the PDE is

$$-u_{,ii} = 4 in \Omega (2a)$$

$$u = 0$$
 on $\partial \Omega_D$ (2b)

$$u_{i} \check{n}_{i} = -2$$
 on $\partial \Omega_{N}$. (2c)

where $\Omega = \{(x_1, x_2) \in \mathbb{R} \mid x_1^2 + x_2^2 < 1\}$ for R > 0, $\partial \Omega_D = \partial \Omega \cap \{(x_1, x_2) \in \mathbb{R}^2 \mid x_1 \ge 0, x_2 \ge 0\}$, and $\partial \Omega_N = \partial \Omega \setminus \partial \Omega_D$. Consider v smooth enough. We proceed with the procedure to obtain the variational equation we have seen in class:

$$\int_{\Omega} (-u_{,ii}v) \ d\Omega = \int_{\Omega} 4v \ d\Omega \tag{3}$$

$$\int_{\Omega} u_{,i} \, v_{,i} \, d\Omega = \int_{\Omega} 4v \, d\Omega + \int_{\partial \Omega} v u_{,i} \, \check{n}_i \, d\Gamma \tag{4}$$

$$= \int_{\Omega} 4v \ d\Omega + \int_{\partial \Omega_N} v u_{,i} \, \check{n}_i \ d\Gamma \qquad \text{request } v = 0 \text{ on } \partial \Omega_D$$
 (5)

$$= \int_{\Omega} 4v \ d\Omega - 2 \int_{\partial \Omega_N} v \ d\Gamma \tag{6}$$

The variational equation that u satisfies is:

$$a(u,v) = l(v) \qquad \forall v \in \mathcal{V}$$
 (7)

$$a(u,v) = \int_{\Omega} u_{,i} \, v_{,i} \, d\Omega \tag{8}$$

$$l(v) = \int_{\Omega} 4v d\Omega - 2 \int_{\partial \Omega_N} v \, d\Gamma. \tag{9}$$

where $\mathcal{V} = \{u \colon \Omega \to \mathbb{R} \mid u = 0 \text{ on } \partial \Omega_D \}.$

2. (3) Identify essential and natural boundary conditions.

Solution: Equation (1c) was incorporated in the variational equation, and hence it is a natural boundary condition. Therefore,

Essential B.C.
$$u = 0 \text{ on } \partial \Omega_D$$
 (10)

Natural B.C.
$$u_{i} \check{n}_{i} = -2 \text{ on } \partial \Omega_{N}.$$
 (11)

3. Consider the approximation space

$$W_h = \text{span}\left(\sin\left(\pi\left(x_1^2 + x_2^2\right)\right), \cos\left(\frac{\pi}{2}\left(x_1^2 + x_2^2\right)\right), 1\right).$$

(a) (10) Identify test and trial spaces, and active and constrained indices, naming the basis functions with indices in the order they appear above.

Solution: We see that the essential boundary condition is satisfied by $\sin \left(\pi \left(x_1^2 + x_2^2\right)\right)$ and $\cos \left(\frac{\pi}{2} \left(x_1^2 + x_2^2\right)\right)$ but not by 1. The test and trial spaces therefore are

$$S_h = V_h = \operatorname{span}\left\{\sin\left(\pi\left(x_1^2 + x_2^2\right)\right), \cos\left(\frac{\pi}{2}\left(x_1^2 + x_2^2\right)\right)\right\}$$

The active and constrained indices are $\eta_a = \{1, 2\}$, and $\eta_g = \{3\}$.

(b) (5) In this problem, there is a possibility of selecting a smaller space \mathcal{W}_h without changing the results. What is this smaller space \mathcal{W}_h ? Identify active and constrained indices in this new space.

Solution: Because the third basis function, $N_3(x) = 1$, is not a basis function for either S_h or V_h , we can eliminate it from W_h . The smaller space is

$$\mathcal{W}_h = \operatorname{span}\left(\sin\left(\pi\left(x_1^2 + x_2^2\right)\right), \cos\left(\frac{\pi}{2}\left(x_1^2 + x_2^2\right)\right)\right).$$

In this space, $\eta_a = \{1, 2\}$, and $\eta_g = \emptyset$.

(c) (15) Using the smaller space W_h , compute the stiffness matrix and load vector.

Solution:

$$u^{h} = u_{1} \sin \left(\pi \left(x_{1}^{2} + x_{2}^{2}\right)\right) + u_{2} \cos \left(\frac{\pi}{2} \left(x_{1}^{2} + x_{2}^{2}\right)\right) = u_{1} N_{1} + u_{2} N_{2}$$
 (12)

$$v^{h} = v_{1} \sin\left(\pi \left(x_{1}^{2} + x_{2}^{2}\right)\right) + v_{2} \cos\left(\frac{\pi}{2} \left(x_{1}^{2} + x_{2}^{2}\right)\right) = v_{1} N_{1} + v_{2} N_{2}$$
(13)

The solution procedure for a variational method implies that for any i = 1, 2,

$$\sum_{j=1}^{2} a(N_j, N_i) u_j = l(N_i)$$
(14)

The stiffness matrix has entries:

$$K_{11} = \int_{\Omega} (N_{1,1}^2 + N_{1,2}^2) d\Omega \qquad = \int_{\Omega} \nabla N_1 \cdot \nabla N_1 d\Omega \qquad (15)$$

$$K_{12} = K_{21} = \int_{\Omega} N_{1,1} N_{2,1} + N_{1,2} N_{2,2} d\Omega \qquad = \int_{\Omega} \nabla N_1 \cdot \nabla N_2 d\Omega \qquad (16)$$

$$K_{22} = \int_{\Omega} (N_{2,1}^2 + N_{2,2}^2) d\Omega \qquad = \int_{\Omega} \nabla N_2 \cdot \nabla N_2 d\Omega \qquad (17)$$

We can reduce the complexity of integration by introducing the change of variables to polar coordinates. For a generic function f,

$$\nabla f = \frac{\partial f}{\partial r} \hat{u}_r + \frac{1}{r} \frac{\partial f}{\partial \theta} \hat{u}_\theta \tag{18}$$

$$N_1 = \sin(\pi r^2) \tag{19}$$

$$N_2 = \cos\left(\frac{\pi}{2}r^2\right) \tag{20}$$

$$\nabla N_1 = 2\pi r \cos(\pi r^2) \hat{u}_r \tag{21}$$

$$\nabla N_2 = -\pi r \sin(\frac{\pi}{2}r^2)\hat{u}_r \tag{22}$$

$$K_{11} = \int_0^{2\pi} d\theta \int_0^1 \nabla N_1 \cdot \nabla N_1 r dr = \pi^3$$
 (23)

$$K_{12} = \int_0^{2\pi} d\theta \int_0^1 \nabla N_1 \cdot \nabla N_2 r dr = \frac{40\pi}{9}$$
 (24)

$$K_{22} = \int_0^{2\pi} d\theta \int_0^1 \nabla N_2 \cdot \nabla N_2 r dr = \frac{\pi(\pi^2 + 4)}{4}$$
 (25)

$$F_1 = \int_{\Omega} 4N_1 d\Omega - 2 \int_{\partial \Omega_N} N_1 d\Gamma = \int_0^{2\pi} d\theta \int_0^1 4N_1 r dr - 2 \int_{\partial \Omega_N} 0 d\Gamma = 8$$
 (26)

$$F_2 = \int_{\Omega} 4N_2 d\Omega - 2 \int_{\partial \Omega_N} N_2 d\Gamma = \int_0^{2\pi} d\theta \int_0^1 4N_2 r dr - 2 \int_{\partial \Omega_N} 0 d\Gamma = 8$$
 (27)

$$K = \begin{bmatrix} \pi^3 & \frac{40\pi}{9} \\ \frac{40\pi}{9} & \frac{\pi(\pi^2 + 4)}{4} \end{bmatrix}$$
$$F = \begin{bmatrix} 8 \\ 8 \end{bmatrix}$$

(d) (5) Find the numerical approximation. Plot it together with the exact solution. **Solution:** We solve Kd = F,

$$d = \begin{bmatrix} \frac{72(9\pi^2 - 124)}{\pi(324\pi^2 + 81\pi^4 - 6400)} \\ \frac{288(9\pi^2 - 40)}{\pi(324\pi^2 + 81\pi^4 - 6400)} \end{bmatrix} \approx \begin{bmatrix} -0.1720 \\ 0.9548 \end{bmatrix}$$

The plot is in Fig.1.

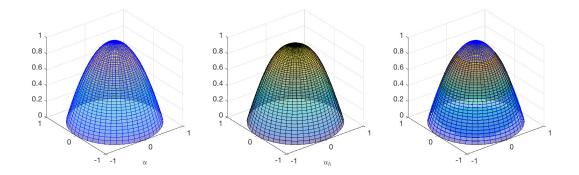


Figure 1: Plot of the exact solution u in blue grids, numerical solution u_h in black grids and two plots superposed together. We can see that the numerical solution underestimates the value at (0,0)

(e) (5) Do you think the numerical approximation would change if we change the boundary condition on the Neumann boundary to

$$\frac{1}{2}\nabla u \cdot \check{n} = -\frac{2}{R}?$$

Solution: Because the two basis functions in V_h are zero on the entire boundary, the Galerkin approximation with this choice of space is not sensitive to the Neumann boundary conditions. A richer approximation space W_h is needed, in particular, one that is not made of only axi-symmetric functions (functions that depend only on r).

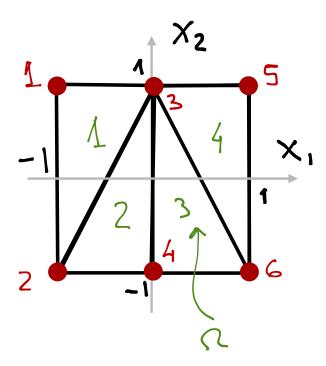
Manual Assembly, Once More (60)

Let $\Omega = [-1,1] \times [-1,1]$, $\partial \Omega_D = \{1\} \times [-1,1]$, and $\partial \Omega_N = \partial \Omega \setminus \partial \Omega_D$. Consider the variational equation that $u \colon \Omega \to \mathbb{R}$ satisfies:

$$\int_{\Omega} \nabla u \cdot \nabla v + uv \ d\Omega = \int_{\Omega} (x_1 + x_2)v \ d\Omega + \int_{\partial \Omega_N} (x_2^2 - 1)v \ d\Gamma$$

for all $v \in \mathcal{V} = \{v : \Omega \to \mathbb{R} \mid v = 0 \text{ on } \partial\Omega_D\}$, where (x_1, x_2) are the Cartesian coordinates in Ω . The function u satisfies the essential boundary condition $u(x_1, x_2) = x_2$ for $(x_1, x_2) \in \partial\Omega_D$.

Consider then the mesh shown in the figure, made of all P_1 elements:



1. (5) What is the local-to-global map for the mesh?

Solution:

$$\mathsf{LG} = \begin{bmatrix} 1 & 2 & 4 & 3 \\ 2 & 4 & 6 & 6 \\ 3 & 3 & 3 & 5 \end{bmatrix}.$$

2. (5) Identify S_h and V_h by providing the general expression for their functions in terms of the P_1 basis functions of the mesh. Identify active and constrained indices.

Solution:

Functions in S_h need to be equal to x_2 on $\partial\Omega_D$, and functions in V_h need to be equal to zero on $\partial\Omega_D$. Fortunately, the function x_2 is in the P_1 finite element space over this mesh, so we can impose this restriction. To this end, we need to set $u_h(1,1) = u_5 = 1$ and $u_h(1,-1) = u_6 = -1$, so that $u_h(1,x_2) = u_5N_5(1,x_2) + u_6N_6(1,x_2) = x_2$.

For \mathcal{V}_h , we need to set the components of basis functions N_5 and N_6 to zero. Therefore,

$$S_h = \{ u_h = u_1 N_1 + u_2 N_2 + u_3 N_3 + u_4 N_4 + N_5 - N_6 \mid (u_1, u_2, u_3, u_4) \in \mathbb{R}^4 \},$$

$$V_h = \{ v_h = v_1 N_1 + v_2 N_2 + v_3 N_3 + v_4 N_4 \mid (v_1, v_2, v_3, v_4) \in \mathbb{R}^4 \}.$$

3. (10) Evaluate the shape function N_3^2 and its derivative ∇N_3^2 at $(x_1, x_2) = (-0.5, -0.1)$.

Solution:

The shape function N_3^2 is the shape function that is equal to 1 at node $\mathsf{LG}(3,2)=3$ and zero at nodes $\mathsf{LG}(1,2)=2$ and $\mathsf{LG}(2,2)=4$. Hence, $N_3^2=\lambda_3$ on element 2.

To solve this problem, we can use the formulas from the notes that give $\lambda_3(x_1, x_2)$. In this case, however, it is simple to build it by inspection. Since N_3^2 is a linear polynomial that is zero when

 $x_2 = -1$ and is 1 when $x_2 = 1$, $N_3^2(x_1, x_2) = (1 + x_2)/2$. Hence,

$$N_3^2(-0.5, -0.1) = 0.45.$$

Its gradient is then

$$\nabla N_3^2(x_1, x_2) = (0, 1/2),$$

which is constant in the element.

4. (20) Compute the element stiffness matrix and load vector for each element. **Solution:** We can write out the expression of every element shape function.

$$N_1^1 = \frac{1}{2}(x_2 - 2x_1 - 1) \tag{28}$$

$$N_2^1 = \frac{1}{2}(-x_2 + 1) \tag{29}$$

$$N_3^1 = x_1 + 1 (30)$$

$$N_1^2 = -x_1 (31)$$

$$N_2^2 = \frac{1}{2}(-x_2 + 2x_1 + 1) \tag{32}$$

$$N_3^2 = \frac{1}{2}(x_2 + 1) \tag{33}$$

$$N_1^3 = \frac{1}{2}(1 - 2x_1 - x_2) \tag{34}$$

$$N_2^3 = x_1 (35)$$

$$N_3^3 = \frac{1}{2}(x_2 + 1) \tag{36}$$

$$N_1^4 = \frac{1}{2}(1 - x_1) \tag{37}$$

$$N_2^4 = \frac{1}{2}(-x_2 + 1) \tag{38}$$

$$N_3^4 = \frac{1}{2}(x_2 + 2x_1 - 1) \tag{39}$$

Then we proceed to assemble element stiffness matrix and load vectors. Because the essential boundary condition is 0, the load vectors associated with the essential boundary condition is 0.

$$K_{ab}^e = \int_{\Omega} \nabla N_a^e \cdot \nabla N_b^e + N_a^e N_b^e \, d\Omega \tag{40}$$

$$F_a^e = \int_{\Omega} (x_1 + x_2) N_a^e \, d\Omega \tag{41}$$

$$K^{1} = \begin{bmatrix} 17/12 & -1/6 & -11/12 \\ -1/6 & 5/12 & 1/12 \\ -11/12 & 1/12 & 7/6 \end{bmatrix}$$

$$K^{2} = \begin{bmatrix} 7/6 & -11/12 & 1/12 \\ -11/12 & 17/12 & -1/6 \\ 1/12 & -1/6 & 5/12 \end{bmatrix}$$

$$K^{3} = \begin{bmatrix} 17/12 & -11/12 & -1/6 \\ -11/12 & 7/6 & 1/12 \\ -1/6 & 1/12 & 5/12 \end{bmatrix}$$

$$K^{4} = \begin{bmatrix} 7/6 & 1/12 & -11/12 \\ 1/12 & 5/12 & -1/6 \\ -11/12 & -1/6 & 17/12 \end{bmatrix}$$

$$F^{1} = \begin{bmatrix} -1/12 \\ -1/4 \\ 0 \end{bmatrix}$$

$$F^{2} = \begin{bmatrix} -1/3 \\ -1/4 \\ -1/12 \end{bmatrix}$$

$$F^{3} = \begin{bmatrix} -1/12 \\ 0 \\ 1/12 \end{bmatrix}$$

$$F^{4} = \begin{bmatrix} 1/3 \\ 1/4 \\ 5/12 \end{bmatrix}$$

5. (5) Denote $\partial\Omega_D$ as line 1, and $\partial\Omega_N$ as line 2. Construct the array of boundary edges BE and compute the load vector associated to the natural boundary condition.

Solution: The array of boundary edges is

$$\mathsf{BE} = \begin{bmatrix} 5 & 1 & 2 & 6 \\ 1 & 2 & 6 & 5 \\ 2 & 2 & 2 & 1 \end{bmatrix}.$$

For the load vector associated with natural boundary condition. We can see that we only need to compute the vector with the edge with nodes 1,2, because $x_2^2 - 1 = 0$ for other edges with natural boundary condition.

$$F^{12} = \begin{bmatrix} -2/3 \\ -2/3 \end{bmatrix}$$

6. (10) Build the stiffness matrix and load vector.

Solution: After the assembly,

$$K = \begin{bmatrix} 17/12 & -1/6 & -11/12 & 0 & 0 & 0 \\ -1/6 & 19/12 & 1/6 & -11/12 & 0 & 0 \\ -11/12 & 1/6 & 19/6 & -1/3 & -11/12 & 1/6 \\ 0 & -11/12 & -1/3 & 17/6 & 0 & -11/12 \\ 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix}$$

$$F = \begin{bmatrix} -3/4 \\ -5/4 \\ 1/3 \\ -1/3 \\ 1 \\ -1 \end{bmatrix}$$

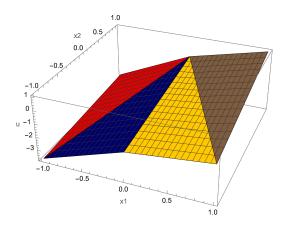


Figure 2: Plot of the numerical solution u_h

7. (5) Compute the finite element approximation, express it as a linear combination of basis functions, and plot it over the square.

Solution:

$$U = \begin{bmatrix} -135345/267089 \\ -364387/267089 \\ 75610/267089 \\ -226828/267089 \\ 1 \\ -1 \end{bmatrix} \approx \begin{bmatrix} -0.506741 \\ -1.36429 \\ 0.283089 \\ -0.84926 \\ 1. \\ -1 \end{bmatrix}$$